

CITY OF SACRAMENTO

Permit No: 9805276

1231 I Street, Sacramento, CA 95814

Insp Area: 2

Site Address: 6331 SEASTONE WY SAC

Sub-Type: RES

Parcel No: 0300412002

Housing (Y/N): N

CONTRACTOR

ZIMMERMAN ROOFING
3560 RAMONA AV
SACRAMENTO, CA

95826

OWNER

KOBAYASHI STANLEY T/VIOLET Y
6331 SEASTONE WY
SACRAMENTO CA

95831

ARCHITECT

Nature of Work: T/O AND REROOF32 SQS WITH TILE

CONSTRUCTION LENDING AGENCY: I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name Lender's Address

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class C-39 License Number 557559 Date 6-17-98 Contractor Signature Kelly Coy

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. B & PC for this reason:

Date Owner Signature

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 6-17-98 Applicant/Agent Signature Kelly Coy

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

X I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier State Fund Policy Number 713 970002021

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 6-17-98 Applicant Signature Kelly Coy

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

Kabowashi addendum

Permit - 98-05276

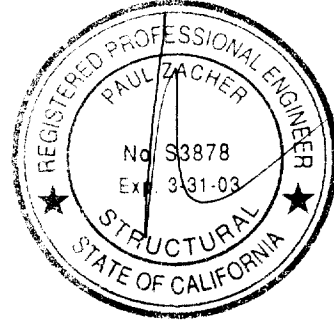
Paul Zacher - Structural Engineers
4701 Lakeside Way
Fair Oaks, CA 95628

TEL: 916.961.3960
FAX: 916.961.6552

June 14, 2001

ADDENDUM

Zimmerman Roofing
3675 R Street
Sacramento, CA 95816
TEL: 916 454 3667
FAX: 916 455 3784



Attn: Mr. Jeff Tucker.

re: Job 98096 KABOWASHI

Subject: ADDENDUM to the Structural Investigation Report of the Roof for the Residence located at 6331 Seastone Way, Sacramento, CA 95831.

Per Mr. Jeff Tucker's request, the following items are addressed:

- Item 1: The 2x12 DF#2 x 12'-0" long purlin at the front of the residence that was to have been scabbed to the 2x4 purlin which spans 12'-0" may be replaced with 2x4 struts from the existing purlins to the bearing walls below
- Item 2: The 1/2" OSB gusset plate at the front of the residence may be replaced with 2x4s scabbed to the existing rafters with 16d's @ 12" on center (total 10).
- Item 3: Two of four the collar ties to be added were relocated to the middle of the residence where the rafters were doubled in item 1 above.
- Item 4: Only 3 full-length rafters in the garage require 2x6's scabbed to the side.

ISSUED

JUN 14 2001

SACRAMENTO, CALIF.

If you have any questions on the above, do not hesitate to call.

Sincerely,

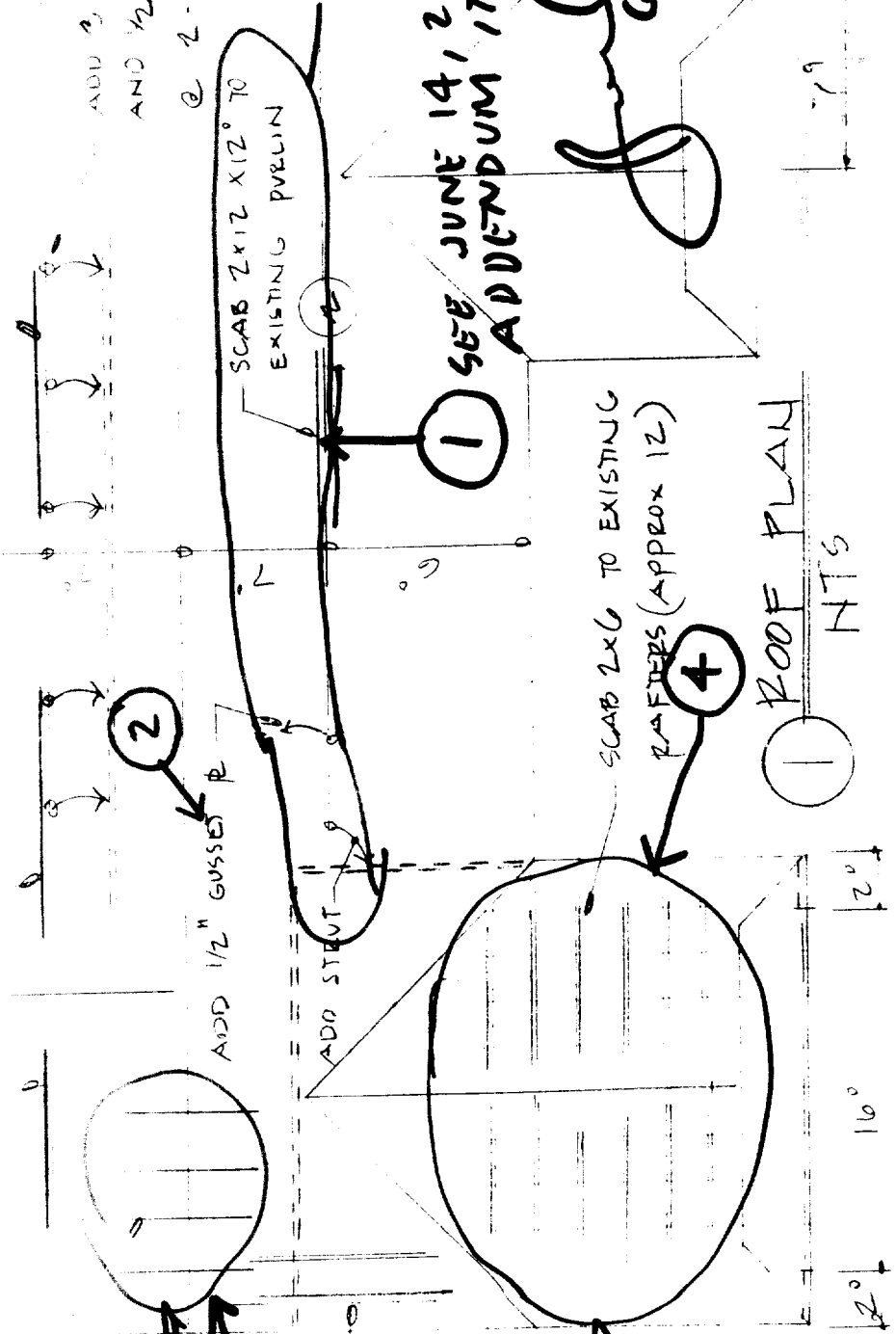
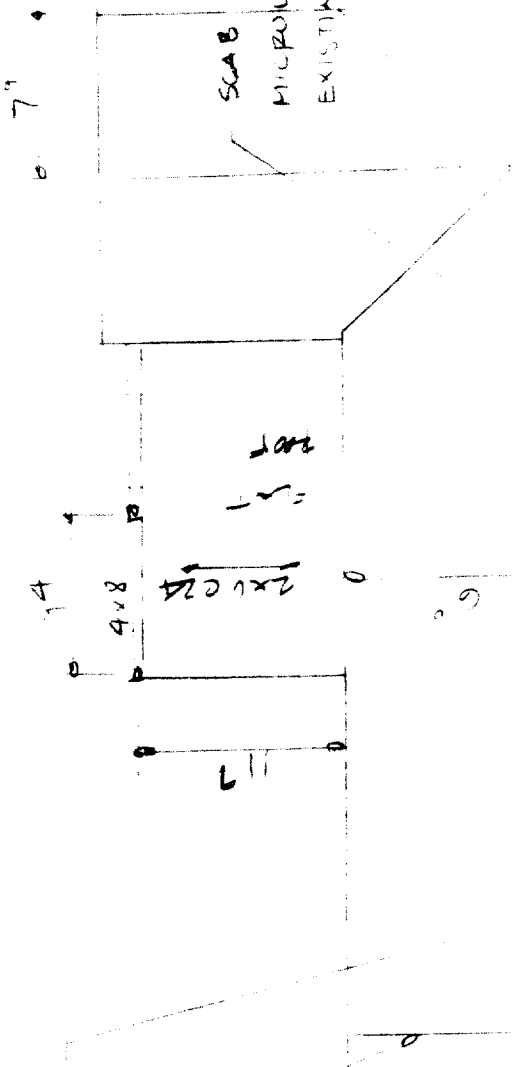
Paul Zacher, P.E., S.E.
file



This set of plans and specifications must be kept on the job at all times and it is unlawful to make any changes or alterations from the same without written permission from the Building Inspection Division.

The approval of this plan and specification SHALL NOT be held to permit or approve the violation of any City Ordinance or State Law.

Paul Zacher
6/14/01



SCAB 2x12 x 12' TO EXISTING PURLIN (2)

SCAB 2x4 TO EXISTING RAFTER

SCAB 1x8 x 8' TO EXISTING PURLIN (2)

ADD 2x6 COLLAR TIES (TOTAL 4)
only two are shown (3)

SCAB 2x4 TO EXISTING RAFTER (TOT 2)

only six are shown.



ROOF PLAN
 HTS

SEE JUNE 14, 2001
 ADDENDUM ITEMS, MP.
6/19/01

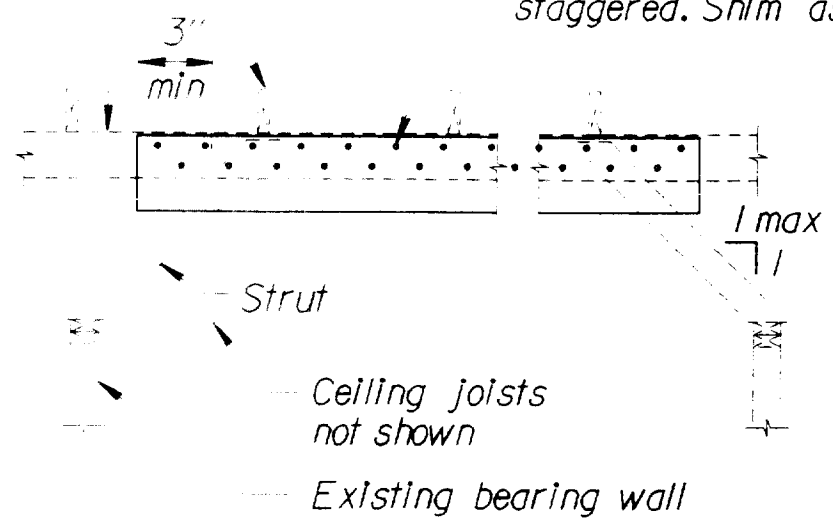
not installed

18588

Existing rafters ---

Existing purlin ---

Purlin Nail to existing purlin w/ 16d @ 3" oc, staggered. Shim as required.



2

PURLIN DETAIL

Add 2x4 shim as req'd w/ 16d @ 8" oc to existing member

Existing rafter

Existing ceiling joist

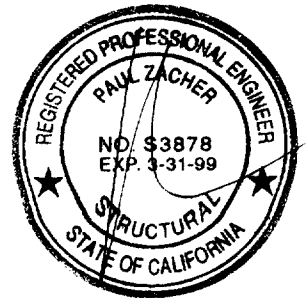
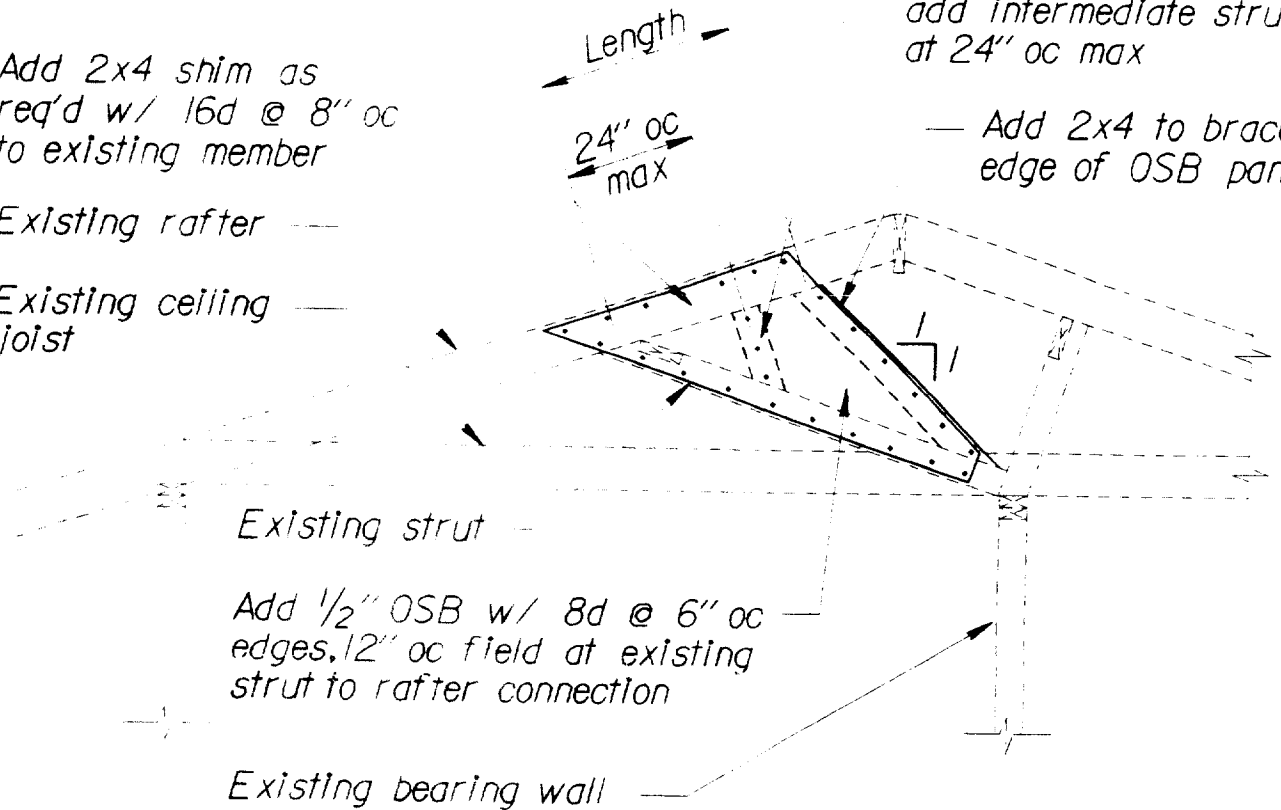
Existing strut

Add 1/2" OSB w/ 8d @ 6" oc edges, 12" oc field at existing strut to rafter connection

Existing bearing wall

If "Length" of panel is greater than 24", add intermediate struts at 24" oc max

— Add 2x4 to brace edge of OSB panel



3

GUSSET PLATE DETAIL

1/2" = 1'-0"

Paul Zacher-Structural Engineer
4701 Lakeside Way
Fair Oaks, CA 95628
TEL: 916.961.3960
FAX: 916.961.3960

June 9, 1998

Zimmerman Roofing
3560 Ramona Avenue
Sacramento, CA 95826
TEL: 916.454.3667
FAX: 916.455.3784
TEL (Jeff) 916.392.1971
FAX (Jeff): 916.392.6853
FAX (Framer) : 916.383.5308

ok
AT
6/16/98

Attn.: Mr. Jeff Tucker,

re: Job 98096: KABOWASHI

Subject: Structural Investigation Report of the Roof for the Residence located at 6331 Seastone Way, Sacramento, CA 95831

As requested by Mr. Jeff Tucker, this is a report to determine what needs should be addressed to correct any structural deficiencies of the roof. Paul Zacher visited the site June 1, 1998. The investigation was made to determine the existing condition of the structure. All information, data and analysis contained within this report is based on the 1994 Uniform Building Code

The following is based on visual observations with no subsurface investigation being made.

DESCRIPTION:

Type of Facility: Residence.
Year Built: Estimated 1970's vintage.
Occupancy: Residential.
No. of Stories: One.
Dimensions: Approximately 1600 square feet with a first story plate height of 8 feet.

CONSTRUCTION:

Roof:

The roof covering will consist of Pioneer Shake Tile over 1/2" solid sheathing. The living area is conventionally framed with 2x4 rafters spaced at 24" on center with 2x4 purlins supported at no more than 14'-0" on center by 2x4 struts bearing on walls below. The garage area is framed with 2x4 rafters spaced at 24" on center and 2x6 cross ties spaced at 4'-0" on center

CONCLUSIONS:

Roof:

The living and garage areas lack sufficient structural capacity for the applied live and dead loads

RECOMMENDATIONS:

If any of the following recommendations do not correspond to actual field conditions, the engineer of record shall be notified for further investigation and evaluation before continuing work.

Living Area.

1. Scab a 2x8 DF#2 x 8'-0" long purlin adjacent to the existing 2x4 purlin which spans 8'-0". Attach it with 16d's @ 3" on center. Support the 2x8 to the bearing walls below with 2x4 struts. See detail 2.
2. Scab a 2x12 DF#2 x 12'-0" long purlin adjacent to the existing 2x4 purlin which spans 12'-0". Attach it with 16d's @ 3" on center. Support the 2x12 to the bearing walls below with 2x4 struts. See detail 2.
3. Scab a 1 3/4" x 9 1/2" microlam x 14'-0" long purlin adjacent to the existing 2x4 purlin which spans 14'-0". Attach it with 16d's @ 3" on center. Support the microlam to the bearing walls below with 2x4 struts. See detail 2.
4. Where the minimum slope of the struts is less than 45 degrees from the horizontal add a 1/2" OSB gusset plate adjacent to each existing strut and rafter connection and attach it with 8d's at 6" on center at the edges. See detail 3.
5. Provide additional 2x4 struts from the existing purlins to the bearing walls below. The maximum spacing between the new and existing struts shall not exceed 4'-0" on center. The unbraced length of the struts shall not exceed 8'-0" and the minimum slope of the struts shall not be less than 45 degrees from the horizontal.
6. Add 2x6 collar ties at 48" on center to tie the existing rafters together. Nail the collar ties to the existing rafter with 4 -16d's at each connection. Place the collar tie as close to the existing ceiling joists as possible.

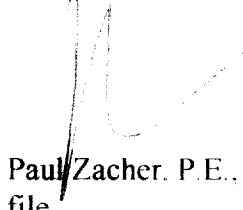
Garage Truss.

7. Scab a 2x4 rafter adjacent to the existing 2x4 rafters where the span is greater than 7'-9" with 16d's @ 12" on center.
8. Scab a 2x6 rafter adjacent to the existing 2x4 rafters where the span is greater than 11'-9" with 16d's @ 12" on center.

The inspection consisted of visual observation only, made solely to determine the structural capacity of the existing roof. Analysis does not determine any effects on the overall structure under lateral forces or effects on the foundation unless specifically noted in the calculations and in this document. No warranties, expressed or implied, are made or intended in conjunction with this report. The inspection was made only to the portions that were accessible. The specific items noted were those that were observable and there may be defects which are not observable, or are hidden by architectural and structural materials.

If you have any questions on the above, do not hesitate to call.

Sincerely,



Paul Zacher, P.E., S.E.
file



DESIGN LOADING:

Roof Pitch	4	in 12
Pitch Adjustment Factor	1.05	

LOCATION: ROOF

<u>MATERIAL</u>	<u>WEIGHT</u>	
Pioneer Standard Wt	10.00	psf
Roofing felt	0.30	psf
1x4 skip sht'g	1.09	psf
1/2" OSB/ plywood	1.50	psf
2x4 rafters @ 24" oc	<u>0.64</u>	psf
	Load	13.5 psf
Roof Pitch Adjustment	<u>0.73</u>	psf
Total Load	14.3	psf

BEAM DESIGN FOR UNIFORM LOAD: 2x4

(Values for DF Larch #2)

Width, b	1.5 inches
Depth, d	3.5 inches
Length of beam	7.75 feet
Dead load roof	14.3 psf
Live load roof	16 psf
Contributory width of roof load	2 feet
Dead load floor	0 psf
Live load floor	0 psf
Contributory width of floor load	0 feet
Dead load wall	0 plf
Live load defl ratio	240
Toal load defl ratio	180
Total dead load	28.6 plf
Total live load	32 plf

Base design values:

Shear, Fv	95 psi
Bending, Fb	1000 psi
Comp. perp. to grain, Fc	625 psi
Mod of Elasticity, E	1700000 psi
Load duration factor, Cd	1.25
Size Factor, Cf	1.50
Repetitive factor, Cr	1.15

Dead load reaction	111 lbs
Live load reaction	124 lbs
Total load reaction	235 lbs

Allowable shear, Fv'	119 psi
Actual shear, fv	62 psi
Allowable bending, Fb'	2156 psi
Actual bending, fb	1783 psi
Allowable live load defl	0.39 inches
Actual live load defl	0.29 inches
Allowable total load defl	0.52 inches
Actual total load defl	0.54 inches

Horizontal Shear OK

Bending OK

Live Load Deflection OK

Beam Fails under Total Load Deflection
OK, Less than 1/32" over

Bearing length req'd	0.25 inches
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BEAM DESIGN FOR UNIFORM LOAD: 2-2x4

(Values for DF Larch #2)

Width, b	3 inches
Depth, d	3.5 inches
Length of beam	9.75 feet
Dead load roof	14.3 psf
Live load roof	16 psf
Contributory width of roof load	2 feet
Dead load floor	0 psf
Live load floor	0 psf
Contributory width of floor load	0 feet
Dead load wall	0 plf
Live load defl ratio	240
Toal load defl ratio	180
Total dead load	28.6 plf
Total live load	32 plf

Base design values:

Shear, F_v	95 psi
Bending, F_b	1000 psi
Comp. perp. to grain, F_c	625 psi
Mod of Elasticity, E	1700000 psi
Load duration factor, C_d	1.25
Size Factor, C_f	1.50
Repetitive factor, C_r	1.15

Dead load reaction	139 lbs
Live load reaction	156 lbs
Total load reaction	295 lbs

Allowable shear, F_v'	119 psi
Actual shear, f_v	40 psi
Allowable bending, F_b'	2156 psi
Actual bending, f_b	1411 psi
Allowable live load defl	0.49 inches
Actual live load defl	0.36 inches
Allowable total load defl	0.65 inches
Actual total load defl	0.68 inches
Bearing length req'd	0.16 inches

Horizontal Shear OK

Bending OK

Live Load Deflection OK

Beam Fails under Total Load Deflection
OK, Less than 1/32" over

BEAM DESIGN FOR UNIFORM LOAD: 2x6

(Values for DF Larch #2)

Width, b	1.5 inches
Depth, d	5.5 inches
Length of beam	11.75 feet
Dead load roof	14.3 psf
Live load roof	16 psf
Contributory width of roof load	2 feet
Dead load floor	0 psf
Live load floor	0 psf
Contributory width of floor load	0 feet
Dead load wall	0 plf
Live load defl ratio	240
Total load defl ratio	180
Total dead load	28.6 plf
Total live load	32 plf

Base design values:

Shear, F_v	95 psi
Bending, F_b	1000 psi
Comp. perp. to grain, F_c	625 psi
Mod of Elasticity, E	1600000 psi
Load duration factor, C_d	1.25
Size Factor, C_f	1.30
Repetitive factor, C_r	1.15

Dead load reaction	168 lbs
Live load reaction	188 lbs
Total load reaction	356 lbs

Allowable shear, F_v'	119 psi
Actual shear, f_v	60 psi
Allowable bending, F_b'	1869 psi
Actual bending, f_b	1659 psi
Allowable live load defl	0.59 inches
Actual live load defl	0.41 inches
Allowable total load defl	0.78 inches
Actual total load defl	0.78 inches

Bearing length req'd	0.38 inches
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Horizontal Shear OK

Bending OK

Live Load Deflection OK

Total Load Deflection OK

BEAM DESIGN FOR UNIFORM LOAD: 2x8 purlin

(Values for DF Larch #2)

Width, b	1.5 inches
Depth, d	7.25 inches
Length of beam	8 feet
Dead load roof	14.3 psf
Live load roof	16 psf
Contributory width of roof load	6.5 feet
Dead load floor	0 psf
Live load floor	0 psf
Contributory width of floor load	0 feet
Dead load wall	0 plf
Live load defl ratio	240
Toal load defl ratio	180
Total dead load	92.95 plf
Total live load	104 plf

Base design values:

Shear, Fv	95 psi
Bending, Fb	1000 psi
Comp. perp. to grain, Fc	625 psi
Mod of Elasticity, E	1600000 psi
Load duration factor, Cd	1.25
Size Factor, Cf	1.20
Repetitive factor, Cr	1.15

Dead load reaction	372 lbs
Live load reaction	416 lbs
Total load reaction	788 lbs

Allowable shear, Fv'	119 psi
Actual shear, fv	92 psi
Allowable bending, Fb'	1725 psi
Actual bending, fb	1439 psi
Allowable live load defl	0.40 inches
Actual live load defl	0.13 inches
Allowable total load defl	0.53 inches
Actual total load defl	0.24 inches

Bearing length req'd	0.84 inches
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Horizontal Shear OK

Bending OK

Live Load Deflection OK

Total Load Deflection OK

BEAM DESIGN FOR UNIFORM LOAD:

(Values for DF Larch #1)

Width, b	3.5 inches
Depth, d	7.25 inches
Length of beam	9.33 feet
Dead load roof	14.3 psf
Live load roof	16 psf
Contributory width of roof load	8 feet
Dead load floor	0 psf
Live load floor	0 psf
Contributory width of floor load	0 feet
Dead load wall	0 plf
Live load defl ratio	360
Toal load defl ratio	240
Total dead load	114.4 plf
Total live load	128 plf

Base design values:

Shear, Fv	95 psi
Bending, Fb	1000 psi
Comp. perp. to grain, Fc	625 psi
Mod of Elasticity, E	1700000 psi
Load duration factor, Cd	1.25
Size Factor, Cf	1.30

Dead load reaction	534 lbs
Live load reaction	597 lbs
Total load reaction	1131 lbs

Allowable shear, Fv'	119 psi	Horizontal Shear	OK
Actual shear, fv	58 psi		
Allowable bending, Fb'	1625 psi	Bending	OK
Actual bending, fb	1032 psi		
Allowable live load defl	0.31 inches	Live Load Deflection	OK
Actual live load defl	0.12 inches		
Allowable total load defl	0.47 inches	Total Load Deflection	OK
Actual total load defl	0.22 inches		

Bearing length req'd 0.52 inches

Permit # 98-05274

Paul Zacher-Structural Engineers

4701 Lakeside Way
Fair Oaks, CA 95628

TEL: 916.961.3960
FAX: 916.961.3960
e-mail: pzacher@softcom.net

July 21, 1998

Zimmerman Roofing
3560 Ramona Avenue
Sacramento, CA 95826
TEL: 916.454.3667
FAX: 916.455.3784
TEL (Jeff): 916.392.1971
FAX (Jeff): 916.392.6853
FAX (Framer) 916.383.5308

826-3020

Attn.: Mr. Jeff Tucker,

re: Job 98096: KABOWASHI Residence located at 6331 Seastone Way, Sacramento, CA 95831.

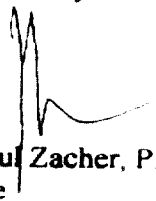
Subject: Contract Change Order Number 01

The front portion of the residence may be framed as follows and will have adequate structural strength to support dead and live loads:

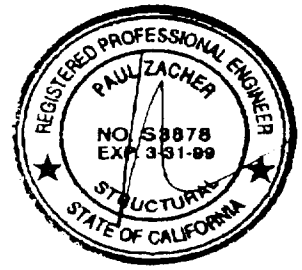
1. The 2x12 DF#2 x 12'-0" long purlin that was to have been scabbed to the 2x4 purlin which spans 12'-0" may be replaced with 2x4 struts from the existing purlins to the bearing walls below.
2. The 1/2" OSB gusset plate and additional 2x4 strut may be replaced with 2x4s scabbed to the existing rafters with 16d's @ 12" on center.

If you have any questions on the above, do not hesitate to call.

Sincerely,



Paul Zacher, P.E., S.E.
file



From: Stanley T. Kobayashi
6331 Seastone Way
Sacramento, CA. 95831

July 29, 1998

To: City of Sacramento
Building Inspection Division
1231 I Street, Room 200
Sacramento, CA. 95814

STANLEY T. KOBAYASHI

JUL 31 1998

STANLEY T. KOBAYASHI

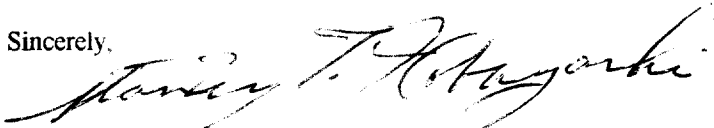
Dear Sir:

Ref : Correction Notice for Permit No. 98-05276R, Area No. 2R

As requested by Mr. Jeff Tucker (Zimmerman Roofing), I am submitting the attached change authorized by Mr. Paul Zacher (structural engineer) for your approval for the re-roofing recently completed on my home. Per Mr. Tucker and carpenters of R. Parsons Construction, the accomplished changes are structurally better than that shown on the original submission. The contents of the Change Order Number 01 should meet the requirements of the Correction Notice, 17 July 1998.

Thank you very much.

Sincerely,



Attachments: Contract Change Order Number 01, 21 July 1998
Correction Notice, Permit No. 98-05276R, Area 2R, 17 July 1998