

CITY OF SACRAMENTO

1231 I Street, Sacramento, CA 95814

Permit No: 9903377

Insp Area: 2

Site Address: 7331 RIVER PLACE WY SAC

Parcel No: 031-0470-027

Sub-Type: RES

Housing (Y/N): N

CONTRACTOR

ZIMMERMAN ROOFING
3560 RAMONA AV
SACRAMENTO CA 95826

OWNER

HUTCHINSON WILBUR T
7331 RIVER PLACE WY
SACRAMENTO CA 95831

ARCHITECT

Nature of Work: TEAR OFF AND REROOF WITH TILE/ENG. APPROVED BY MATT P

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class: 70 License Number: 1234567 Date: 10/27/99 Contractor Signature: [Signature]

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00):

____ I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

____ I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

____ I am exempt under Sec. _____ B & PC for this reason: _____

Date _____ Owner Signature _____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and herby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date: 10/27/99 Applicant/Agent Signature: [Signature]

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

____ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

____ I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier STATE COMP INS FUND Policy Number 713-98-2021 Exp Date 10/01/1999

____ (This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date: 10/27/99 Applicant Signature: [Signature]

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.



DEPARTMENT OF
PLANNING AND DEVELOPMENT

CITY OF SACRAMENTO
CALIFORNIA

1231 I STREET
ROOM 200
SACRAMENTO, CA
95814-2928

Permit Services
916-264-7619
FAX 916-264-7046

TILE ROOF WORKSHEET

This worksheet must be filled out whenever any type of tile roof is applied for.

If the answer to question #5 is yes, a written engineering report from a registered engineer must be provided with each application.

1. BRAND AND MODEL OF TILE Pioneer Lightweight
2. TILE WEIGHT PER SQUARE 7.30 lbs.
3. WEIGHT OF ROOF SYSTEM PER SQUARE 180 lbs.
4. TOTAL WEIGHT OF ROOF SYSTEM 910 lbs.
5. DOES TOTAL WEIGHT OF ROOF SYSTEM EXCEED 750# PER SQUARE? YES NO
6. ROOF SLOPE 4/12

PLEASE PROVIDE A SEPARATE WORKSHEET FOR EACH APPLICATION INVOLVING A TILE ROOF

All attached engin. report

Huthinson

Paul Zacher - Structural Engineers
4701 Lakeside Way
Fair Oaks, CA 95628

TEL: 916.961.3960
FAX: 916.961.3960

March 24, 1999

Zimmerman Roofing
3560 Ramona Avenue
Sacramento, CA 95826
TEL: 916.454.3667
FAX: 916.455.3784
TEL (Jeff): 916.392.1971
FAX (Jeff): 916.392.6853
FAX (Framer): 916.383.5308



Attn.: Mr. Jeff Tucker,

re: Job 99050: HUTCHINSON

Subject: Structural Investigation Report of the Roof for the Residence located at 7331 Riverplace Way, Sacramento, CA 95831.

As requested by Mr. Jeff Tucker, this is a report to determine what needs should be addressed to correct any structural deficiencies of the roof. Paul Zacher visited the site March 23, 1999. The investigation was made to determine the existing condition of the structure. All information, data and analysis contained within this report is based on the 1994 Uniform Building Code.

The following is based on visual observations with no subsurface investigation being made.

DESCRIPTION:

Type of Facility: Residence.
Year Built: Estimated 1970's vintage.
Occupancy: Residential.
No. of Stories: One.
Dimensions: Approximately 2000 square feet with a first story plate height of 8 feet.

CONSTRUCTION:

Roof:
The roof covering will consist of Pioneer Light Weight Concrete Tile over 1/2" solid sheathing.
The living and garage areas are framed with pre-engineered wood trusses spaced at 24" on center.

*Reviewed by M.H.P. 4/12/99
This report does not require
1/10 any structural mods.*

Huthinson



Paul Zacher - Structural Engineers
4701 Lakeside Way
Fair Oaks, CA 95628

TEL: 916.961.3960
FAX: 916.961.3960

CONCLUSIONS:

Roof:

The living and garage areas have sufficient structural capacity for the applied live and dead loads.

RECOMMENDATIONS:

None.

It shall be noted that small hairline cracking may occur at exterior stucco and interior gypboard finished walls which are load bearing or distributing roof strut loads. These cracks are a natural occurrence as the existing structure re-distributes the new roof weight. They are cosmetic in nature and are not an indication of a structural hazard or failure.

It shall be noted that some deflection of the rafters may be evident after installation of the tile. The existing roof framing has deflected but this may not be readily evident due to the uneven nature of the existing roofing material. Concrete tile is a very consistent and uniform product and when installed in an even plane, even small deflections can become apparent. This is only a cosmetic issue and not a structural concern.

The inspection consisted of visual observation only, made solely to determine the structural capacity of the existing roof. Analysis does not determine any effects on the overall structure under lateral forces or effects on the foundation unless specifically noted in the calculations and in this document. No warranties, expressed or implied, are made or intended in conjunction with this report. The inspection was made only to the portions that were accessible. The specific items noted were those that were observable and there may be defects which are not observable, or are hidden by architectural and structural materials.

If you have any questions on the above, do not hesitate to call.

Sincerely,

Paul Zacher, P.E., S.E.
file

DESIGN LOADING:

Roof Pitch	4	in 12
Pitch Adjustment Factor	1.05	

LOCATION: ROOF

<u>MATERIAL</u>	<u>WEIGHT</u>	
Pioneer Hacienda Light Wt	5.60	psf
Roofing felt	0.30	psf
1x4 skip sht'g	1.09	psf
1/2" OSB/ plywood	1.50	psf
2x6 rafters @ 24" oc	<u>1.00</u>	psf
Load	9.5	psf
Roof Pitch Adjustment	<u>0.51</u>	psf
Total Load	10.0	psf

LOCATION: VAULT

<u>MATERIAL</u>	<u>WEIGHT</u>	
Pioneer Hacienda Light Wt	5.60	psf
Roofing felt	0.30	psf
1/2" OSB/ plywood	1.50	psf
1x4 skip sht'g	1.09	psf
2x6 rafters @ 24" oc	1.00	psf
Batt/blown insul	0.50	psf
1/2" Gypboard	<u>2.50</u>	psf
Load	12.5	psf
Roof Pitch Adjustment	<u>0.68</u>	psf
Total Load	13.2	psf

LOCATION: TOP CHORD

<u>MATERIAL</u>	<u>WEIGHT</u>	
Pioneer Hacienda Light Wt	5.60	psf
Roofing felt	0.30	psf
1/2" OSB/ plywood	1.50	psf
1x4 skip sht'g	1.09	psf
2x4 truss @ 24" oc	<u>1.28</u>	psf
Load	9.8	psf
Roof Pitch Adjustment	<u>0.53</u>	psf
Total Load	10.3	psf

LOCATION: BOTTOM CHORD

<u>MATERIAL</u>	<u>WEIGHT</u>	
Batt/blown insul	0.50	psf
2x4 truss @ 24" oc	0.64	psf
1/2" Gypboard	<u>2.50</u>	psf
Load	3.6	psf

Title :
Dsgnr:
Description :

Date:
Job #

Scope :

Rev. 510001

Timber Beam & Joist

Page 1

Description RAFTERS AND BEAMS

Timber Member Information

		6x14 vault	4x12 vault
Timber Section		6x14	4x12
Beam Width	in	5.500	3.500
Beam Depth	in	13.500	11.250
Le: Unbraced Length	ft	0.00	0.00
Timber Grade		Douglas Fir - Larch	Douglas Fir - Larch
Fb - Basic Allow	psi	1,350.0	1,000.0
Fv - Basic Allow	psi	85.0	95.0
Elastic Modulus	ksi	1,600.0	1,700.0
Load Duration Factor		1.250	1.250
Member Type		Sawn	Sawn
Repetitive Status		No	No

Center Span Data

Span	ft	18.50	13.25
Dead Load	#/ft	170.00	132.00
Live Load	#/ft	192.00	148.00

Results Ratio = 0.6679 0.7264

Mimax @ Center	in-k	185.84	73.74
@ X =	ft	9.25	6.62
fb : Actual	psi	1,112.4	998.8
Fb : Allowable	psi	1,665.6	1,375.0
		Bending OK	Bending OK
fv : Actual	psi	59.5	61.1
Fv : Allowable	psi	106.3	118.8
		Shear OK	Shear OK

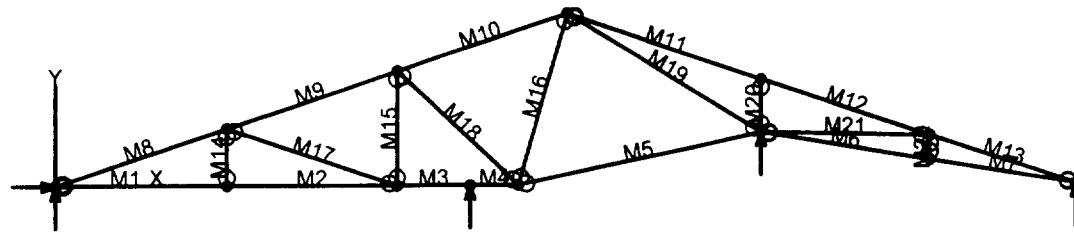
Reactions

@ Left End	DL	lbs	1,572.50	874.50
	LL	lbs	1,776.00	980.50
	Max. DL+LL	lbs	3,348.50	1,855.00
@ Right End	DL	lbs	1,572.50	874.50
	LL	lbs	1,776.00	980.50
	Max. DL+LL	lbs	3,348.50	1,855.00

Deflections

Center DL Defl	in	-0.248	-0.130
L/Defl Ratio		894.0	1,226.3
Center LL Defl	in	-0.280	-0.145
L/Defl Ratio		791.6	1,093.7
Center Total Defl	in	-0.529	-0.275
Location	ft	9.250	6.625
L/Defl Ratio		419.8	578.1

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VisualAnalysis 3.50.c Report

03/24/93 14:23:21

Project:

File: C:\Program Files\IES\VA35\Untitled.vap

Company: PK Associates Engineers

Engineer: Paul Cacher

Default Units: Feet, Pounds, Degrees, °Fahrenheit, Seconds.

Nodes

Node	X ft	Y ft	Fix	DX	Fix	DY	Fix	RZ
N1	0.00	0.00	Yes		Yes		No	
N2	7.00	0.00	No		No		"	
N3	14.00	0.00	"		"		"	
N4	17.00	0.00	"		Yes		"	
N5	19.00	0.00	"		No		"	
N6	29.00	2.17	"		Yes		"	
N7	36.00	1.00	"		No		"	
N8	42.00	0.00	"		Yes		"	
N9	7.00	2.33	"		No		"	
N10	14.00	4.67	"		"		"	
N11	21.00	7.00	"		"		"	
N12	29.00	4.33	"		"		"	
N13	36.00	2.00	"		"		"	

Member Elements

Member	Section	Material	Length ft	Weight lbs	Theta deg
M1	SS2x4	Wood	7.00	10.33	0.00
M2	"	"	7.00	10.33	0.00
M3	"	"	3.00	4.43	0.00
M4	"	"	2.00	2.95	0.00
M5	"	"	10.23	15.10	0.00
M6	"	"	7.10	10.47	0.00
M7	"	"	6.08	8.97	0.00
M8	"	"	7.38	10.88	0.00
M9	"	"	7.38	10.89	0.00
M10	"	"	7.38	10.88	0.00
M11	"	"	8.43	12.44	0.00
M12	"	"	7.38	10.88	0.00
M13	"	"	6.32	9.33	0.00
M14	"	"	2.33	3.44	0.00
M15	"	"	4.67	6.89	0.00
M16	"	"	7.28	10.74	0.00
M17	"	"	7.38	10.88	0.00
M18	"	"	6.84	10.09	0.00
M19	"	"	9.34	13.79	0.00
M20	"	"	2.16	3.19	0.00
M21	"	"	7.00	10.33	0.00
M22	"	"	1.00	1.48	0.00

Section Properties

Category	Section	Ax in ²	Iz in ⁴	Sy+ in ³	Sy- in ³
Wood Sha	SS2x4	5.25	5.36	3.06	3.06

Material Properties

Material	Strength ksi	Elasticity ksi	Poisson	Density lb/ft ³	Therm. /F
Wood	-NA-	1200.00	0.36	40.47	0.00

VisualAnalysis 3.50.c Report

7/24/94 14:11:39

Project:

File: C:\Program Files\IES\VA35\Untitled.vap

Company: PK Associates Engineers

Engineer: Paul Zacher

Default Units: Feet, Pounds, Degrees, °Fahrenheit, Seconds.

Load Cases

Load Case	Strength	Service	Results
(1)Service Case 1	No	No	None
(2)Service Case 2	"	"	"
(3)Equation Case 1	"	"	1st Ord

Service Load Cases

Load Case	Load Source	Self Weight	Loads
Service Case 1	Dead loads	None	
Service Case 2	Roof Live 1	"	

Load Combination Summary

Equation Case: Equation Case 1

Combination: +1D+1L+1Lr+1R+1W+1S+1E+1H+1F+1TS+1T+1TC+1I+1U+1LE

Contributing Cases & Source

Service Case 1 (Dead loads)

Service Case 2 (Roof Live loads)

Equation Case Combinations

Load Case	Cases	Equation
Equation Case 1	0.00	0.00

Member Uniform Loads

Load Case	Member	Direction	Offset ft	End Off ft	Magnitude
Service Case 1	M1	DY proj.	0.00	7.00	-0.01 K/ft
"	M2	"	0.00	7.00	-0.01 K/ft
"	M3	"	0.00	3.00	-0.01 K/ft
"	M4	"	0.00	2.00	-0.01 K/ft
"	M5	"	0.00	10.23	-0.01 K/ft
"	M6	"	0.00	7.10	-0.01 K/ft
"	M7	"	0.00	6.08	-0.01 K/ft
"	M8	"	0.00	7.38	-0.02 K/ft
"	M9	"	0.00	7.38	-0.02 K/ft
"	M10	"	0.00	7.38	-0.02 K/ft
"	M11	"	0.00	8.43	-0.02 K/ft
"	M12	"	0.00	7.38	-0.02 K/ft
"	M13	"	0.00	6.32	-0.02 K/ft
Service Case 2	M8	"	0.00	7.38	-0.03 K/ft
"	M9	"	0.00	7.38	-0.03 K/ft
"	M10	"	0.00	7.38	-0.03 K/ft
"	M11	"	0.00	8.43	-0.03 K/ft
"	M12	"	0.00	7.38	-0.03 K/ft
"	M13	"	0.00	6.32	-0.03 K/ft

VisualAnalysis 3.50.c Report

03/14/99 14:23:39

Project:

File: D:\Program Files\IES\VA35\Untitled.vap

Company: PK Associates Engineers

Engineer: Paul Zacher

Default Units: Feet, Pounds, Degrees, °Fahrenheit, Seconds.

Load Cases

Load Case	Strength Service Results		
(1) Service Case 1	No	No	None
(2) Service Case 2	"	"	"
(3) Equation Case 1	"	"	1st Ord

Member Extreme Results

Member	Fx(lc) K	Fy(lc) K	Mz(lc) K-ft	fc max(lc) ksi	fc min(lc) ksi	Dx(lc) in	Dy(lc) in
M1	1.53(3)	-0.03(3)	-0.04(3)	0.29(3)	0.14(3)	0.00(3)	-0.20(3)
"	1.53(3)	0.02(3)	0.03(3)	0.44(3)	0.29(3)	0.02(3)	-0.00(3)
M2	1.53(3)	-0.01(3)	-0.04(3)	0.30(3)	-0.05(3)	0.02(3)	-0.28(3)
"	1.53(3)	0.04(3)	0.09(3)	0.63(3)	0.28(3)	0.04(3)	-0.17(3)
M3	0.83(3)	-0.14(3)	-0.29(3)	0.18(3)	-1.00(3)	0.04(3)	-0.17(3)
"	0.83(3)	-0.11(3)	0.08(3)	1.31(3)	0.13(3)	0.05(3)	0.00(3)
M4	0.83(3)	0.14(3)	-0.29(3)	0.16(3)	-1.00(3)	0.05(3)	-0.12(3)
"	0.83(3)	0.15(3)	-0.00(3)	1.31(3)	0.16(3)	0.05(3)	-0.00(3)
M5	0.34(3)	-0.04(3)	0.00(3)	0.06(3)	-0.29(3)	0.02(3)	-0.34(3)
"	0.36(3)	0.04(3)	0.09(3)	0.42(3)	0.07(3)	0.03(3)	-0.01(3)
M6	0.55(3)	-0.03(3)	-0.01(3)	0.11(3)	-0.06(3)	0.03(3)	-0.13(3)
"	0.56(3)	0.02(3)	0.04(3)	0.26(3)	0.11(3)	0.04(3)	0.00(3)
M7	0.55(3)	-0.02(3)	-0.01(3)	0.10(3)	-0.01(3)	0.04(3)	-0.12(3)
"	0.55(3)	0.02(3)	0.03(3)	0.22(3)	0.11(3)	0.04(3)	0.01(3)
M8	-1.67(3)	-0.21(3)	-0.23(3)	-0.32(3)	-1.21(3)	-0.02(3)	-0.38(3)
"	-1.55(3)	0.14(3)	0.22(3)	0.62(3)	-0.32(3)	-0.00(3)	-0.00(3)
M9	-0.93(3)	-0.18(3)	-0.26(3)	-0.16(3)	-1.18(3)	-0.04(3)	-0.22(3)
"	-0.81(3)	0.17(3)	0.08(3)	0.87(3)	-0.18(3)	-0.02(3)	-0.16(3)
M10	-0.49(3)	-0.14(3)	-0.26(3)	-0.08(3)	-1.12(3)	-0.04(3)	-0.38(3)
"	-0.38(3)	0.21(3)	0.20(3)	0.93(3)	-0.07(3)	-0.03(3)	-0.10(3)
M11	0.67(3)	-0.24(3)	-0.36(3)	0.15(3)	-1.28(3)	0.02(3)	-0.47(3)
"	0.80(3)	0.16(3)	0.26(3)	1.54(3)	0.15(3)	0.04(3)	0.01(3)
M12	0.70(3)	-0.15(3)	-0.36(3)	0.17(3)	-1.25(3)	0.04(3)	-0.11(3)
"	0.82(3)	0.20(3)	0.07(3)	1.56(3)	0.13(3)	0.05(3)	0.03(3)
M13	-0.61(3)	-0.13(3)	-0.16(3)	-0.12(3)	-0.76(3)	0.04(3)	-0.21(3)
"	-0.51(3)	0.17(3)	0.16(3)	0.54(3)	-0.12(3)	0.05(3)	0.01(3)
M14	0.07(3)	-0.00(3)	-0.00(3)	0.01(3)	0.01(3)	0.20(3)	0.02(3)
"	0.07(3)	-0.00(3)	0.00(3)	0.01(3)	0.01(3)	0.20(3)	0.04(3)
M15	0.13(3)	0.00(3)	0.00(3)	0.02(3)	0.02(3)	0.17(3)	0.02(3)
"	0.13(3)	0.00(3)	0.00(3)	0.02(3)	0.02(3)	0.17(3)	0.04(3)
M16	0.23(3)	0.00(3)	0.00(3)	0.04(3)	0.04(3)	0.10(3)	0.02(3)
"	0.23(3)	0.00(3)	0.00(3)	0.04(3)	0.04(3)	0.11(3)	0.08(3)
M17	-0.75(3)	0.00(3)	0.00(3)	-0.14(3)	-0.14(3)	0.09(3)	-0.17(3)
"	-0.75(3)	0.00(3)	0.00(3)	-0.14(3)	-0.14(3)	0.10(3)	-0.15(3)
M18	-0.58(3)	-0.00(3)	-0.00(3)	-0.11(3)	-0.11(3)	0.12(3)	-0.11(3)
"	-0.58(3)	-0.00(3)	0.00(3)	-0.11(3)	-0.11(3)	0.13(3)	-0.06(3)
M19	-1.23(3)	0.00(3)	0.00(3)	-0.23(3)	-0.23(3)	0.02(3)	-0.09(3)
"	-1.23(3)	0.00(3)	0.00(3)	-0.23(3)	-0.23(3)	0.05(3)	0.01(3)
M20	-0.47(3)	-0.00(3)	-0.00(3)	-0.09(3)	-0.09(3)	0.00(3)	0.03(3)
"	-0.47(3)	-0.00(3)	0.00(3)	-0.09(3)	-0.09(3)	0.00(3)	0.04(3)
M21	-1.25(3)	-0.00(3)	-0.00(3)	-0.24(3)	-0.24(3)	0.01(3)	-0.12(3)
"	-1.25(3)	-0.00(3)	0.00(3)	-0.24(3)	-0.24(3)	0.03(3)	0.00(3)
M22	0.05(3)	0.00(3)	0.00(3)	0.01(3)	0.01(3)	0.12(3)	0.01(3)
"	0.05(3)	0.00(3)	0.00(3)	0.01(3)	0.01(3)	0.12(3)	0.02(3)

BENDING & COMP: TRUSS 1; MEMBER 8

Buckling Factor, C_T is
neglected due to small contribution

Grading:

2x or 4x Doug-fir larch: No. 2

Assumptions:

Lateral support at points of bearing
SPS or gypboard attached to compression face
Maximum center-center spacing = 24"

Width, b	1.5 inches
Depth, d	3.5 inches
Length	7.38 feet
Max Axial Comp, C	1670 lbs
Max Reaction, R	210 lbs
Max Moment, M	230 ft-lbs
Max LL Deflection	0.19 inches
Max TL Deflection	0.38 inches
LL Defl Criteria = $L/$	240
TL Defl Criteria = $L/$	180
Duration factor, C_d	1.25
Repetitive Factor, C_r	1.15
$f_c =$	318 psi
$F_{ce} =$	1171 psi
$F_c^* =$	1094 psi
$F'_c =$	781 psi
$f_b =$	75 psi
$F'_b =$	1258 psi
Shear D/C ratio	$0.51 < 1.0$, Member OK
Interaction equation: $(f_c/F'_c)^2 +$	
$f_b/(F'_b(1-f_c/F_{ce})) =$	$0.25 < 1.0$, Member OK
Live Load defl ratio	$0.51 < 1.0$, Member OK
Total Load defl ratio	$0.77 < 1.0$, Member OK