

CITY OF SACRAMENTO

Permit No: 9802323

1231 I Street, Sacramento, CA 95814

Insp Area: 4

Site Address: 1638 VALLARTA CR SAC

Sub-Type: RES

Parcel No: 2250890061

Housing (Y/N): N

CONTRACTOR

D H CONSTRUCTION
4324 ORANGE GROVE AV
SACRAMENTO CA
Phone: 916-481-3297

95841

OWNER

ANDERSON THERESA
1638 VALLARTA CR
SACRAMENTO CA
Phone:

95834

ARCHITECT

Phone:

Nature of Work: FIRE REPAIR [SUBSTANTIAL] TO PRE-FIRE CONDITION NO ALTERATION TO FLOOR PLA

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name

Lender's Address

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class B-1

License Number 362161

Date 3/24/98

Contractor Signature

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his or her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he or she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. B & PC for this reason:

Date

Owner Signature

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

Date 3/24/98

Applicant/Agent Signature

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier State Fund

Policy Number 046-95141/0004903

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 3/24/98

Applicant Signature

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

Sacramento Fire Department - Incident Report

Incident No : 980003810 Call# : 98008220 Date: 01/26/98 Time: 2:55
Address : 1638 VALLARTA CR
Type : 11 BUILDING FIRE
Action Taken: 15 EXTINGUISHED
Property : 1-2 FAMILY RESIDENTIAL: SINGLE FAMILY
UBC : DWELLINGS AND LODGING HOUSES

Weather : 45 Degrees / Clear
Resources : 1 Engine
1 Other Apparatus

Fire Casualties : None

Fire Damage : Confined to structure of origin
Smoke Damage : Confined to structure of origin
Property Loss : \$100,000 Contents Loss :
Property Value : \$135,000 Contents Value:

Area of Origin : Lounge area
Caused by : No equipment involved
Form of Heat : Undetermined
Ignition Factor : Suspicious
Type of Material : Undetermined
Form of Material : Undetermined
Type of Material : Undetermined
Form of Material : Undetermined

Level: A01

Other Factors : Acts or Omissions Insufficient information
Delays Insufficient information
Extinguished by : Water from hydrant, draft, standpipe
Structure Type : Building with one specific property use
Structure Status : In use
Not occupied
Construction Type: Type V - Wood Frame
Roof Type : Wood shake - treated
Number of Stories: 2

Detector Type : Smoke detector - photoelectric
Power : Hard Wire
Performance : Undetermined/not reported
Reason Failed : No failure

Extinguishing Sys: No extinguishing system

Report Author : F263

C6E①

C④

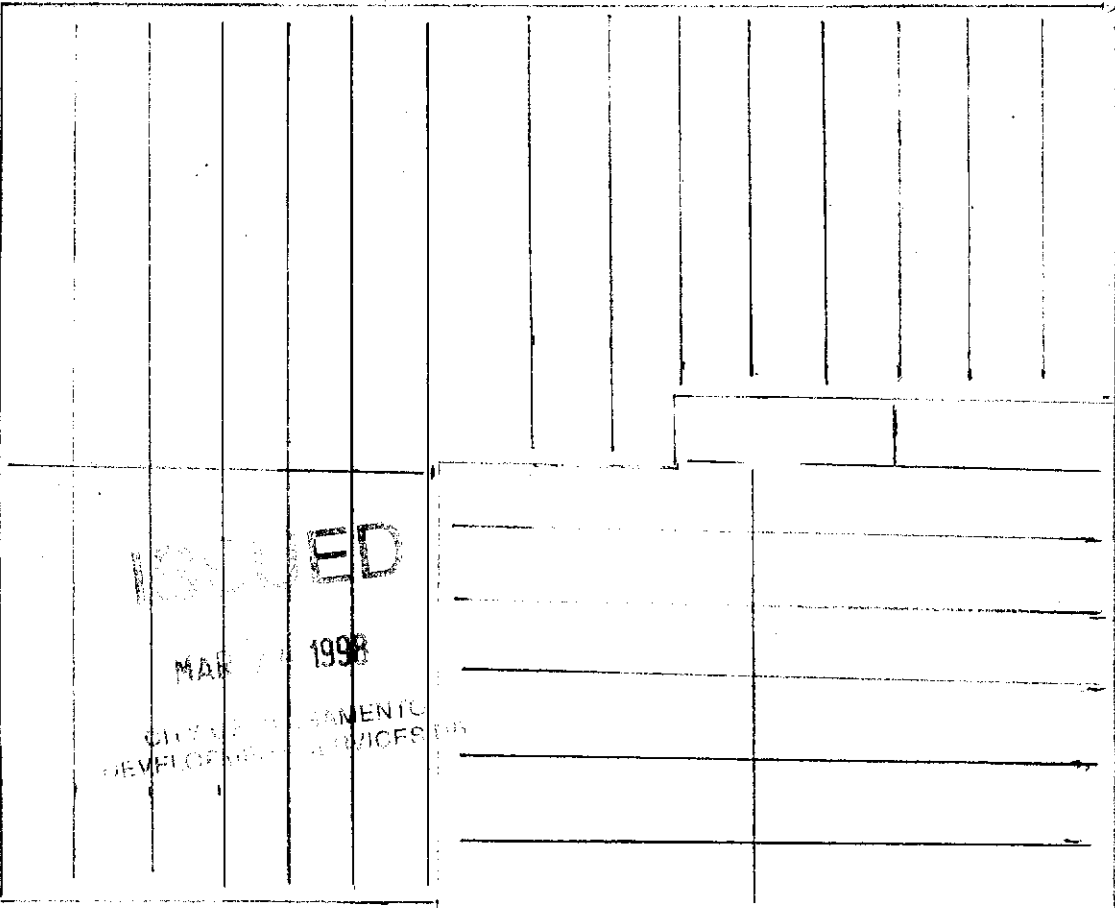
D②

E④

E6E①

30'3"

24'-8"



ISSUED
 MAR 7 1998
 CIVIL ENGINEERING SERVICES

B6E①
B①

A1④

A1GIR①

54'

↑
ENTRY

AGIR①

TRUSSES 2' OAC

A③

OK
AT
3/24/98

* ANDERSON
VALLARTA CIN
1638

A2①

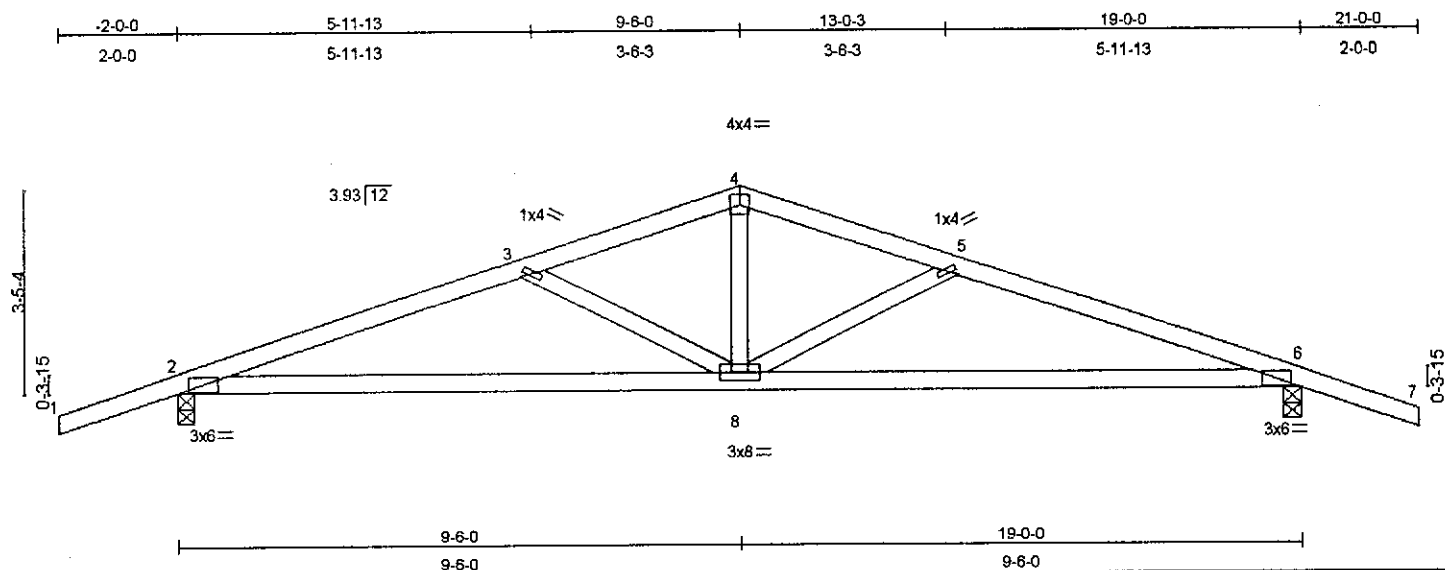
AGE①

20'3"

Job	Truss	Truss Type	Qty	Ply	
DHC	A	QUEENPOST	3	1	

GENERAL TRUSS CO., SACRAMENTO, CA 95828

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LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 20.0	Plates Increase 1.00	TC 0.25	Vert(LL) -0.19 2-8 >999	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.70	Vert(TL) -0.34 2-8 >657	
BCLL 0.0	Rep Stress Incr YES	WB 0.20	Horz(TL) 0.04 6 n/a	
BCDL 7.0	Code UBC/ICBO		Min Length / LL defl = 360	Weight: 73 lb

LUMBER

TOP CHORD 2 X 4 DF No.1&Btr-G
 BOT CHORD 2 X 4 DF No.1&Btr-G
 WEBS 2 X 4 DF Std-G

BRACING

TOP CHORD Sheathed or 4-11-3 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=913/0-3-8, 6=913/0-3-8

FORCES (lb) - First Load Case Only

TOP CHORD 1-2=20, 2-3=-1579, 3-4=-1203, 4-5=-1203, 5-6=-1579, 6-7=20
 BOT CHORD 2-8=1495, 6-8=1495
 WEBS 3-8=-396, 4-8=497, 5-8=-396

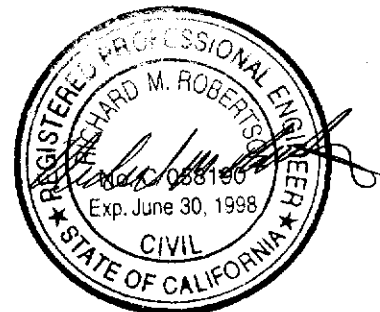
NOTES

- 1) This truss has been checked for unbalanced loading conditions.
- 2) All plates are M20 plates unless otherwise indicated.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
- 4) A plate rating reduction of 20% has been applied for the green lumber members.
- 5) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is **always** required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

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Job	Truss	Truss Type	Qty	Ply	
DHC	A1	QUEENPOST	4	1	

GENERAL TRUSS CO., SACRAMENTO, CA 95828

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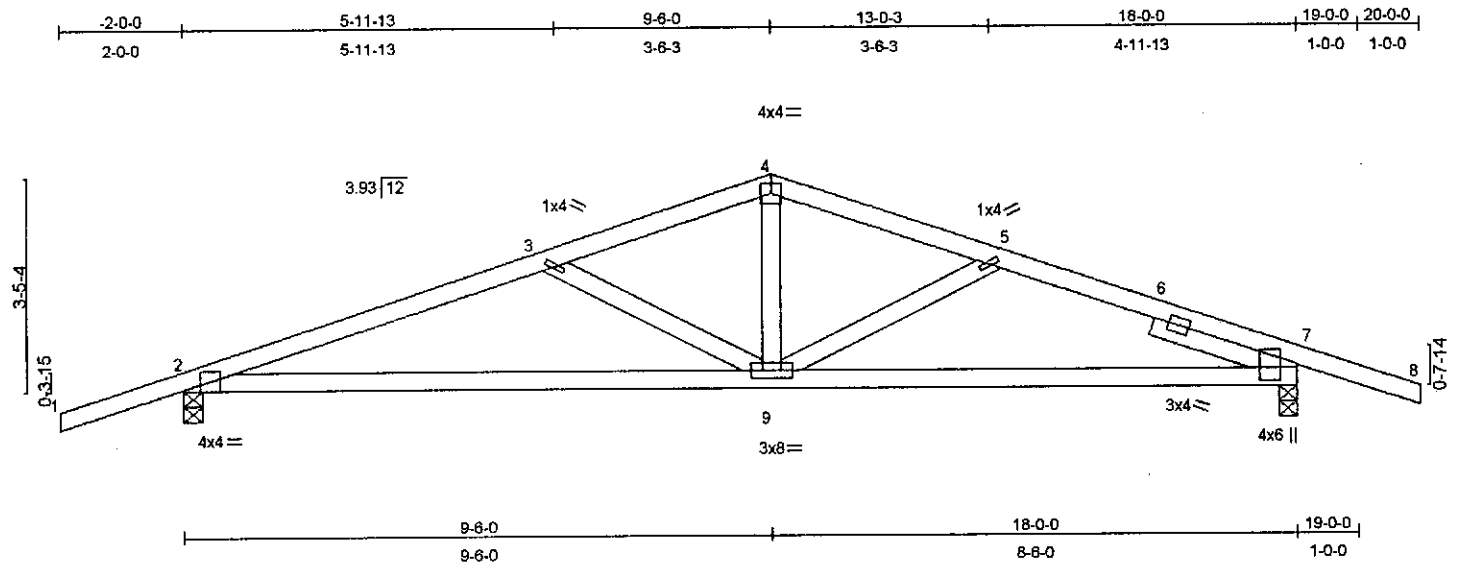


Plate Offsets (X,Y): [2:0-3-3,edge], [7:0-3-1,0-3-3]

LOADING (psf)	SPACING	CSI	DEFL (in)	(loc)	l/defl	PLATES GRIP
TCLL 20.0	Plates Increase 1.00	TC 0.26	Vert(LL) -0.19	2-9	>999	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.63	Vert(TL) -0.33	2-9	>640	
BCLL 0.0	Rep Stress Incr YES	WB 0.17	Horz(TL) 0.03	7	n/a	
BCDL 7.0	Code UBC/ICBO		Min Length / LL defl = 360			Weight: 74 lb

LUMBER

TOP CHORD 2 X 4 DF No.1&Btr-G
 BOT CHORD 2 X 4 DF No.1&Btr-G
 WEBS 2 X 4 DF Std-G
 SLIDER Right 2 X 4 DF Std 2-5-5

BRACING

TOP CHORD Sheathed or 5-1-7 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=878/0-3-8, 7=868/0-3-8

FORCES (lb) - First Load Case Only

TOP CHORD 1-2=20, 2-3=-1470, 3-4=-1093, 4-5=-1093, 5-6=-1374, 6-7=-1374, 7-8=10
 BOT CHORD 2-9=1392, 7-9=1269
 WEBS 3-9=-398, 4-9=428, 5-9=-259

NOTES

- 1) This truss has been checked for unbalanced loading conditions.
- 2) All plates are M20 plates unless otherwise indicated.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
- 4) A plate rating reduction of 20% has been applied for the green lumber members.
- 5) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	
DHC	A1GIR	QUEENPOST	1	2	

GENERAL TRUSS CO., SACRAMENTO, CA 95828

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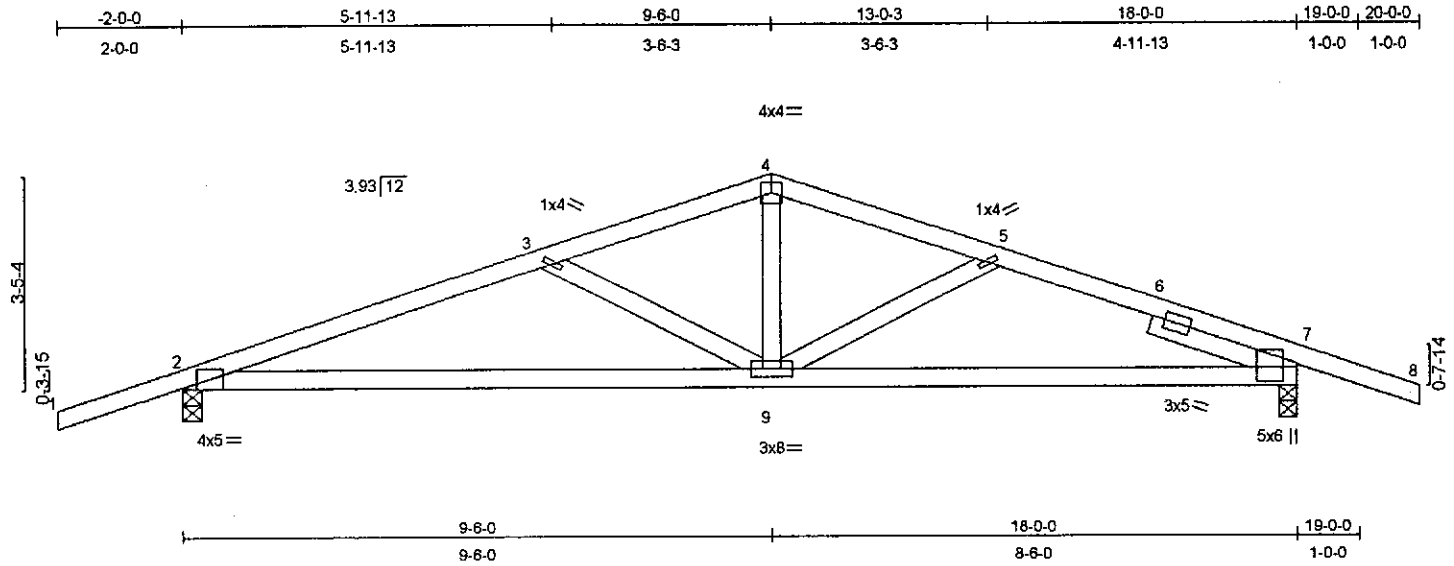


Plate Offsets (X,Y): [2:0-2-11,edge], [7:0-2-9,0-3-7]					
LOADING (psf)	SPACING 5-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP	
TCLL 20.0	Plates Increase 1.00	TC 0.38	Vert(LL) -0.24 2-9 >910	M20 186/148	
TCDL 14.0	Lumber Increase 1.25	BC 0.88	Vert(TL) -0.42 2-9 >512		
BCLL 0.0	Rep Stress Incr NO	WB 0.22	Horz(TL) 0.04 7 n/a		
BCDL 7.0	Code UBC/CBO		Min Length / LL defl = 360		Weight: 148 lb

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr-G
BOT CHORD 2 X 4 DF No.1&Btr-G
WEBS 2 X 4 DF Std-G
SLIDER Right 2 X 4 DF Std 2-5-5

BRACING
TOP CHORD Sheathed or 6-0-0 on center purlin spacing.
BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=2195/0-3-8, 7=2170/0-3-8

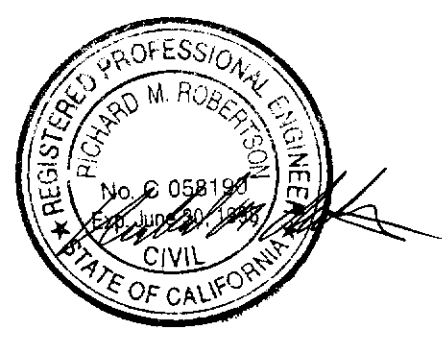
FORCES (lb) - First Load Case Only
TOP CHORD 1-2=51, 2-3=-3676, 3-4=-2733, 4-5=-2733, 5-6=-3436, 6-7=-3436, 7-8=25
BOT CHORD 2-9=3479, 7-9=3172
WEBS 3-9=-995, 4-9=1071, 5-9=-648

NOTES
1) 2-ply truss to be connected together with 10d Common(.148"x3") Nails as follows: Top chords connected with 1 row(s) at 0-9-0 on center. Bottom chords connected with 1 row(s) at 0-9-0 on center. Webs connected with 1 row(s) at 0-9-0 on center.
2) This truss has been checked for unbalanced loading conditions.
3) All plates are M20 plates unless otherwise indicated.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
5) A plate rating reduction of 20% has been applied for the green lumber members.
6) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	
DHC	A2	SCISSORS	7	1	

GENERAL TRUSS CO., SACRAMENTO, CA 95828

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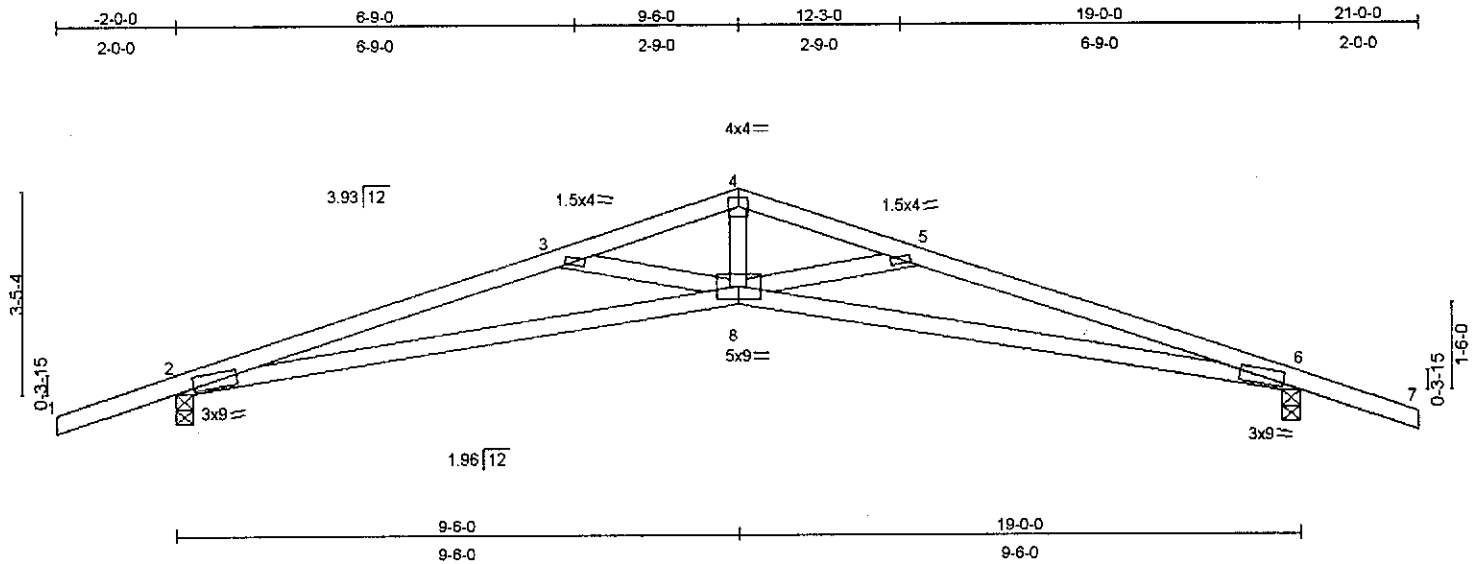


Plate Offsets (X,Y): [2-0-4-0,0-1-8], [6-0-4-0,0-1-8]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 20.0	Plates Increase 1.00	TC 0.47	Vert(LL) -0.40 6-8 >557	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.82	Vert(TL) -0.76 6-8 >297	
BCLL 0.0	Rep Stress Incr YES	WB 0.50	Horz(TL) 0.20 6 n/a	
BCDL 7.0	Code UBC/ICBO		Min Length / LL defl = 360	Weight: 69 lb

LUMBER
 TOP CHORD 2 X 4 DF No.1&Btr-G
 BOT CHORD 2 X 4 DF No.1&Btr-G
 WEBS 2 X 4 DF Std-G

BRACING
 TOP CHORD Sheathed or 3-4-4 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=913/0-3-8, 6=913/0-3-8

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=20, 2-3=-2872, 3-4=-2314, 4-5=-2314, 5-6=-2872, 6-7=20
 BOT CHORD 2-8=2753, 6-8=2753
 WEBS 4-8=1233, 3-8=-529, 5-8=-529

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) All plates are M20 plates unless otherwise indicated.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 4) A plate rating reduction of 20% has been applied for the green lumber members.
 - 5) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 6) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

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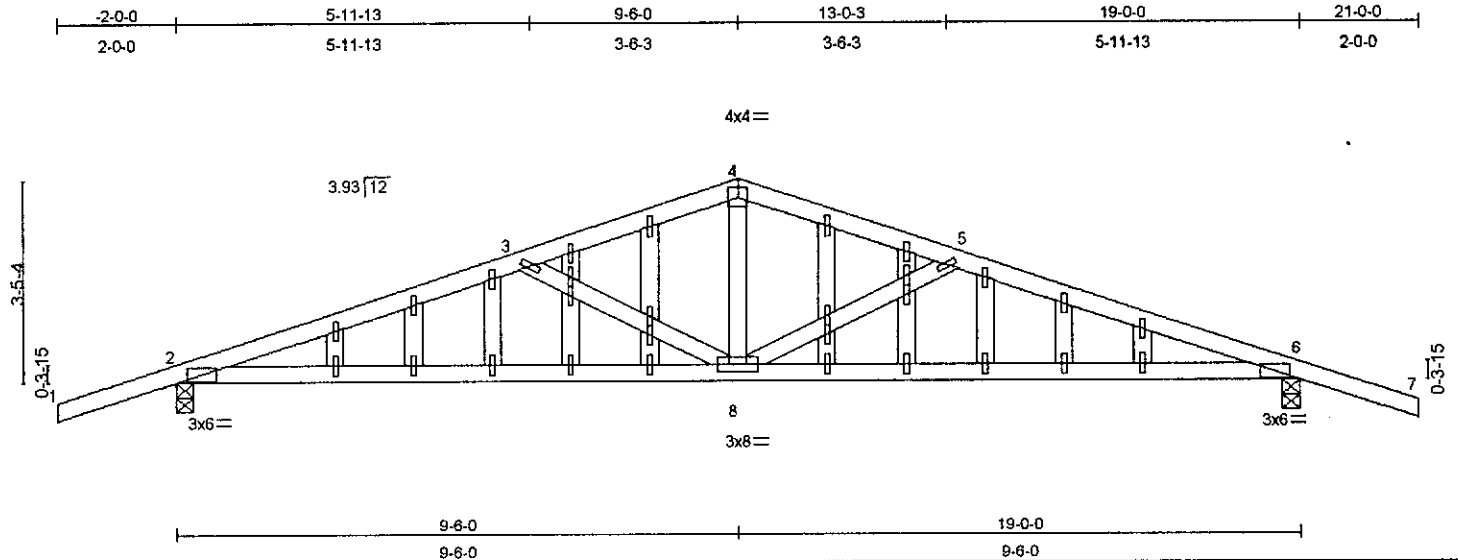


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Job	Truss	Truss Type	Qty	Ply	
DHC	AGE	QUEENPOST	1	1	

GENERAL TRUSS CO., SACRAMENTO, CA 95828

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LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 20.0	Plates Increase 1.00	TC 0.25	Vert(LL) -0.19 2-8 >999	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.70	Vert(TL) -0.34 2-8 >657	
BCLL 0.0	Rep Stress Incr YES	WB 0.20	Horz(TL) 0.04 6 n/a	
BCDL 7.0	Code UBC/CBO		Min Length / LL defl = 360	Weight: 93 lb

LUMBER

TOP CHORD 2 X 4 DF No.1&Btr-G
 BOT CHORD 2 X 4 DF No.1&Btr-G
 WEBS 2 X 4 DF Std-G
 OTHERS 2 X 4 DF Std-G

BRACING

TOP CHORD Sheathed or 4-11-3 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=913/0-3-8, 6=913/0-3-8

FORCES (lb) - First Load Case Only

TOP CHORD 1-2=20, 2-3=-1579, 3-4=-1203, 4-5=-1203, 5-6=-1579, 6-7=20
 BOT CHORD 2-8=1495, 6-8=1495
 WEBS 3-8=-396, 4-8=497, 5-8=-396

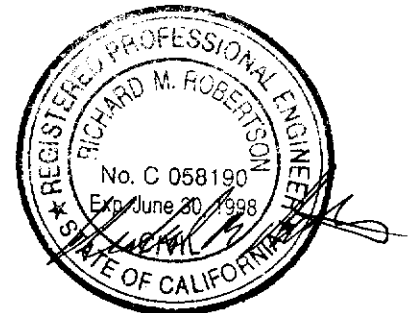
NOTES

- 1) This truss has been checked for unbalanced loading conditions.
- 2) All plates are M20 plates unless otherwise indicated.
- 3) All plates are 1x4 M20 unless otherwise indicated.
- 4) Gable studs spaced at 1-4-0 on center.
- 5) For studs exposed to wind, see MiTek "Standard Gable End Detail"
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
- 7) A plate rating reduction of 20% has been applied for the green lumber members.
- 8) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

MAR 05 1998

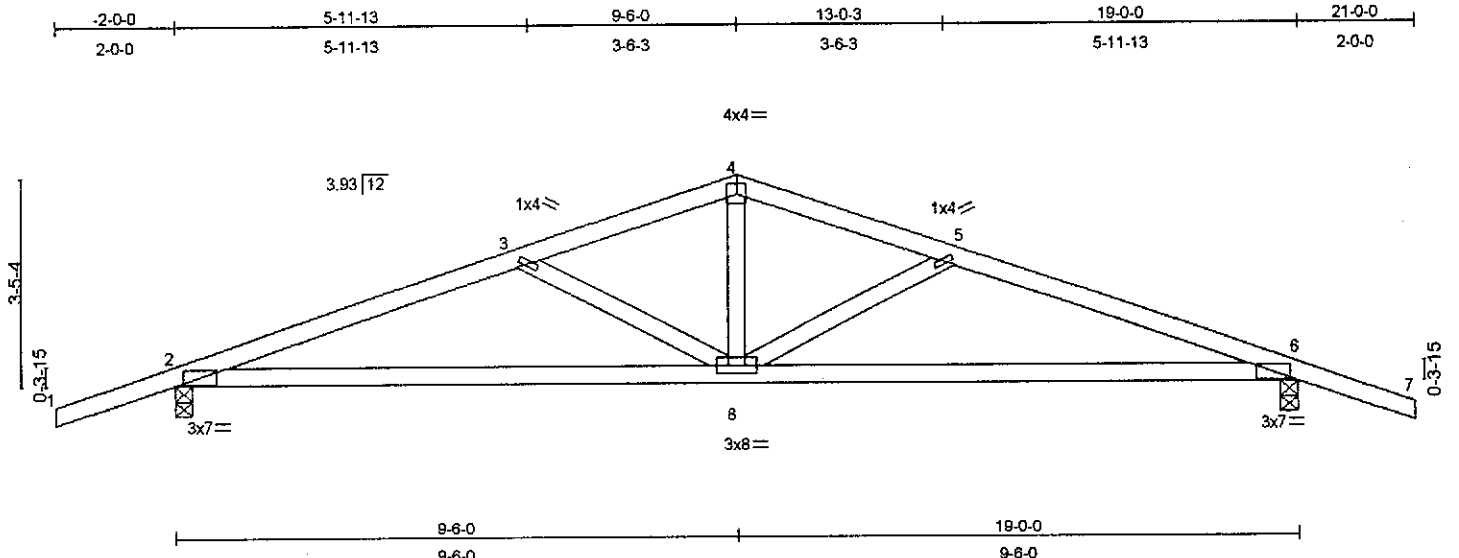
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Job	Truss	Truss Type	Qty	Ply	
DHC	AGIR	QUEENPOST	1	2	

GENERAL TRUSS CO., SACRAMENTO, CA 95828

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LOADING (psf)	SPACING 5-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 20.0	Plates Increase 1.00	TC 0.36	Vert(LL) -0.24 2-8 >948	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.98	Vert(TL) -0.43 2-8 >526	
BCLL 0.0	Rep Stress Incr NO	WB 0.25	Horz(TL) 0.04 6 n/a	
BCDL 7.0	Code UBC/ICBO		Min Length / LL defl = 360	Weight: 146 lb

LUMBER
 TOP CHORD 2 X 4 DF No.1&Btr-G
 BOT CHORD 2 X 4 DF No.1&Btr-G
 WEBS 2 X 4 DF Std-G

BRACING
 TOP CHORD Sheathed or 6-0-0 on center purfin spacing.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

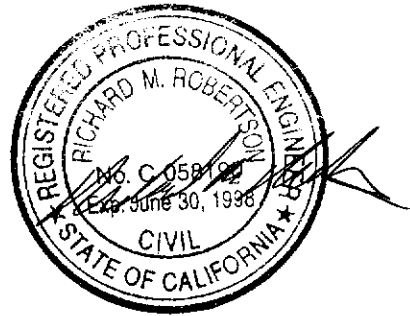
REACTIONS (lb/size) 2=2282/0-3-8, 6=2282/0-3-8

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=51, 2-3=-3947, 3-4=-3007, 4-5=-3007, 5-6=-3947, 6-7=51
 BOT CHORD 2-8=3736, 6-8=3736
 WEBS 3-8=-990, 4-8=1242, 5-8=-990

- NOTES**
- 1) 2-ply truss to be connected together with 10d Common(.148"x3") Nails as follows: Top chords connected with 1 row(s) at 0-9-0 on center. Bottom chords connected with 1 row(s) at 0-9-0 on center. Webs connected with 1 row(s) at 0-9-0 on center.
 - 2) This truss has been checked for unbalanced loading conditions.
 - 3) All plates are M20 plates unless otherwise indicated.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 5) A plate rating reduction of 20% has been applied for the green lumber members.
 - 6) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

MAR 05 1998

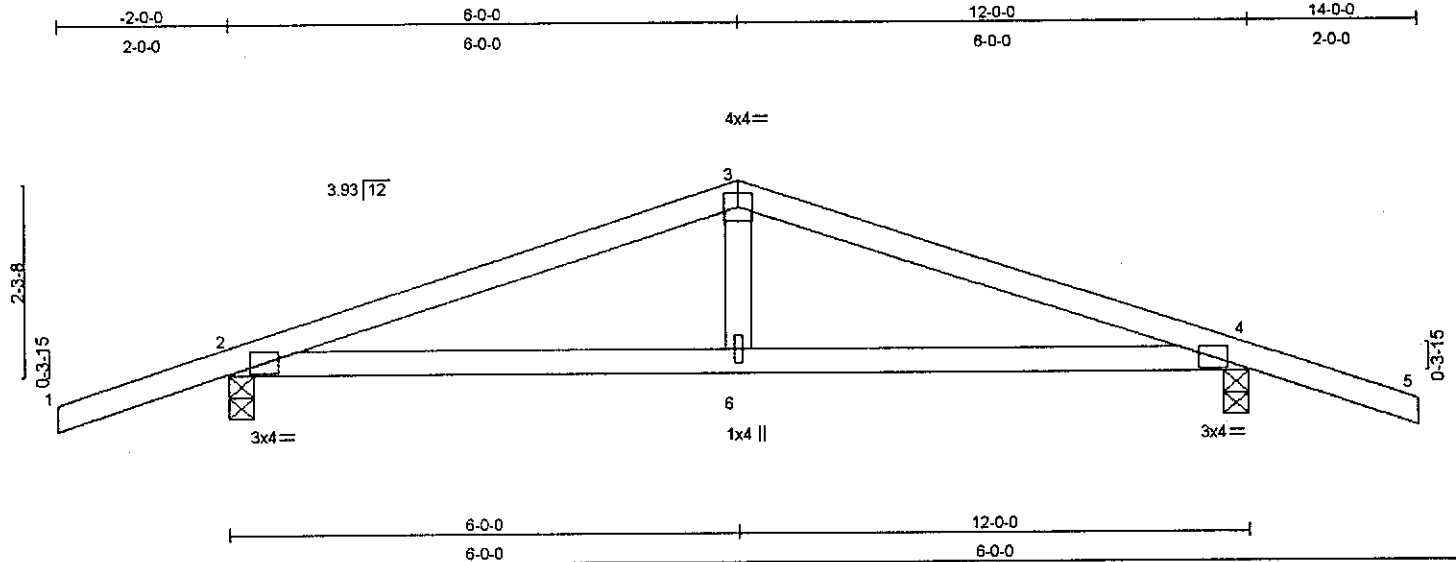


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Job	Truss	Truss Type	Qty	Ply	
DHC	B	KINGPOST	1	1	

GENERAL TRUSS CO., SACRAMENTO, CA 95828

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LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES, GRIP
TCLL 20.0	Plates Increase 1.00	TC 0.40	Vert(LL) -0.03 2-6 >999	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.29	Vert(TL) -0.06 2-6 >999	
BCLL 0.0	Rep Stress Incr YES	WB 0.08	Horz(TL) 0.01 4 n/a	
BCDL 7.0	Code UBC/CBO		Min Length / LL defl = 360	Weight: 42 lb

LUMBER
 TOP CHORD 2 X 4 DF No.1&Btr-G
 BOT CHORD 2 X 4 DF No.1&Btr-G
 WEBS 2 X 4 DF Std-G

BRACING
 TOP CHORD Sheathed or 6-0-0 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=626/0-3-8, 4=626/0-3-8

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=20, 2-3=-745, 3-4=-745, 4-5=20
 BOT CHORD 2-6=705, 4-6=705
 WEBS 3-6=82

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) All plates are M20 plates unless otherwise indicated.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 4) A plate rating reduction of 20% has been applied for the green lumber members.
 - 5) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

MAR 05 1998

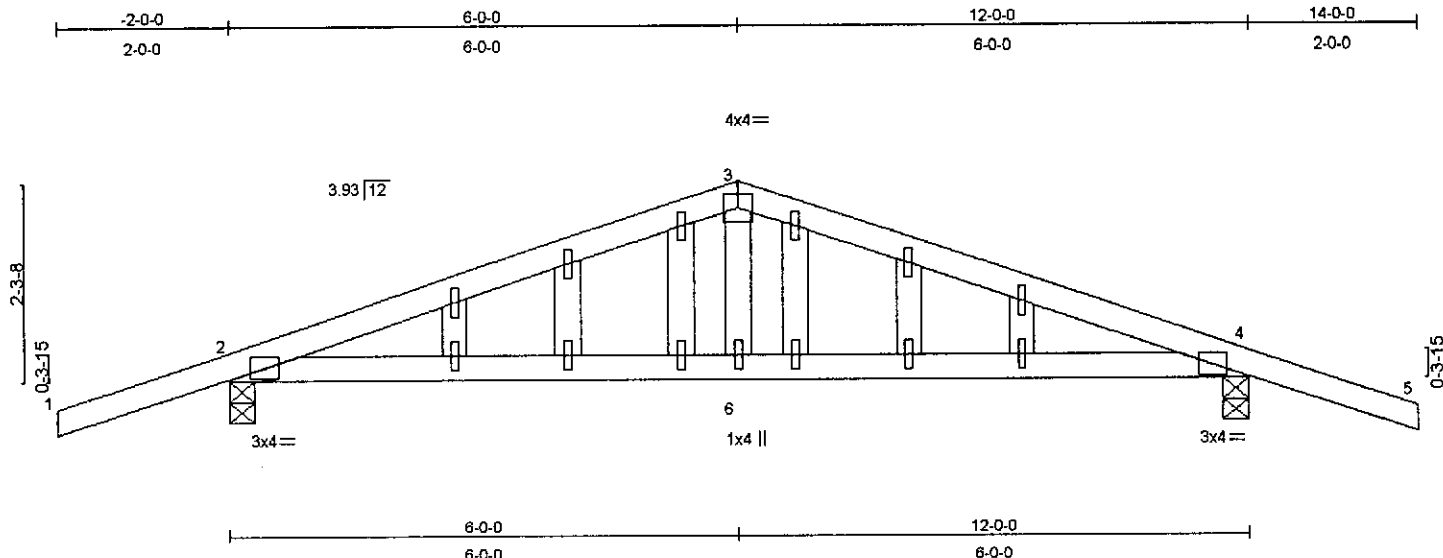


NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	
DHC	BGE	KINGPOST	1	1	

GENERAL TRUSS CO., SACRAMENTO, CA 95828

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LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 20.0	Plates Increase 1.00	TC 0.40	Vert(LL) -0.03 2-6 >999	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.29	Vert(TL) -0.06 2-6 >999	
BCLL 0.0	Rep Stress Incr YES	WB 0.08	Horz(TL) 0.01 4 n/a	
BCDL 7.0	Code UBC/CBO		Min Length / LL defl = 360	Weight: 50 lb

LUMBER
 TOP CHORD 2 X 4 DF No.1&Btr-G
 BOT CHORD 2 X 4 DF No.1&Btr-G
 WEBS 2 X 4 DF Std-G
 OTHERS 2 X 4 DF Std-G

BRACING
 TOP CHORD Sheathed or 6-0-0 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=626/0-3-8, 4=626/0-3-8

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=20, 2-3=-745, 3-4=-745, 4-5=20
 BOT CHORD 2-6=705, 4-6=705
 WEBS 3-6=82

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) All plates are M20 plates unless otherwise indicated.
 - 3) Gable studs spaced at 1-4-0 on center.
 - 4) For studs exposed to wind, see MiTek "Standard Gable End Detail"
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 6) A plate rating reduction of 20% has been applied for the green lumber members.
 - 7) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

MAR 05 1998



NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is **always** required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	
DHC	C	STUDIO VAULT	4	1	

GENERAL TRUSS CO., SACRAMENTO, CA 95828

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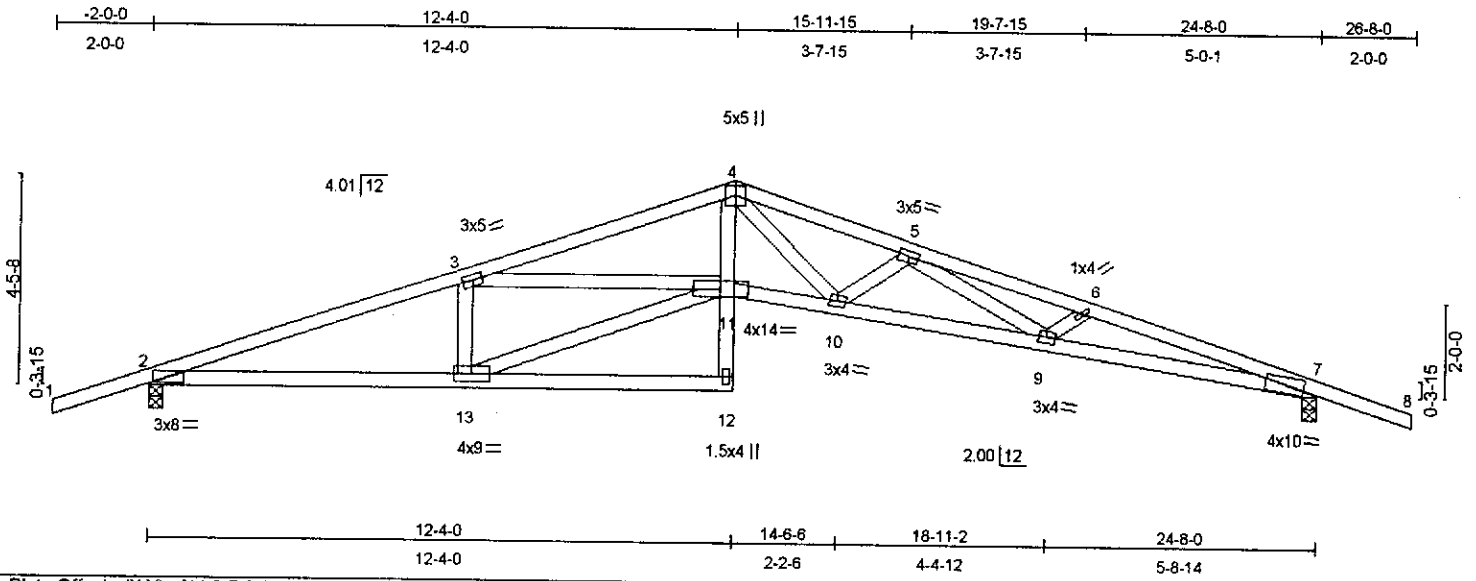


Plate Offsets (X,Y): [11:0-7-0,0-1-13]				
LOADING (psf)	SPACING	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 20.0	Plates Increase 2-0-0 Lumber Increase 1.00	TC 0.61	Vert(LL) -0.28 10 >999	M20 186/148
TCDL 14.0	Rep Stress Incr YES	BC 0.63	Vert(TL) -0.58 10 >509	
BCLL 0.0	Code UBC/ICBO	WB 0.92	Horz(TL) 0.27 7 n/a	Weight: 110 lb
BCDL 7.0			Min Length / LL defl = 360	

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr-G
BOT CHORD 2 X 4 DF No.1&Btr-G *Except*
4-12 2 X 4 DF Std-G
WEBS 2 X 4 DF Std-G

BRACING
TOP CHORD Sheathed or 2-6-13 on center purlin spacing.
BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing, Except:
12-13=5-6-7.

REACTIONS (lb/size) 2=1146/0-3-8, 7=1145/0-3-8

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=21, 2-3=-2238, 3-4=-3267, 4-5=-3421, 5-6=-4277, 6-7=-4487, 7-8=20
BOT CHORD 2-13=2114, 12-13=0, 11-12=39, 4-11=1516, 10-11=3058, 9-10=3639, 7-9=4286
WEBS 4-10=391, 5-10=-460, 5-9=567, 6-9=-226, 3-13=-737, 3-11=984, 11-13=2268

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) All plates are M20 plates unless otherwise indicated.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 4) A plate rating reduction of 20% has been applied for the green lumber members.
 - 5) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 6) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

MAR 05 1998

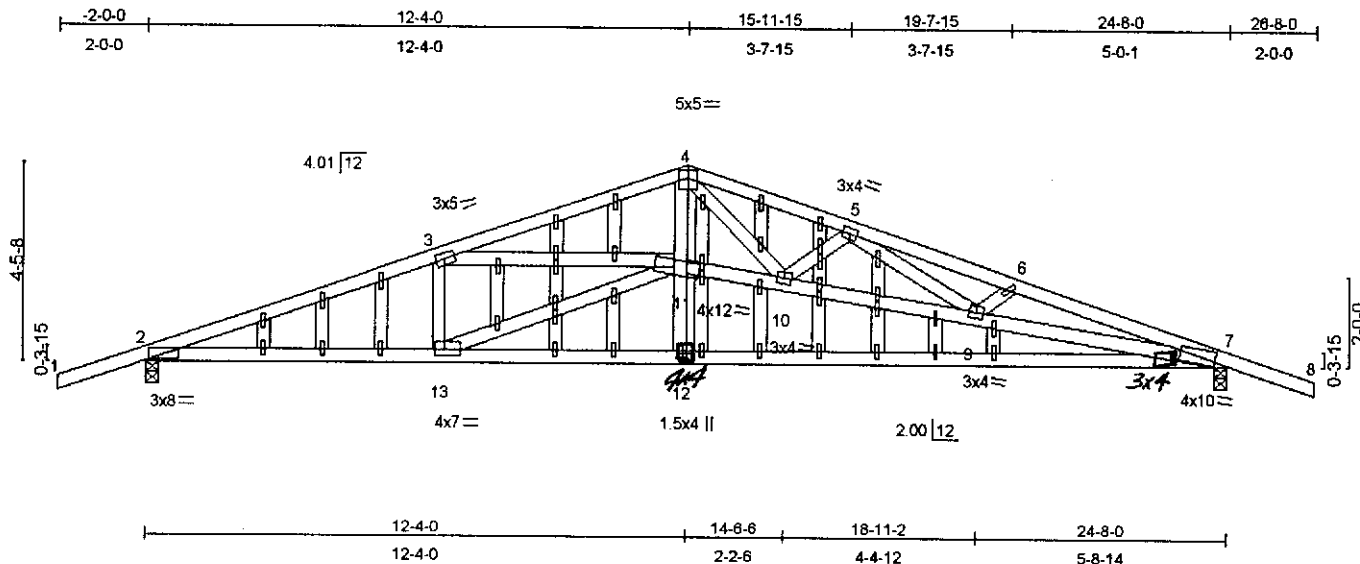


NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	
DHC	CGE	STUDIO VAULT	1	1	

GENERAL TRUSS CO., SACRAMENTO, CA 95828

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Offsets (X,Y): [4:0-2-8,0-2-0], [5:0-1-12,0-1-8], [7:0-0-12,0-2-8], [11:0-6-12,0-2-0], [13:0-2-8,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 20.0	Plates Increase 2-0-0 Lumber Increase 1.00	TC 0.61	Vert(LL) -0.28 10 >999	M20 186/148
TCDL 14.0	Rep Stress Incr YES	BC 0.63	Vert(TL) -0.58 10 >509	
BCLL 0.0	Code UBC/ICBO	WB 0.92	Horz(TL) 0.27 7 n/a	Weight: 155 lb
BCDL 7.0			Min Length / LL defl = 360	

LUMBER

TOP CHORD 2 X 4 DF No.1&Btr-G
 BOT CHORD 2 X 4 DF No.1&Btr-G *Except*
 4-12 2 X 4 DF Std-G
 WEBS 2 X 4 DF Std-G
 OTHERS 2 X 4 DF Std-G

BRACING

TOP CHORD Sheathed or 2-6-13 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing, Except:
 12-13=5-6-7.

REACTIONS (lb/size) 2=1146/0-3-8, 7=1145/0-3-8

FORCES (lb) - First Load Case Only

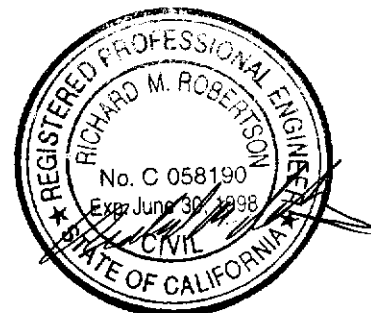
TOP CHORD 1-2=21, 2-3=-2238, 3-4=-3267, 4-5=-3421, 5-6=-4277, 6-7=-4487, 7-8=20
 BOT CHORD 2-13=2114, 12-13=0, 11-12=39, 4-11=1516, 10-11=3058, 9-10=3639, 7-9=4286
 WEBS 4-10=391, 5-10=-460, 5-9=567, 6-9=-226, 3-13=-737, 3-11=984, 11-13=2268

NOTES

- 1) This truss has been checked for unbalanced loading conditions.
- 2) All plates are M20 plates unless otherwise indicated.
- 3) All plates are 1x4 M20 unless otherwise indicated.
- 4) Gable studs spaced at 1-4-0 on center.
- 5) For studs exposed to wind, see MiTek "Standard Gable End Detail"
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
- 7) A plate rating reduction of 20% has been applied for the green lumber members.
- 8) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

MAR 05 1998



NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is **always** required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute; 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	
DHC	D	MONO SCISSOR	2	1	

GENERAL TRUSS CO., SACRAMENTO, CA 95828

4.0-32 s Dec 16 1997 MiTek Industries, Inc. Thu Mar 05 10:14:08 1998 Page 1

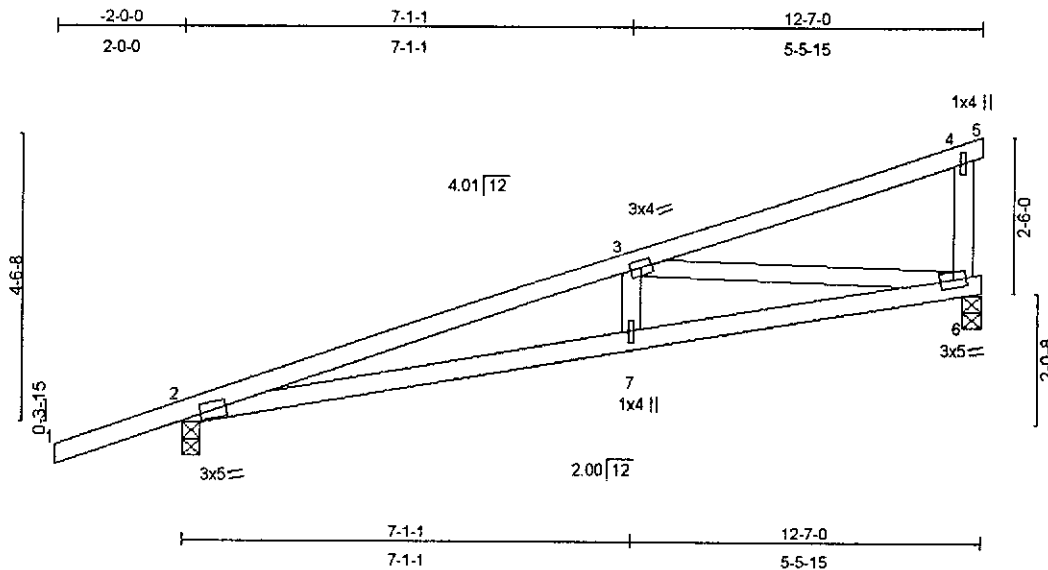


Plate Offsets (X,Y): [2:0-3-10,0-0-7]

LOADING (psf)	SPACING	CSI	DEFL (in)	(loc)	l/defl	PLATES	GRIP
TCLL 20.0	2-0-0 Plates Increase 1.00	TC 0.49	Vert(LL) -0.07	2-7	>999	M20	186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.37	Vert(TL) -0.13	2-7	>999		
BCLL 0.0	Rep Stress Incr YES	WB 0.56	Horz(TL) 0.02	6	n/a		
BCDL 7.0	Code UBC/ICBO		Min Length / LL defl = 360				Weight: 49 lb

LUMBER
 TOP CHORD 2 X 4 DF No.1&Btr-G
 BOT CHORD 2 X 4 DF No.1&Btr-G
 WEBS 2 X 4 DF Std-G

BRACING
 TOP CHORD Sheathed or 4-11-9 on center purlin spacing, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=643/0-3-8, 6=519/0-3-8

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=20, 2-3=-1237, 3-4=0, 4-5=-3, 4-6=-197
 BOT CHORD 2-7=1184, 6-7=1184
 WEBS 3-7=85, 3-6=-1172

- NOTES**
- 1) All plates are M20 plates unless otherwise indicated.
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 3) A plate rating reduction of 20% has been applied for the green lumber members.
 - 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 5) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

MAR 05 1998



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Job	Truss	Truss Type	Qty	Ply	
DHC	E	MONO SCISSOR	6	1	

GENERAL TRUSS CO., SACRAMENTO, CA 95828

4.0-32 s Dec 16 1997 MiTek Industries, Inc. Thu Mar 05 10:14:11 1998 Page 1

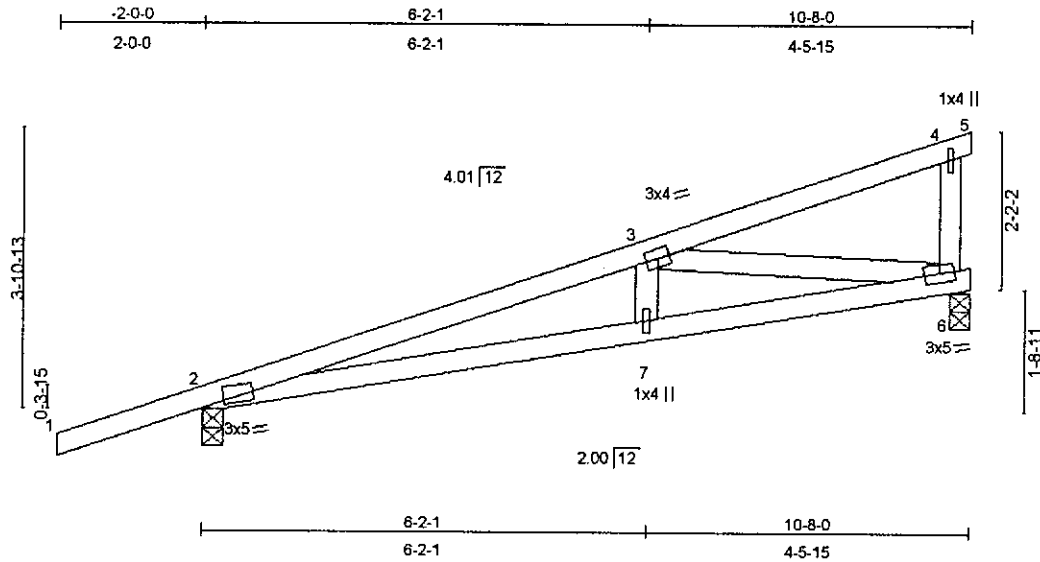


Plate Offsets (X,Y): [2:0-0-4,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL (in) (loc) 1/defl	PLATES GRIP
TCLL 20.0	2-0-0	TC 0.32	Vert(LL) -0.04 2-7 >999	M20 186/148
TCDL 14.0	Plates Increase 1.00	BC 0.28	Vert(TL) -0.08 2-7 >999	
BCLL 0.0	Lumber Increase 1.25	WB 0.30	Horz(TL) 0.01 6 n/a	
BCDL 7.0	Rep Stress Incr YES		Min Length / LL defl = 360	Weight: 42 lb
	Code UBC/ICBO			

LUMBER

TOP CHORD 2 X 4 DF No.1&Btr-G
 BOT CHORD 2 X 4 DF No.1&Btr-G
 WEBS 2 X 4 DF Std-G

BRACING

TOP CHORD Sheathed or 4-5-3 on center purlin spacing, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=565/0-3-8, 6=440/0-3-8

FORCES (lb) - First Load Case Only

TOP CHORD 1-2=20, 2-3=-989, 3-4=0, 4-5=-3, 4-6=-163
 BOT CHORD 2-7=946, 6-7=946
 WEBS 3-7=72, 3-6=-937

NOTES

- 1) All plates are M20 plates unless otherwise indicated.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
- 3) A plate rating reduction of 20% has been applied for the green lumber members.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

MAR 05 1998

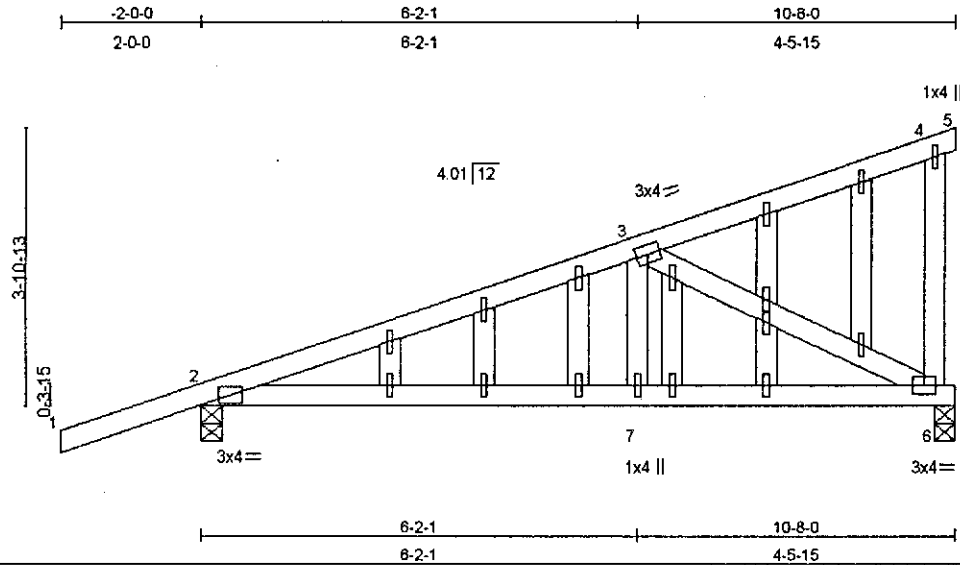
NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is **always** required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job DHC	Truss EGE	Truss Type MONO SCISSOR	Qty 1	Ply 1	
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GENERAL TRUSS CO., SACRAMENTO, CA 95828

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LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 20.0	Plates Increase 1.00	TC 0.30	Vert(LL) -0.03 2-7 >999	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.24	Vert(TL) -0.06 2-7 >999	
BCLL 0.0	Rep Stress Incr YES	WB 0.21	Horz(TL) 0.01 6 n/a	
BCDL 7.0	Code UBC/ICBO		Min Length / LL defl = 360	Weight: 58 lb

LUMBER

TOP CHORD 2 X 4 DF No.1&Bfr-G
 BOT CHORD 2 X 4 DF No.1&Bfr-G
 WEBS 2 X 4 DF Std-G
 OTHERS 2 X 4 DF Std-G

BRACING

TOP CHORD Sheathed or 4-5-3 on center purlin spacing, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=565/0-3-8, 6=439/0-3-8

FORCES (lb) - First Load Case Only

TOP CHORD 1-2=21, 2-3=-525, 3-4=0, 4-5=-3, 4-6=-163
 BOT CHORD 2-7=495, 6-7=495
 WEBS 3-7=72, 3-6=-554

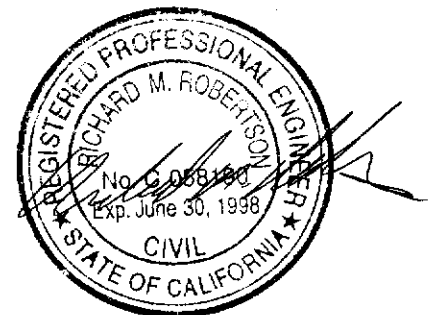
NOTES

- 1) All plates are M20 plates unless otherwise indicated.
- 2) Gable studs spaced at 1-4-0 on center.
- 3) For studs exposed to wind, see MiTek "Standard Gable End Detail"
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
- 5) A plate rating reduction of 20% has been applied for the green lumber members.
- 6) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

MAR 05 1998

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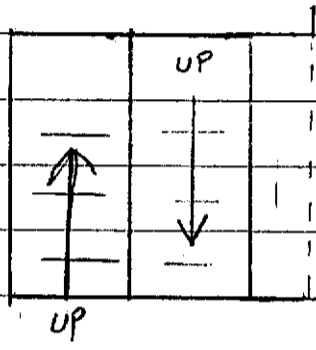


30'3"

4'x5' FP 4'x5' 8'x6' slider

Living Rm.

Nook



Kitchen

3'x3'

Bath

3'x1'

Bedroom

Laundry

54

Scope of Work

- REPLACE 2nd Store TRUSSES
- " Repair
- Repair Plumbing Vents
- Electrical Rewire
- Roofing Complete
- Cabinets
- Asphalt
- Paint Int. & ext.
- 25yr Comp Roofing

GARAGE

20'3"

* Andersen
 Vallarta Circle
 1638

Existing
 Floor
 plans
 No change
 ok 47

