

CITY OF SACRAMENTO

Permit No: 9802471

1231 I Street, Sacramento, CA 95814

Insp Area: 2

Site Address: 7554 ASHWOOD WY SAC
Parcel No: 0490266003

Sub-Type: NOTHR
Housing (Y/N): N

CONTRACTOR
MINI BARNES
2400 PRINCETON ST
SAC CA 95815
Phone: 641-7433

OWNER
CLARKSON MARY ALICE
7554 ASHWOOD WY
SACRAMENTO CA 95822
Phone:

ARCHITECT
Phone:

Nature of Work: STORAGE SHED 240 SF

CONSTRUCTION LENDING AGENCY: I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name Lender's Address

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class B License Number 569596 Date Mar 27 Contractor Signature Lynette Paulhus

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his or her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he or she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. B & PC for this reason:
Date Owner Signature

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

Date Mar 27, 98 Applicant/Agent Signature Lynette Paulhus

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:
I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

X I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:
Carrier Unicare Insurance Coe Policy Number SA50-0997-17349

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date Mar 27, 98 Applicant Signature Lynette Paulhus

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

Planning Division COMMERCIAL PRELIMINARY Information Request

BUILDING CHECK ONE:

- Over the counter review and issue permit
- Will be taken in and reviewed for site conditions
- Will be taken in but not reviewed for site conditions
- Information only, pre-submittal information

Customer Name: William Parnis Phone Number: \_\_\_\_\_

Project address: ~~163~~ <sup>7934</sup> ~~163~~ ASHWOOD WY.

APN: \_\_\_\_\_ Current site use: \_\_\_\_\_

INITIAL

Need to verify AN Proposed Site use: \_\_\_\_\_

Describe what is being requested: APPROVAL & COMMENTS

to build within setback

Requested by: Bill Date: \_\_\_\_\_

Zone R-1 Overlay / SPD / PUD / R-review \_\_\_\_\_

- Planning staff Review required
- Planning Hearing required
- Design Review required
- No Planning Issues
- Counter ok review by site cond.

Prior Applications on site P# \_\_\_\_\_ Z# \_\_\_\_\_

DR# \_\_\_\_\_ PB# \_\_\_\_\_ IR# \_\_\_\_\_

Comments: Setback + lot coverage  
O.K.

Planning review by: H Perry Date: 3-20-98

MUST BE REVIEWED BY PLANNING

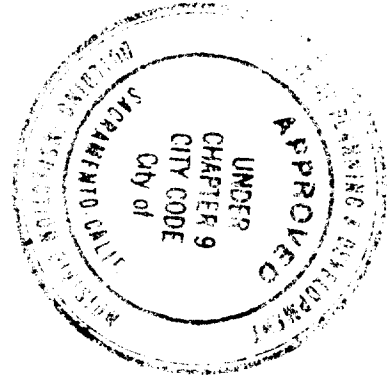
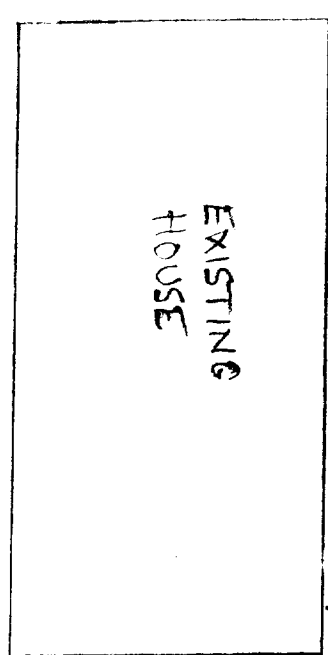
- |                 |                      |               |
|-----------------|----------------------|---------------|
| Care Facilities | Anything Residential | Restaurants   |
| Churches        | Day care             | Sidewalk Cafe |
| Drive-through   | Lot Line adjustments |               |
| Medical Offices | Bars                 |               |

Security cars  
CELLULAR COMMUNICATION FACILITIES

101.78

67.45

15' SETBACK



This set of plans and specifications must be kept on file for a period of times and is unlawful to alter, amend, or otherwise change from the original intent of the City of Sacramento from the date of approval of this plan and specification without the prior written approval of the City of Sacramento or State Law.

105.03

57.27

7554 Ashwood way

ISSUED

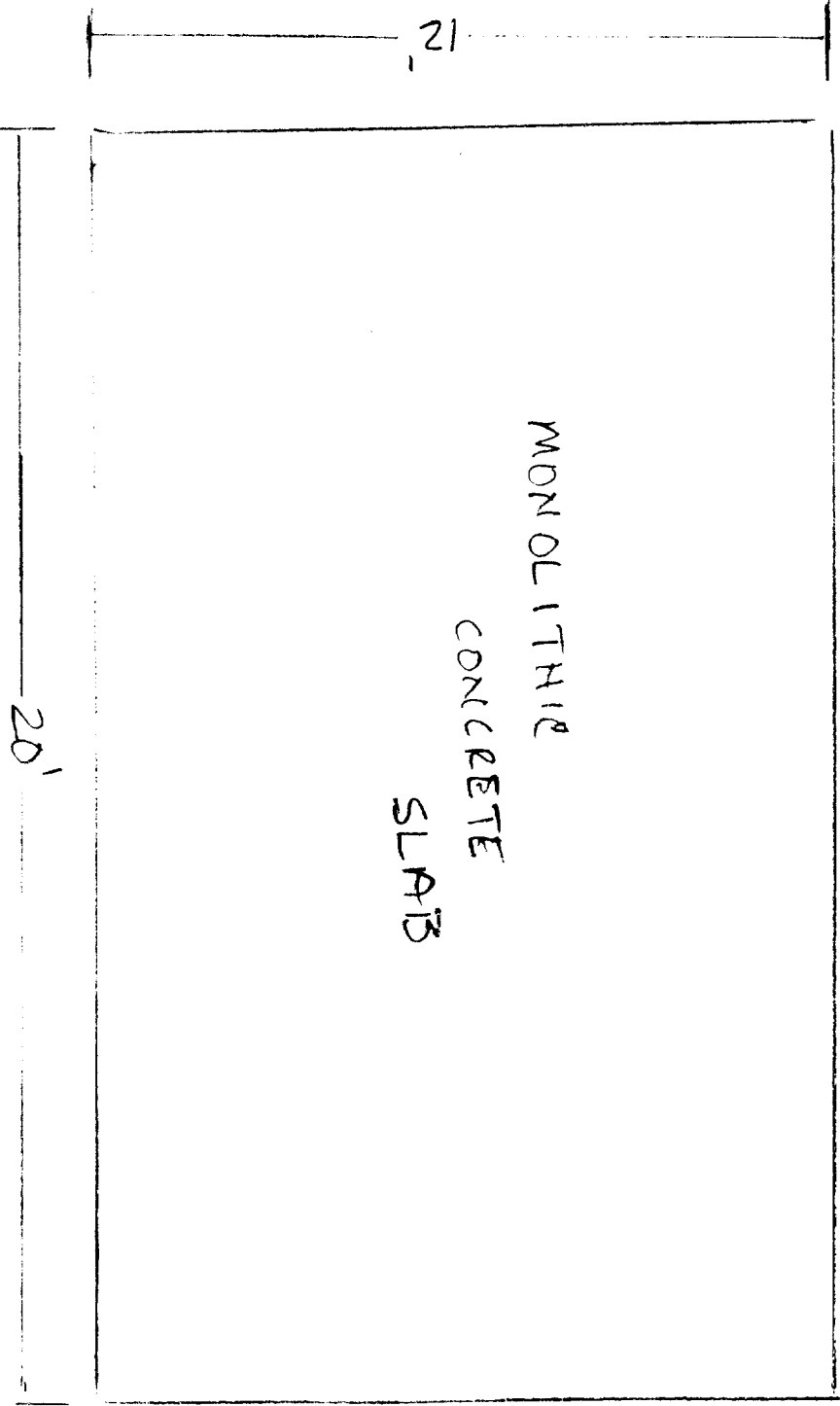
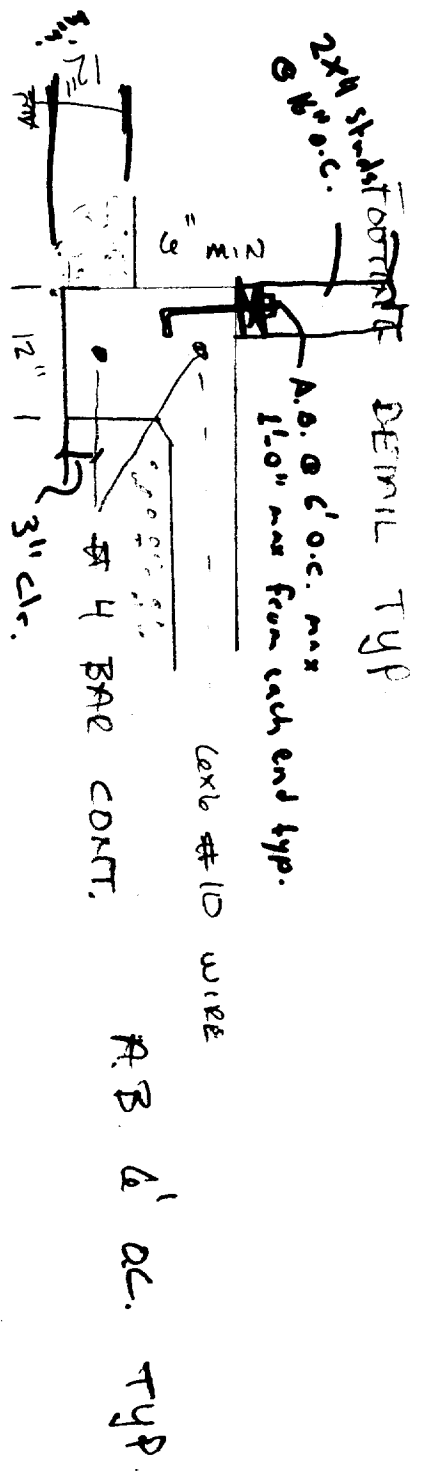
MAR 27 1998

CITY OF SACRAMENTO  
PLANNING & DEVELOPMENT

9802471R

B. Planchack  
G. Thomas

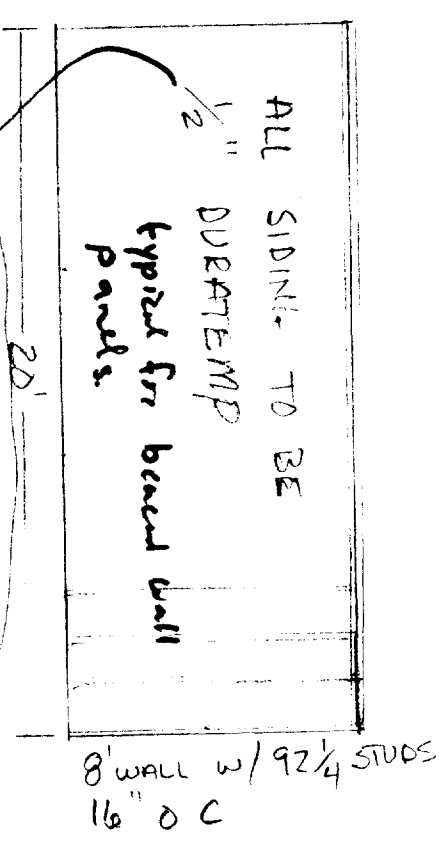
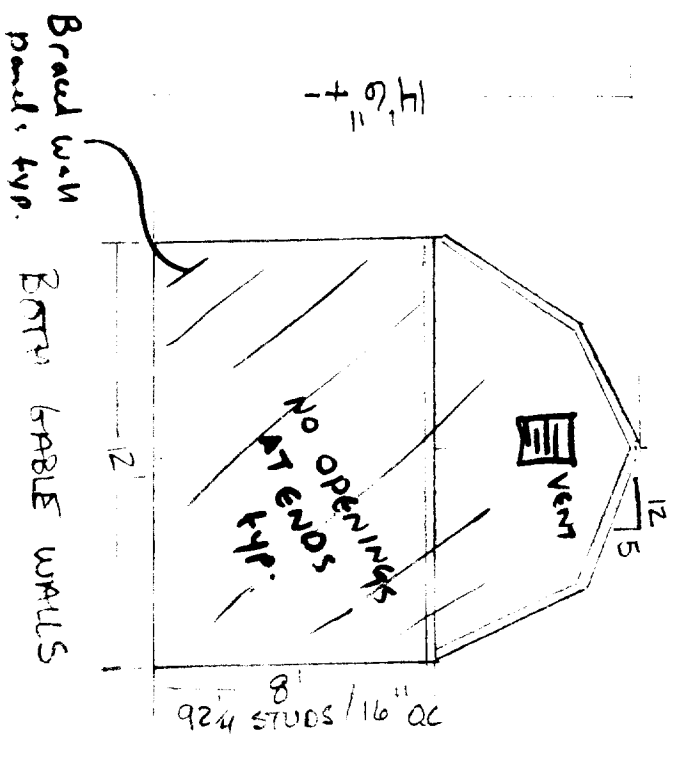
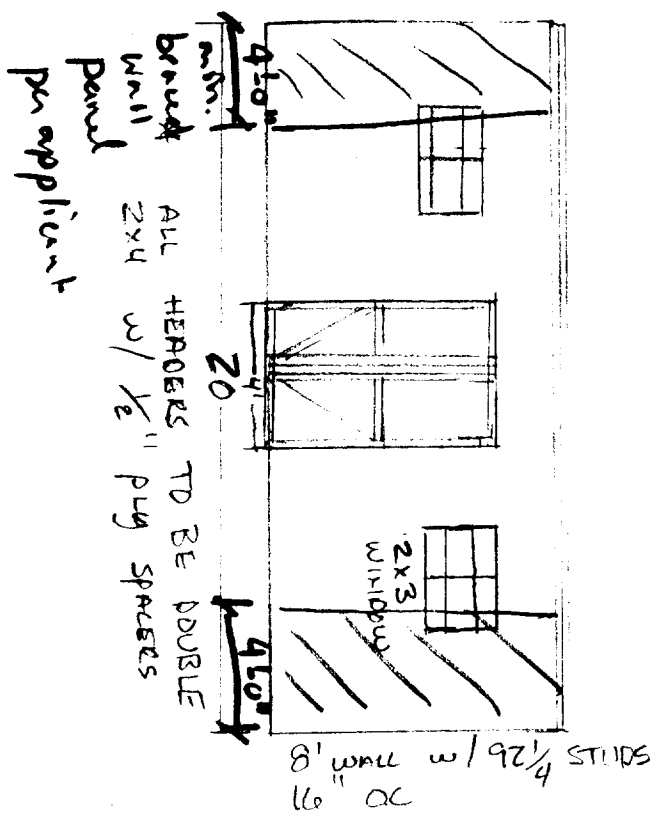
SCALE  $\frac{3}{4}'' = 10'$



**ISSUED**

APR 27 1998

SACRAMENTO  
MENT SERVICES



Equivalent to 1/2" Plyud - cut sheet to be provided to the Field Inspector

ok 12/17

**ISSUED**

10/27/1998

DEVELOP

← DOUBLE STUD

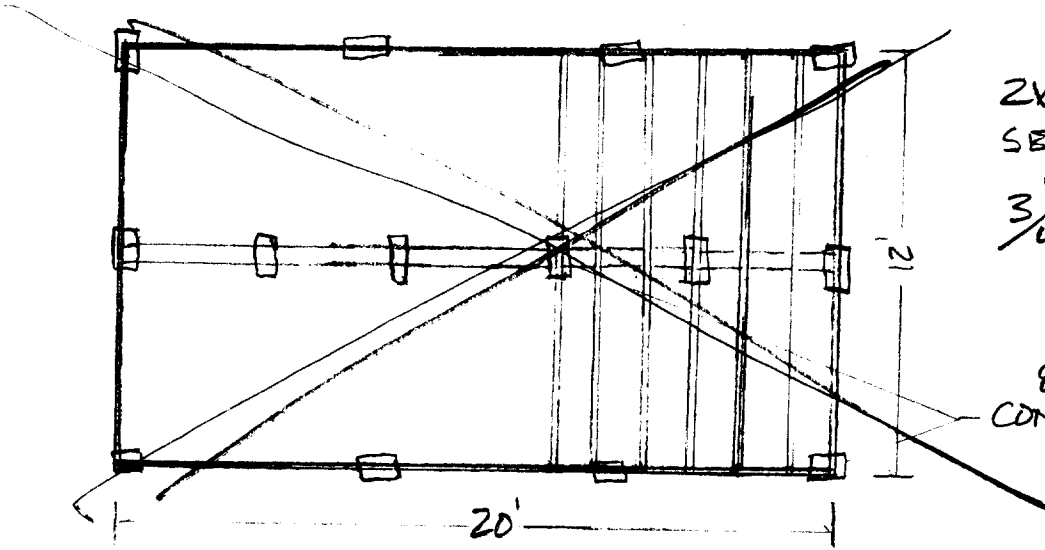
← PANEL 42 INSTALLED AT END CORNER

2x4 PLASTIC SOICR

12

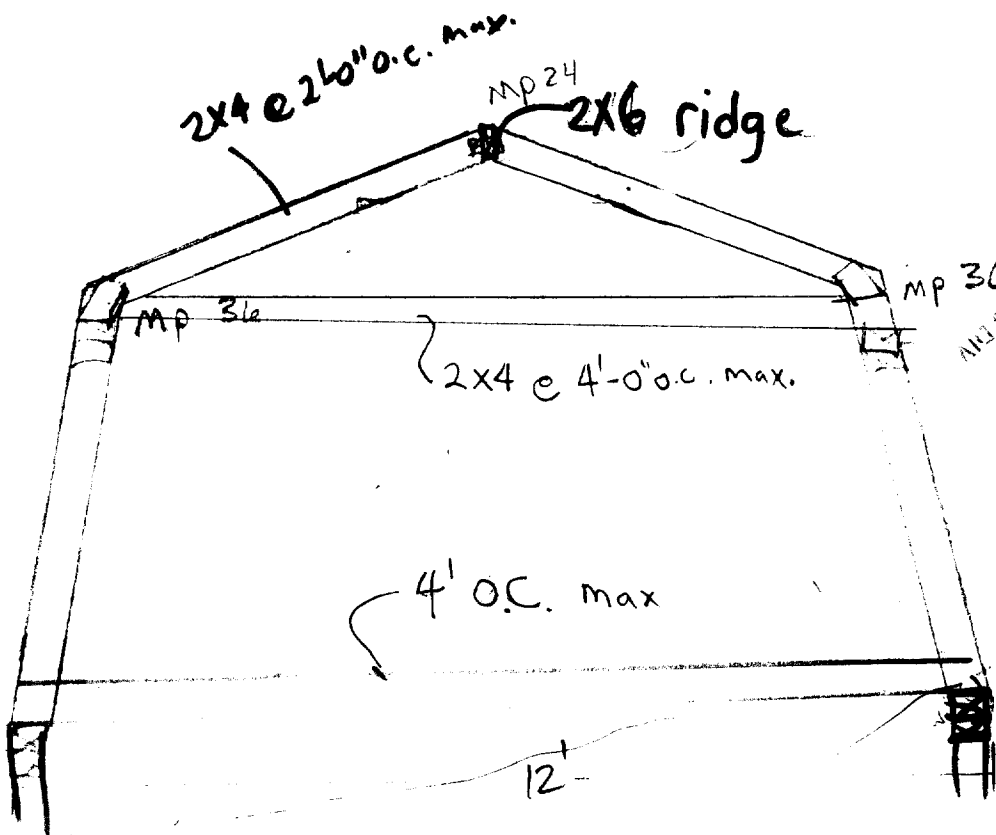
SCALE 3/16

# FLOOR STRUCTURE



2x6 p.t 16" OC.  
 SET ON CONCRETE BLOCK  
 $\frac{3}{4}$ " T&G STURDI FLOOR

8"x16"x2"  
 CONCRETE BLOCK



ISSUED  
 MAR 27 1998  
 CITY OF SACRAMENTO  
 DEVELOPMENT SERVICES DIV

double top  
 plate

ZR

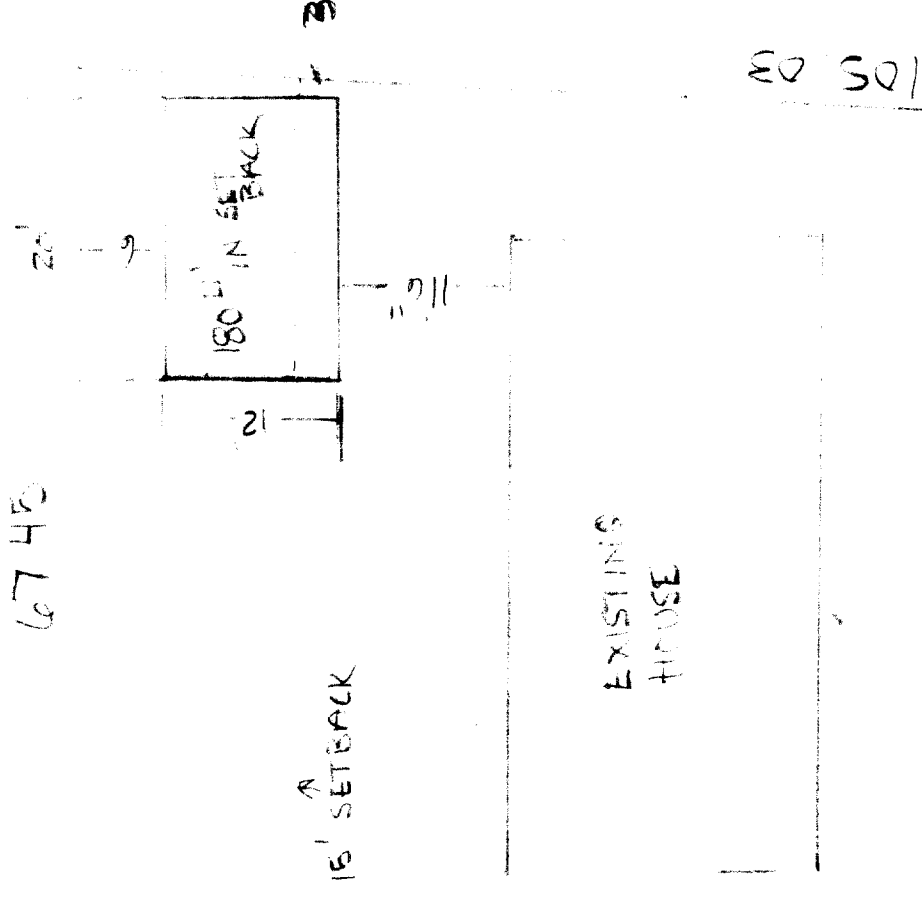
7554 Ashwood way

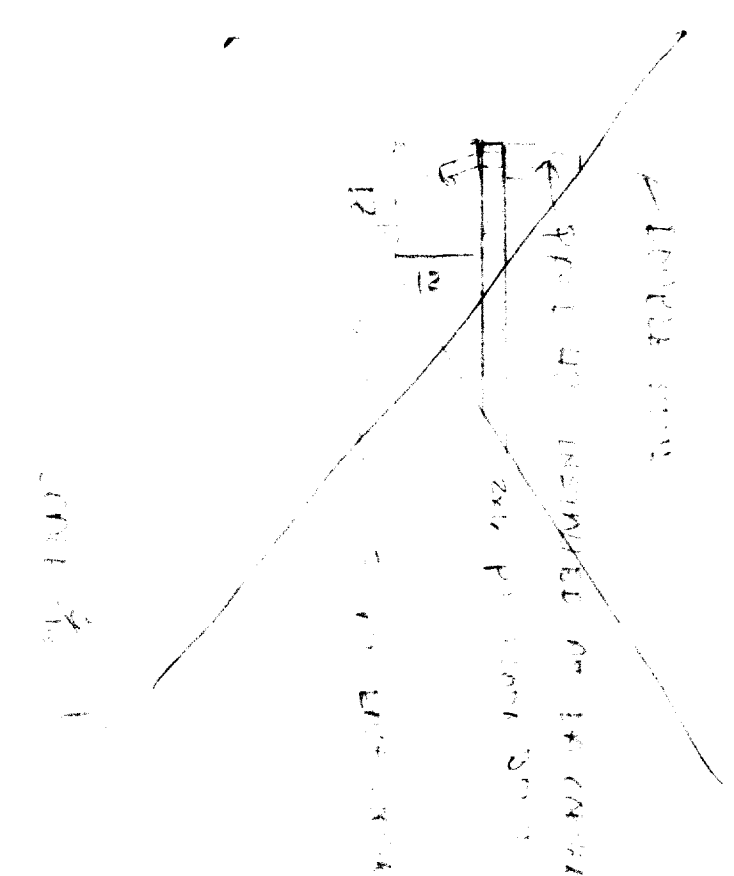
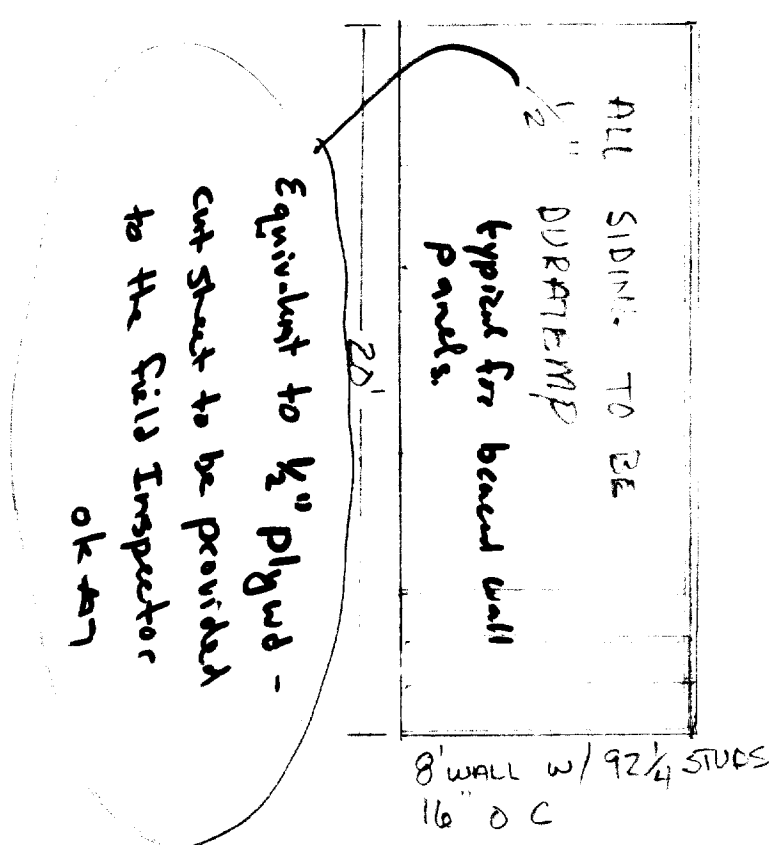
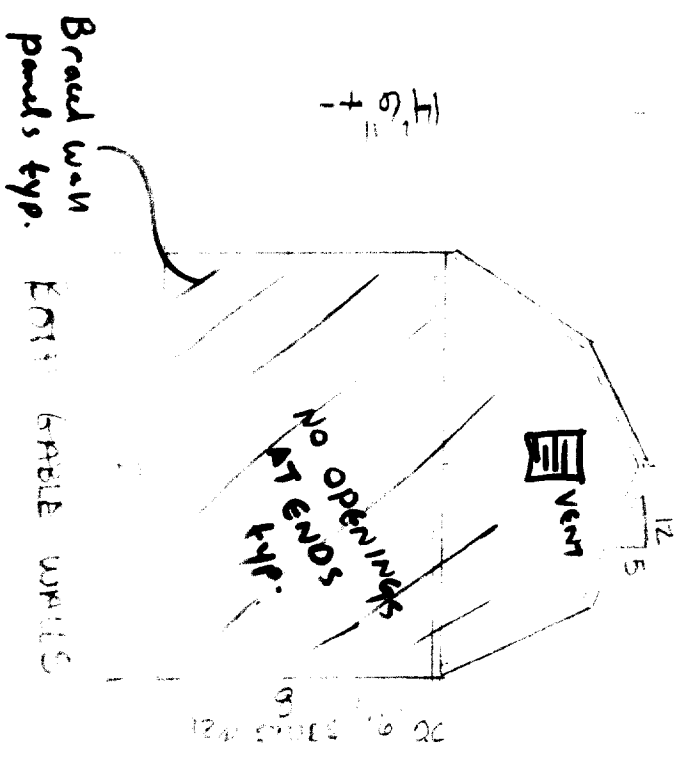
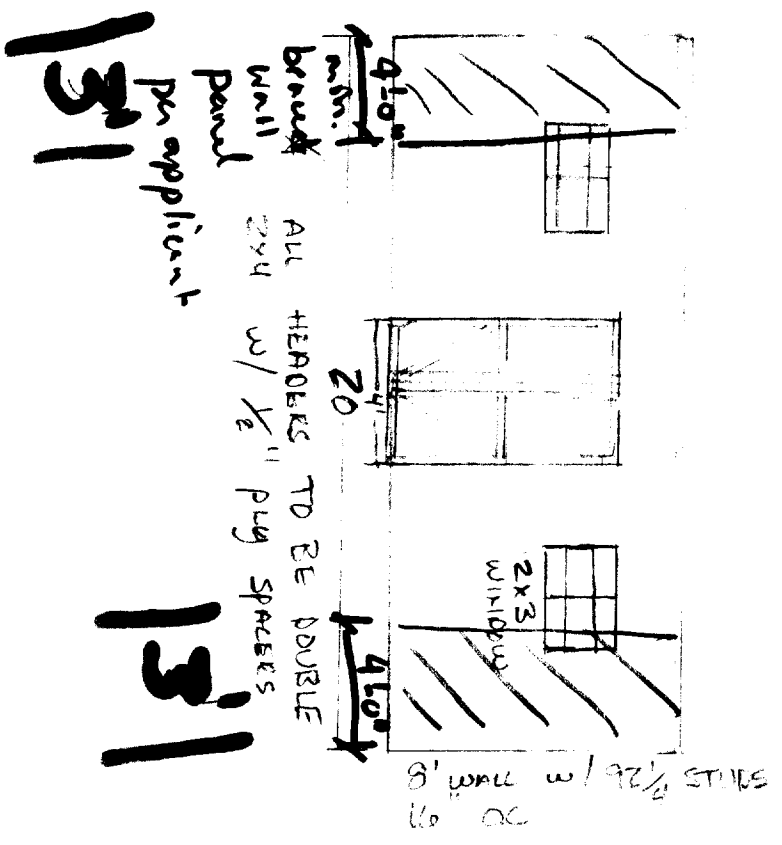
PERMIT #

980427112

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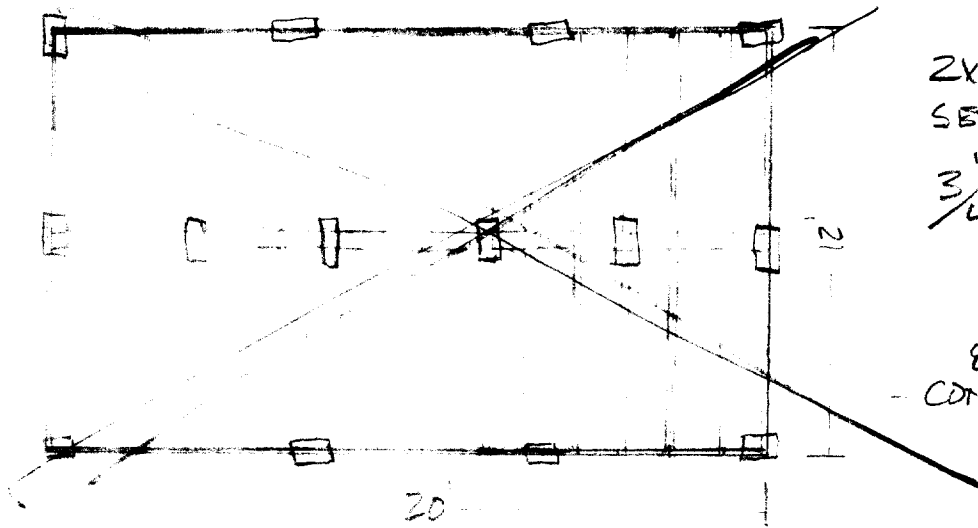
SCALE 1/4" = 10'







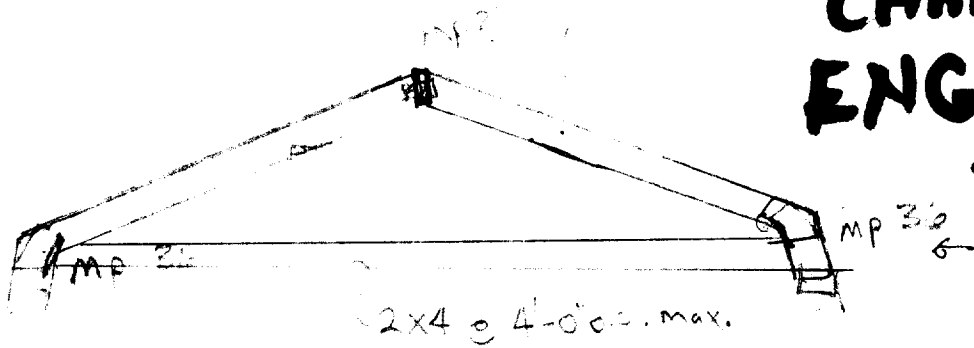
# FLOOR STRUCTURE



2x6 p.t. 16" OC  
SET ON CONCRETE BLOCK  
 $\frac{3}{4}$ " T&G STURD FLOOR

8x16x2"  
CONCRETE BLOCK

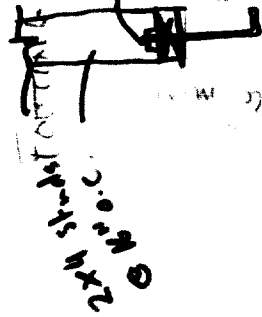
## CHANGE TO ENGINEERED TRUSS



4' O.C. max

12'

PERIM TYP

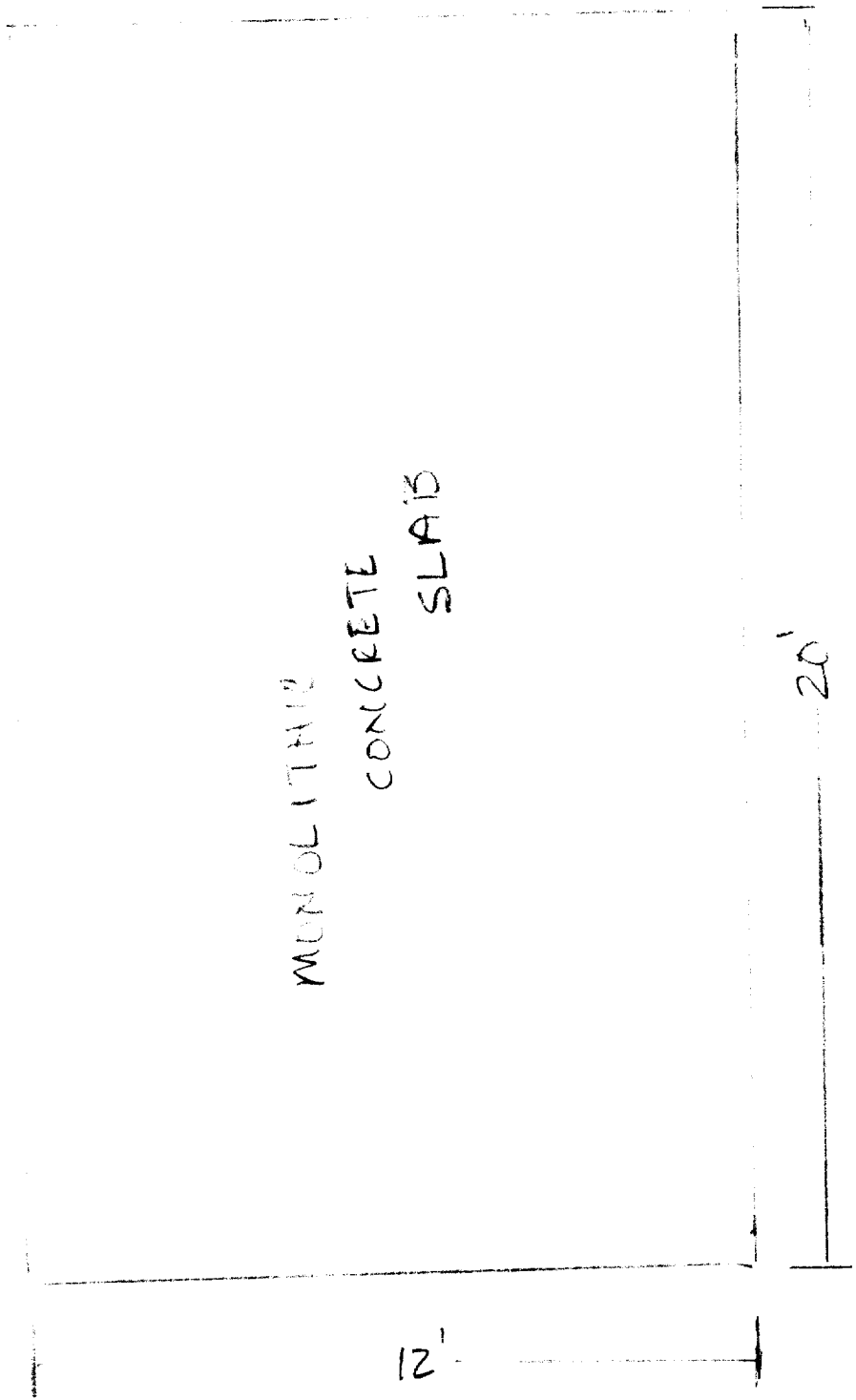


A. 6' 0" o.c. max  
11-0" max from each end typ.

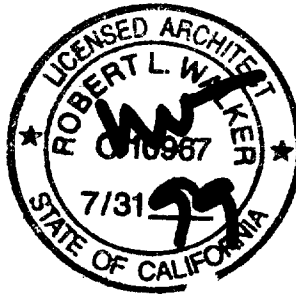
AT 6' 0" o.c. TYP



MONOLITHIC  
CONCRETE  
SLAB



## STRUCTURAL DESIGN



FOR  
MINI-BARNS (THE SHED DEPOT)  
12 X 20 BARN  
2400 PRINCETON ST.  
SACRAMENTO, CA 95815

(916) 641-7433

Scott + Lynette

Bus.  
shop

BY

ROBERT L. WALKER  
ARCHITECT  
IMAGES DESIGN GROUPE  
203 1/2 WEST MAIN STREET  
GRASS VALLEY, CALIFORNIA 95945  
530/ 274-8144  
May 6, 1998

1994 Uniform Building Code - Douglas Fir Larch -

Size	Grade	Base Fb	Cf	Single	Cr	Repetitive	Shear Fv	Fc Bearing	Fc (II)	E X 1000 psi	
				SNOW NO		SNOW NO					
2 x 4	#1 and BTR	1150	1.5	1984	1725	1.15	282	1984	625	1500	1800
2 x 4	#1	1000	1.5	1725	1500	1.15	1983	1725	625	1450	1700
2 x 4	#2	875	1.5	1510	1313	1.15	1735	1509	625	1300	1600
2 x 4	Const.	1000	1	1150	1000	1.15	1322	1150	625	1600	1500
2 x 4	Standard	550	1	632	550	1.15	728	633	625	1350	1400
2 x 4	Stud	675	1.1	854	743	1.15	981	854	625	825	1400
2 x 6	#1 and BTR	1150	1.3	1719	1495	1.15	1777	1719	625	1500	1800
2 x 6	#1	1000	1.3	1495	1300	1.15	1719	1495	625	1450	1700
2 x 6	#2	875	1.3	1309	1138	1.15	1564	1308	625	1300	1600
2 x 6	Stud	675	1	776	675	1.15	812	776	625	825	1400
2 x 8	#1 and BTR	1150	1.2	1587	1380	1.15	1825	1587	625	1500	1800
2 x 8	#1	1000	1.2	1380	1200	1.15	1587	1380	625	1450	1700
2 x 8	#2	875	1.2	1208	1050	1.15	1389	1208	625	1300	1600
2 x 10	#1 and BTR	1150	1.1	1455	1265	1.15	1673	1455	625	1500	1800
2 x 10	#1	1000	1.1	1265	1100	1.15	1455	1265	625	1450	1700
2 x 10	#2	875	1.1	1107	963	1.15	1273	1107	625	1300	1600
2 x 12	#1 and BTR	1150	1	1322	1150	1.15	1521	1323	625	1500	1800
2 x 12	#1	1000	1	1150	1000	1.15	1322	1150	625	1450	1700
2 x 12	#2	875	1	1008	875	1.15	1160	1006	625	1300	1600
4 x 6	#1 and BTR	1150	1.3	1719	1495			0	625	1500	1800
4 x 6	#1	1000	1.3	1495	1300			0	625	1450	1700
4 x 6	#2	875	1.3	1309	1138	-		0	625	1300	1600
4 x 8	#1 and BTR	1150	1.3	1719	1495			0	625	1500	1800
4 x 8	#1	1000	1.3	1495	1300			0	625	1450	1700
4 x 8	#2	875	1.3	1309	1138			0	625	1300	1600
4 x 10	#1 and BTR	1150	1.2	1587	1380			0	625	1500	1800
4 x 10	#1	1000	1.2	1380	1200			0	625	1450	1700
4 x 10	#2	875	1.2	1208	1050			0	625	1300	1600

Size	Grade	Base Fb	Cf	Single	Cr	Repetitive	Shear Fv	Fc Bearing	Fc (II)	E X 1000 psi
4 x 12	#1 and BTR	1150	1.1	1455	1265	0	95	625	1500	1800
4 x 12	#1	1000	1.1	1165	1100	0	95	625	1450	1700
4 x 12	#2	875	1.1	1107	963	0	95	625	1300	1600
4 x 14	#1 and BTR	1150	1	1222	1150	0	95	625	1500	1800
4 x 14	#1	1000	1	1150	1000	0	95	625	1450	1700
6 x 6	Select	1600	1.08	1487	1728	0	85	625	1100	1600
6 x 6	#1	1300	1.08	1615	1404	0	85	625	925	1600
6 x 8	Select	1600	1.04	1914	1664	0	85	625	1100	1600
6 x 8	#1	1300	1.04	1555	1352	0	85	625	925	1600
6 x 10	Select	1600	1.02	1877	1632	0	85	625	1100	1600
6 x 10	#1	1300	1.02	1525	1326	0	85	625	925	1600
6 x 12	Select	1600	1	1840	1600	0	85	625	1100	1600
6 x 12	#1	1300	1	1415	1300	0	85	625	925	1600
4 x 4/6/8/10	Select	1500							1150	1600
4 x 4 Post	#1	1200							1000	1600
4 x 4 Post	#2	725							700	1300
6 x 6/8/10	Select	1500							1150	1600
6 x 6 Post	#1	1200							1000	1600
									700	1300
Glue-Lam										
24F-V4	DF/DF	2160	2400	simple beam type			165	650	1650	1800
24F-V8	DF/DF	2160	2400	used for positive and negative moment			165	650	1650	1800

L 5



# ROOF CONNECTION SOLUTIONS

## LOADING INFORMATION

DEAD LOAD: 10 PSF  
LIVE LOAD: 16 PSF (PER '94 UBC TABLE 16-C)

## JOINT FORCE SOLUTIONS (WORST CASE):

### RIDGE:

159 X 2 = 318 LBS (VERT)  
1.25 X 23 = 29  
29 X 2 = 58 LBS (LATERAL)

### @PITCH CHANGE:

159 X 1 = 159 LBS (VERT)  
3.25 X 23 = 75  
75 X 2 = 150 LBS (LATERAL)

### @ TOP PLATE:

159 X 1 = 159 LBS (VERT) (N/A)  
2 X 23 = 46  
46 X 2 = 92 LBS (LATERAL)

## LOADS ON JOINTS (WORST POSSIBLE CASE)

USE MITEK CONNECTOR PLATES OR SIMPSON CLIPS

### FOR LOAD @ RIDGE CONNECTION

$$(318+58) / 119 = 3.16$$

USE: MITEK M-16 W/ 4 SQ. IN. OF PLATE EA MEMBER  
(W/ SLOTS (MOSTLY) PARALLEL TO GRAIN)

### FOR LOAD @ HORIZ / VERT ROOF MEMBER CONNECTION

$$(159+150) / 119 = 2.5$$

USE: MITEK M-16 W/ 3 SQ. IN. OF PLATE EA MEMBER  
(W/ SLOTS (MOSTLY) PARALLEL TO GRAIN)

### FOR TOP PLATE / VERT ROOF MEMBER CONNECTION

LATERAL PRESSURE = 92 LBS

H2.5 = 150 LBS

USE: SIMPSON H2.5 TIE  
(SEE MANUFACTURER'S SPECIFICATIONS FOR ATTACHMENT)

1996 Accumulative Supplement: This report is unaffected by the supplement.

This report is subject to re-examination in two years.

TABLE 1--ALLOWABLE LATERAL RESISTANCE VALUES

PLATE MODEL	LUMBER SPECIES	SPECIFIC GRAVITY	DIRECTION OF GRAIN AND LOAD WITH RESPECT TO LENGTH OF PLATE <sup>1</sup>			
			AA	EA	AE	EE
			Allowable Load Per Plate <sup>2</sup> (pounds per square inch of plate contact area)			
M-20 and M II 20	Southern yellow pine	0.55	249	190	184	200
	Douglas fir-larch	0.50	220	195	180	190
	Hem-fir	0.43	185	148	129	145
	Spruce-pine-fir	0.42	197	144	143	137
M-18 and M II 18	Southern yellow pine	0.55	195	188	159	151
	Douglas fir-larch	0.50	170	148	157	147
	Hem-fir	0.43	151	147	148	141
	Spruce-pine-fir	0.42	140	138	134	108
M-16 and M II 16	Southern yellow pine	0.55	174	126	147	122
	Douglas fir-larch	0.50	176	121	137	126
	Hem-fir	0.43	119	64	102	98
	Spruce-pine-fir	0.42	127	82	75	107
M-20 HS and M II 20 HS	Southern yellow pine	0.55	187	143	138	150
	Douglas fir-larch	0.50	165	146	135	143
	Hem-fir	0.43	139	111	97	109
	Spruce-pine-fir	0.42	148	108	107	109

For SI: 1 lb/inch<sup>2</sup> = 6.89 kPa.

<sup>1</sup>See Figure 1 for a description of plate orientation.

<sup>2</sup>Metal connector plates are installed in pairs on opposite faces of truss members.

TABLE 2--ALLOWABLE TENSION VALUES AND TENSION EFFICIENCY RATIOS<sup>1</sup>

PLATE MODEL	DIRECTION OF LOAD WITH RESPECT TO LENGTH OF PLATE			
	0°	90°	0°	90°
	Allowable Tension Load <sup>2</sup> (pounds per inch per pair of plates)		Tension Load Efficiency Ratio	
M-20 and M II 20	1126	840	0.72	0.47
M-18 and M II 18	1402	1120	0.59	0.47
M-16 and M II 16	1985	802	0.69	0.28
M-20 HS and M II 20 HS	1661	1063	0.64	0.41

For SI: 1 lb/inch = 0.175 N/mm.

<sup>1</sup>See Figure 2 for a description of plate orientation.

<sup>2</sup>The length of plate refers to the dimension of the longitudinal axis of the area of the plate from which the plate teeth were sheared during plate fabrication.

<sup>3</sup>The allowable tension loads are based on base metal thicknesses of 0.0368, 0.0493 and 0.060 inch for the No. 20, 18 and 16 gage steel plates, respectively.

TABLE 3--ALLOWABLE SHEAR VALUES AND SHEAR EFFICIENCY RATIOS

PLATE MODEL	DIRECTION OF LOAD WITH RESPECT TO LENGTH OF PLATE											
	0°	30°	60°	90°	120°	150°	0°	30°	60°	90°	120°	150°
	Allowable Shear Load <sup>1</sup> (pounds per inch per pair of plates)						Shear Load Efficiency Ratio					
M-20 and M II 20	574	738	936	645	490	544	0.49	0.63	0.79	0.55	0.42	0.46
M-18 and M II 18	748	991	1174	815	626	589	0.47	0.63	0.74	0.52	0.40	0.37
M-16 and M II 16	1041	1173	1402	1055	914	672	0.54	0.61	0.73	0.55	0.48	0.35
M-20 HS and M II 20 HS	761	1085	1184	792	608	537	0.43	0.61	0.67	0.45	0.34	0.30

For SI: 1 lb/inch = 0.175 N/mm.

<sup>1</sup>The allowable shear loads are based on base metal thicknesses of 0.0368, 0.0493 and 0.060 inch for the No. 20, 18 and 16 gage steel plates, respectively.



## **LATERAL ANALYSIS**

EXPOSURE: C  
WIND SPEED: 80 mph  
LIVE LOAD: 16 psf

### **FRONT WALL OF BUILDING**

#### **UPLIFT SOLUTION**

Dist. from Plate to Plate:	<u>8 ft</u>
Dist. from Upper Plate to Roof Top	<u>6.5 ft</u>
Wind Load ( $P=C_e \cdot C_q \cdot I \cdot Q_s$ ):	<u>22.59 psf</u>
Length of Wall:	<u>20 ft</u>
Dead Load on Wall Supported:	<u>10 psf</u>
Trib Width of Dead Load:	<u>6 ft</u>
Length of Wall Attached to Fdn:	<u>16 ft</u>
TOTAL UPLIFT:	<u>3276 lbs</u>
TOTAL RESISTANCE TO UPLIFT:	<u>1200 lbs</u>
NET UPLIFT:	<u>2076 lbs</u>
UPLIFT PER FOOT:	<u>130 plf</u>

#### **SHEAR SOLUTION PARALLEL TO WALL**

1/2 Width of Perp. Wall:	<u>6 ft</u>
Length of Shear Panel:	<u>3 ft</u>
TOTAL SHEAR @ SUPPORTING WALL:	<u>1084 lbs</u>
SHEAR PER FOOT :	<u>361.4 ft</u>

#### **SHEAR SOLUTION PERP. TO WALL**

TOTAL SHEAR @ BASE:	<u>1807 lbs</u>
Length of Wall Attached to Fdn:	<u>16 ft</u>
SHEAR PER FOOT:	<u>113 plf</u>

**USE:** 3' MIN PANEL SIZE, WITH:



3/8" CDX PLYWOOD SHEAR WALL W/8d  
@ 4"/12" O.C. BOTTOM PLATE TO  
FLOOR AND RIM JOIST OR BLOCKING  
TO PLATE NAILING 16d @ 3" O.C. 1/2"  
DIAMETER ANCHOR BOLTS @ 24" O.C.

(OR 1/2" DURATEMP - SAME NAILING)

# FOUNDATION ANALYSIS

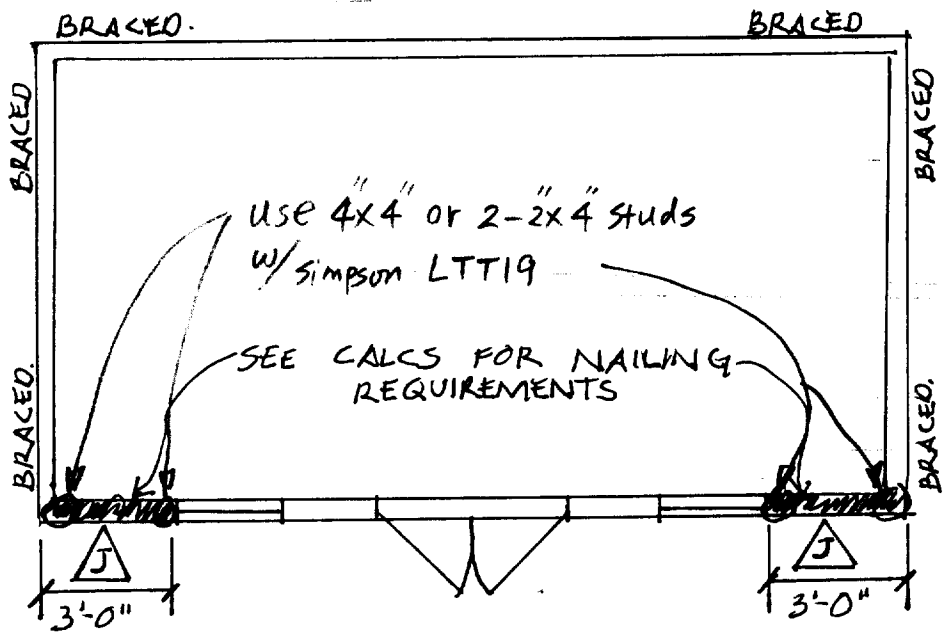
FOUNDATION CONTINUOUS FOOTING TYPICAL  
ALLOWABLE SOIL BEARING PRESSURE: 1000 PSF  
REQ. WIDTH = TOTAL / S.B.P. X 12

LOADS:	ROOF:	106 lbs
	WALL:	66 lbs
	FOOTING:	150 lbs
	<u>TOTAL:</u>	<u>322 lbs</u>

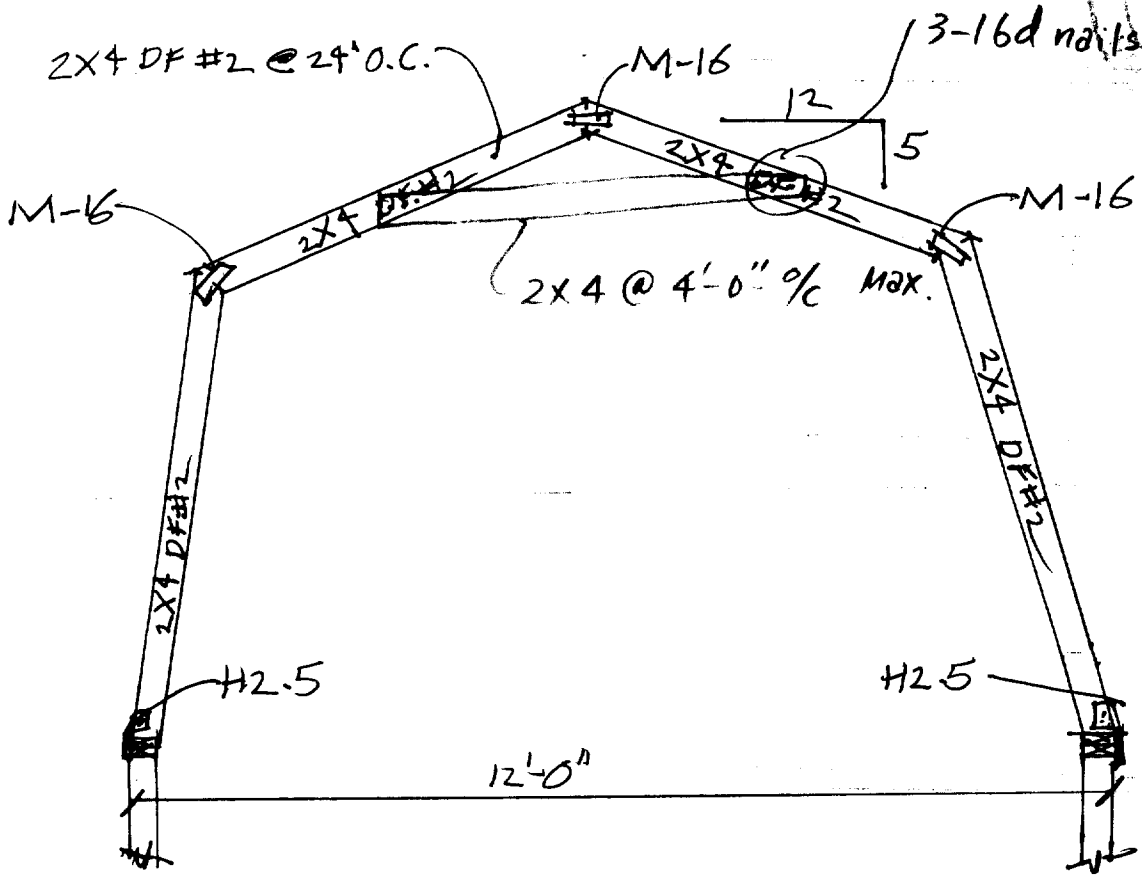
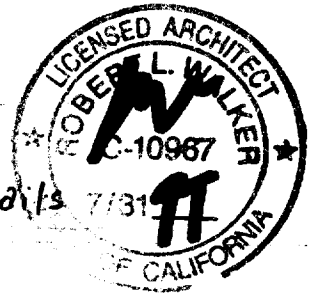
REQ. WIDTH: 3.86 in  
USE: STANDARD UBC FOOTINGS  
12" WIDE X 12" DP  
W/ 2-#4 DEF-BAR CONTINUOUS

**BOTTOM PLATE TO SLAB ATTACHMENT FOR BRACED WALLS:**

1/2" DIA ANCHOR BOLTS, 7" DEEP MIN. @ 6'-0" O.C. MAX SPACING  
(1'-0" MAX FROM EDGE OF EA. WALL)  
(IF SLAB IS EXISTING, DRILL 5/8" DIA HOLES AND EPOXY OR WEDG



SHEAR DIAGRAM.



ROOF SECTION.