

CITY OF SACRAMENTO

Permit No: 9900593

1231 I Street, Sacramento, CA 95814

Insp Area: 1

Site Address: 2142 MORLEY WY SAC

Sub-Type: ASFR

Parcel No: 293-0121-010

Housing (Y/N): N

CONTRACTOR

OWNER

ARCHITECT

HAGEN NEIL ERIC/HEIDI
2142 MORLEY WY
SACRAMENTO CA 95864

SEISS WAGNER
266 39th st
SAC CA

Nature of Work: 2ND STORY ADDITION AND MASTER BATH REMODEL

CONSTRUCTION LENDING AGENCY: I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name Lender's Address

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class B License Number 428383 Date 3-3-99 Contractor Signature Cy Paul

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. B & PC for this reason: CITY OF SACRAMENTO

Date Owner Signature

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the above-mentioned property for inspection purposes.

Date 3-3-99 Applicant/Agent Signature Cy Paul

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier STATE FUND Policy Number 1325661-98 Exp Date 10-1-99

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 3-3-99 Applicant Signature Cy Paul

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

**CERTIFICATION OF COMPLIANCE**  
**SCHOOL DISTRICT DEVELOPMENT FEES**

SAN JUAN UNIFIED  
SCHOOL DISTRICT

(Print or Type)

**PART I TO BE COMPLETED BY APPLICANT**

PROPERTY OWNER'S NAME MEIL HEIDI HAGEN  
OWNER'S ADDRESS 2142 MORLEY WY SAC CA  
PROJECT ADDRESS 2142 MORLEY WY SAC CA  
PARCEL NO. 293-121-010 LOT NO. \_\_\_\_\_  
SUBDIVISION NAME \_\_\_\_\_  
NUMBER OF UNITS \_\_\_\_\_  
APPLICANT'S SIGNATURE [Signature]  
TITLE OF APPLICANT GEN. CONTRACTOR  
DATE 3-2-99 TELEPHONE NUMBER 9874890

**PART II TO BE COMPLETED BY BUILDING DEPARTMENT**

PLAN IDENTIFICATION NO. 9900593  
BUILDING TYPE (CHECK ONE)  
RESIDENTIAL  APARTMENT/CONDOMINIUM ( ) COMMERCIAL/INDUSTRIAL ( )  
SQUARE FEET OF CHARGEABLE BUILDING AREA 1536  
SIGNATURE [Signature]  
TITLE INSPECTOR DATE \_\_\_\_\_

**PART III TO BE COMPLETED BY SCHOOL DISTRICT**

SCHOOL DISTRICT See Attached Form 755  
DISTRICT CERTIFICATION NO. \_\_\_\_\_  
FEES COLLECTED \_\_\_\_\_  
RESIDENTIAL \_\_\_\_\_ SQ.FT X \$ \_\_\_\_\_ = \$ \_\_\_\_\_  
APARTMENT/CONDOMINIUM \_\_\_\_\_ SQ.FT X \$ \_\_\_\_\_ = \$ \_\_\_\_\_  
COMMERCIAL/INDUSTRIAL \_\_\_\_\_ SQ.FT X \$ \_\_\_\_\_ = \$ \_\_\_\_\_

This Certification covers only the amount of square footage indicated above. Any additions or corrections to the square footage for this project will require an amendment to the Certificate of Compliance.

As the authorized school district official, I hereby certify that the requirements of Government Code Section 65995 have been complied with by the above signed applicant.

AUTHORIZED SCHOOL DISTRICT OFFICIAL  
SIGNATURE \_\_\_\_\_  
TITLE \_\_\_\_\_ DATE \_\_\_\_\_

- Original: School District
- 1st Copy: School District
- 2nd Copy: Building Department
- 3rd Copy: Applicant

# Certification of Compliance

## School District Development Fees

### Part I—To be completed by APPLICANT

Owner's name NEEL . NEELI WAGEN  
 Owners's address 2147 MORLEY WY, SACO 95864  
 Project address 2147 MORLEY WY  
 Parcel number 293 121 010 Lot no. \_\_\_\_\_  
 Subdivision name \_\_\_\_\_ No. of units 1  
 Applicants signature [Signature] Title Gen. Contractor  
 Phone no. 7974890 Date 3-2-99

NOTICE TO APPLICANT: Pursuant to government code section 66020(d), this will serve to notify you that the 90-day approval period in which you may protest the fees or other payment identified above will begin to run on the date in which the building or installation permit for this project is issued or on which they are paid to the district(s) or to another public entity authorized to collect them on behalf of the district(s), whichever is earlier.

### Part II—To be completed by BUILDING DEPARTMENT

Plan identification number \_\_\_\_\_  
 Building type (check one)  
 Residential       Apartment/condominium       Commercial/industrial  
 Square feet of chargeable building area \_\_\_\_\_  
 Signature/title See Attached Form Date \_\_\_\_\_

### Part III—To be completed by SCHOOL DISTRICT

School district SAN JUAN UNIFIED SCHOOL DISTRICT Cert. no. SJ-99-7107 <sup>7E</sup>  
 Exempt—Comments \_\_\_\_\_  
 Residential/apartment/etc. 1536 Square ft. X \$ 193 = \$ 296,448  
 Commercial/industrial \_\_\_\_\_ Square ft. X \$ \_\_\_\_\_ = \$ \_\_\_\_\_  
 Total fees collected CK 2990 = \$ 2,964.45

This certification covers only the amount of square footage indicated above. Any additions or corrections to the square footages for this project will require an amendment to the Certificate of Compliance.

As the authorized school district official, I hereby certify that the requirements of government code section 65995 and any other authorized requirements have been complied with by the above signed applicant.

Signature Richard J. Edwards 4115 Date 3-2-99

**City of Sacramento Development Services Division  
Planning and Zoning Information Request**

Project Address: 2142 MOTLEY WAY, SAC. 95864

Assessor's Parcel Number: 293-0121-010

Current Land Use: SINGLE FAMILY

Description of Request/Proposed Use: RESIDENTIAL 2ND FLOOR  
ADDITION / M. BATH ADDITION

Zoning Designation: R-1

Prior Applications for Project Site(P#,Z#,DRPB#): \_\_\_\_\_

Comments: Comply w/ R-1 zone  
reqs

Are There Any Planning Issues?: (Circle One) YES  NO

Site Plan Check Required? (Circle One) YES  NO

Design Review/ Preservation Required?: (Circle One) YES  NO

Planning Review by/Date: W. J. Bourne 1/21/99

A list of items that must be reviewed by Planning is provided on the reverse side of this form.

Job	Truss	Truss Type	Qty	Ply	Neubauer / Hagen	R1327142
3172GS	A-08	CAL HIP	1	1		

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Placerville Truss, Placerville, CA 95667

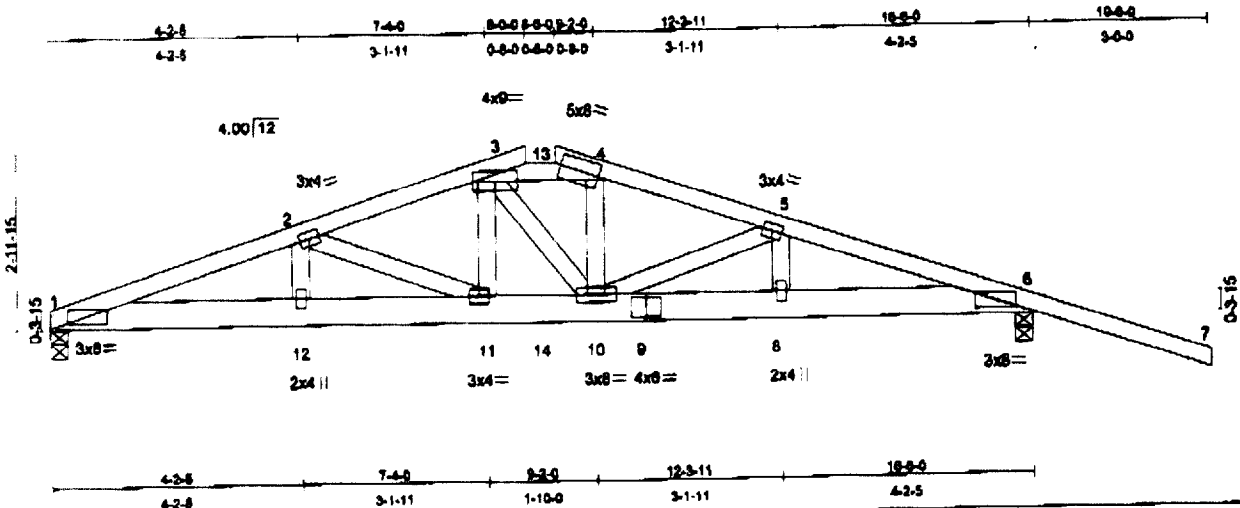


Plate Offset (X,Y): [1:0-3-6, 0-0-14], [8:0-3-6, 0-0-14]							
LOADING (pcf)	SPACING	2-0-0	CSI	DEFL (in)	(loc)	Vdef	PLATES GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.61	Vert(LL)	-0.11	10	M20 185/144
TCDL 10.0	Lumber Increase	1.15	BC 0.37	Vert(TL)	-0.20	10	>993
BCLL 0.0	Rep Stress Incr	NO	WB 0.16	Horz(TL)	0.05	8	n/a
BCDL 8.0	Code	UBC/ANSI96		1st LC LL Min Vdef	= 360		Weight 72 lb

**LUMBER**  
 TOP CHORD 2 X 4 SPF M 19  
 BOT CHORD 2 X 6 SPF 1650F 1 SE  
 WEBS 2 X 4 HF Std

**BRACING**  
 TOP CHORD Sheathed or 3-7-2 on center purfin spacing.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

**REACTIONS (lb/size)** 1=1212/0-3-8, 8=1431/0-3-8

**FORCES (lb) - First Load Case Only**  
 TOP CHORD 1-2=3308, 2-3=2873, 3-13=2846, 4-13=2846, 4-5=2967, 5-6=3404, 6-7=30  
 BOT CHORD 1-12=3140, 11-12=3140, 11-14=2701, 10-14=2701, 8-10=3231, 8-9=3231, 6-8=3231  
 WEBS 2-12=87, 2-11=460, 3-11=239, 3-10=251, 4-10=18, 5-10=431, 6-8=87

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
  - 2) Except as shown below, special connection(s) required to support concentrated load(s). Design of connection(s) is delegated to the building designer.
  - 3) Provide adequate drainage to prevent water ponding.
  - 4) All plates are M20 plates unless otherwise indicated.
  - 5) This truss has been designed for a 10.0 pcf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
  - 6) This truss has been designed with ANSI/TPI 1-1985 criteria.

**LOAD CASE(S) Standard**  
 1) Regular: Lumber Increase=1.15, Plate Increase=1.15  
 Uniform Loads (pcf)  
 Vert 1-2=60.0, 2-3=60.0, 3-13=67.8, 4-13=67.8, 4-5=60.0, 6-8=60.0, 8-7=60.0, 1-12=24.3, 11-12=24.3,  
 11-14=24.3, 10-14=24.3, 9-10=24.3, 8-9=24.3, 6-8=24.3  
 Concentrated Loads (lb)  
 Vert 3=520 4=520



April 1, 1999

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standards, DSB-88 Bracing Specification, and HIB-81 Hoisting and Bracing Recommendation available from Truss Plate Institute, 665 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	Neubauer / Hagen	R1327143
3172GS	A	MONO TRUSS	1	2		

Piecerilla Truss, Piecerilla, CA 95667

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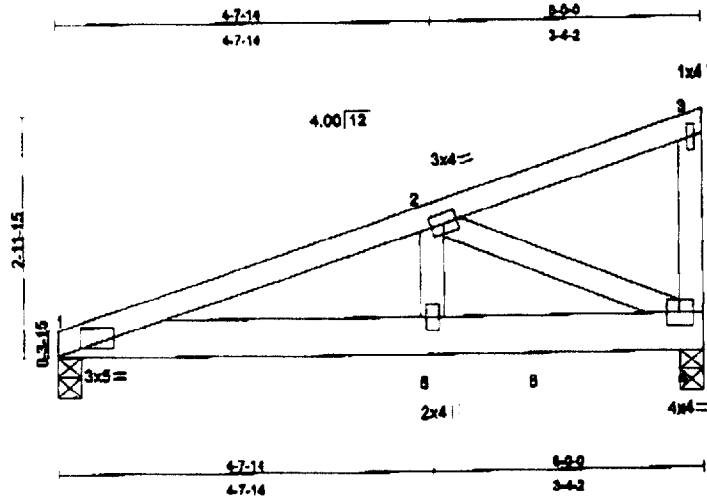


Plate Offsets (X,Y): [1:0-4-8,edge]						
LOADING (psf)	SPACING	2-0-0	CSI	DEPL (in)	(loc)	Vdef
TCLL 20.0	Plates Increase	1.16	TC 0.10	Vert(LL)	-0.02	4-5 >999
TCDL 10.0	Lumber Increase	1.16	BC 0.43	Vert(TL)	-0.03	4-5 >999
BCLL 0.0	Rep Stress Incr	NO	WB 0.21	Horz(TL)	0.01	4 N/A
BCDL 5.0	Code	UBC/ANSI95	(Matrix)	1st LC LL Min Vdef	= 360	
						PLATES GRIP M20 185/144 Weight: 63 lb

**LUMBER**  
 TOP CHORD 2 X 4 SPF M 19  
 BOT CHORD 2 X 6 SPF 1650F 1.6E  
 WEBS 2 X 4 HF Std

**BRACING**  
 TOP CHORD Sheathed or 6-0-0 on center purlin spacing, except end verticals.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

**REACTIONS (lb/size)** 1=681/0-3-8, 4=1190/0-3-8

**FORCES (lb) - First Load Case Only**  
 TOP CHORD 1-2=1487, 2-3=39, 3-4=70  
 BOT CHORD 1-5=1384, 5-6=1384, 4-6=1384  
 WEBS 2-5=832, 2-4=1520

- NOTES**
- 2-ply truss to be connected together with 0.131"x3" Nails as follows: Top chords connected with 1 row(s) at 0-0-0 on center. Bottom chords connected with 2 row(s) at 0-0-0 on center. Webs connected as follows: 2 X 4 - 1 row(s) at 0-0-0 on center.
  - Except as shown below, special connection(s) required to support concentrated load(s). Design of connection(s) is delegated to the building designer.
  - All plates are M20 plates unless otherwise indicated.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
  - This truss has been designed with ANSUTP1 1-1995 criteria.

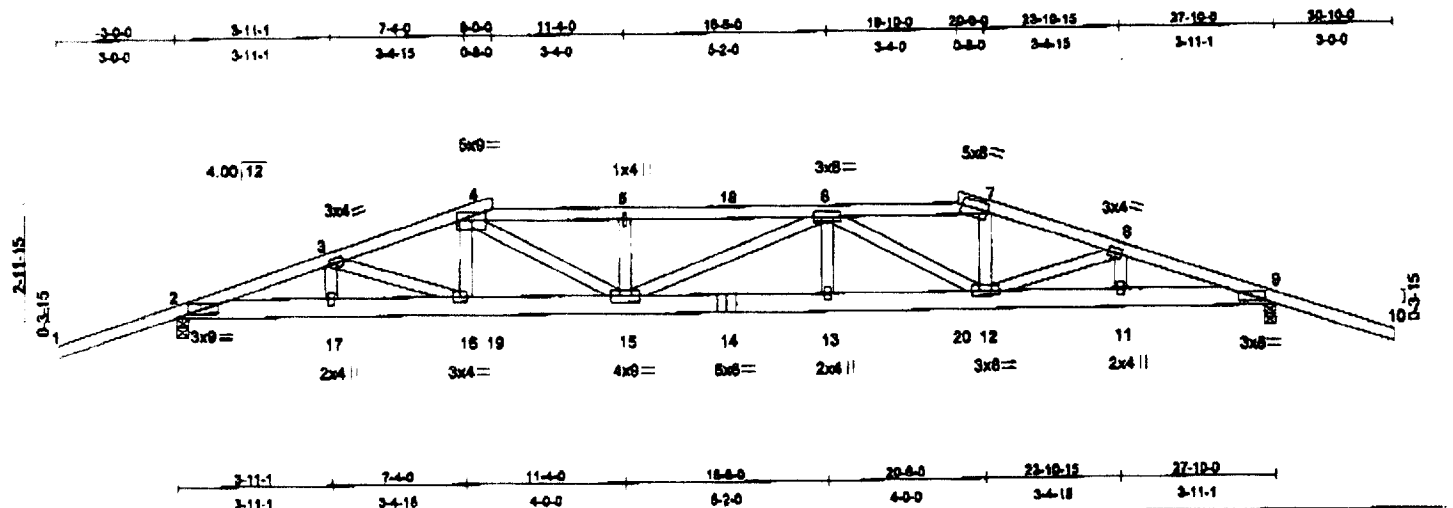
**LOAD CASE(S) Standard**  
 1) Regular: Lumber Increase=1.15, Plate Increase=1.15  
 Uniform Loads (psf)  
 Vert: 1-5=10.0, 5-6=10.0, 4-6=10.0, 1-2=60.0, 2-3=60.0  
 Concentrated Loads (lb)  
 Vert: 8=1212



April 1, 1999

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QBT-88 Quality Standard, DSB-88 Bracing Specification, and MIB-91 Handling, Installing and Bracing Recommendation available from Truss Plate Institute, 583 O' Donofrio Drive, Madison, WI 53718

**MITek Industries, Inc.**



LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) V/defl	PLATES GRIP
TCLL 20.0	Plates Increase 1.15	TC 0.48	Vert(LL) -0.33 13-15 >989	M20 185/144
TCCL 10.0	Lumber Increase 1.15	BC 0.88	Vert(TL) -0.58 13-15 >565	
BCCL 0.0	Rep Stress Incr NO	WB 0.85	Horz(TL) 0.11 9 n/a	Weight 253 lb
BCDL 6.0	Code UBC/ANSI95	(Matrix)	1st LC LL Min V/defl = 360	

**LUMBER**  
 TOP CHORD 2 X 4 SPF M 18  
 BOT CHORD 2 X 6 SPF 1650F 1.5E  
 WEBS 2 X 4 HF Std

**BRACING**  
 TOP CHORD Sheathed or 4-1-6 on center purlin spacing.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

**REACTIONS (lb/size)** 9=2513/0-3-8, 2=2989/0-3-8  
 Max Grav 9=2868(load case 4), 2=3073(load case 3)

**FORCES (lb) - First Load Case Only**  
 TOP CHORD 1-2=60, 2-3=7645, 3-4=7839, 4-5=8886, 5-16=8881, 6-16=8881, 6-7=8178, 7-8=8389, 8-9=8206, 9-10=80  
 BOT CHORD 2-17=7112, 18-17=7112, 18-19=7387, 16-19=7387, 14-15=8675, 13-14=8675, 13-20=8675, 12-20=8675, 11-12=5841, 6-11=5841  
 WEBS 3-17=348, 3-16=336, 4-16=386, 4-15=2989, 5-15=778, 5-16=1321, 6-13=439, 6-12=2885, 7-12=1715, 8-12=208, 8-11=248

- NOTES**
- 2-ply truss to be connected together with 0.131"x3" Nails as follows: Top chords connected with 1 row(s) at 0-8-0 on center. Bottom chords connected with 2 row(s) at 0-8-0 on center. Webs connected as follows: 2 X 4 - 1 row(s) at 0-8-0 on center.
  - This truss has been checked for unbalanced loading conditions.
  - Except as shown below, special connection(s) required to support concentrated load(s). Design of connection(s) is delegated to the building designer.
  - Provide adequate drainage to prevent water ponding.
  - All plates are M20 plates unless otherwise indicated.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
  - This truss has been designed with ANSI/TPI 1-1985 criteria.
  - Load case(s) 1 has been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.

**LOAD CASE(S) Standard Except:**

- Regular: Lumber Increase=1.15, Plates Increase=1.15  
 Uniform Loads (psf)  
 Vert: 1-2=80.0, 2-3=80.0, 3-4=80.0, 4-5=87.8, 5-16=87.8, 6-16=87.8, 6-7=87.8, 7-8=80.0, 8-9=80.0, 9-10=80.0,  
 2-17=24.3, 18-17=24.3, 18-19=24.3, 16-19=24.3, 14-15=84.3, 13-14=84.3, 13-20=84.3, 12-20=24.3, 11-12=24.3,  
 9-11=24.3  
 Concentrated Loads (lb)  
 Vert: 4=520 16=1180



April 1, 1999

**Design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.**

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, D65-88 Bracing Specification, and HIG-91 Handling, Installing and Bracing Recommendation available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53718

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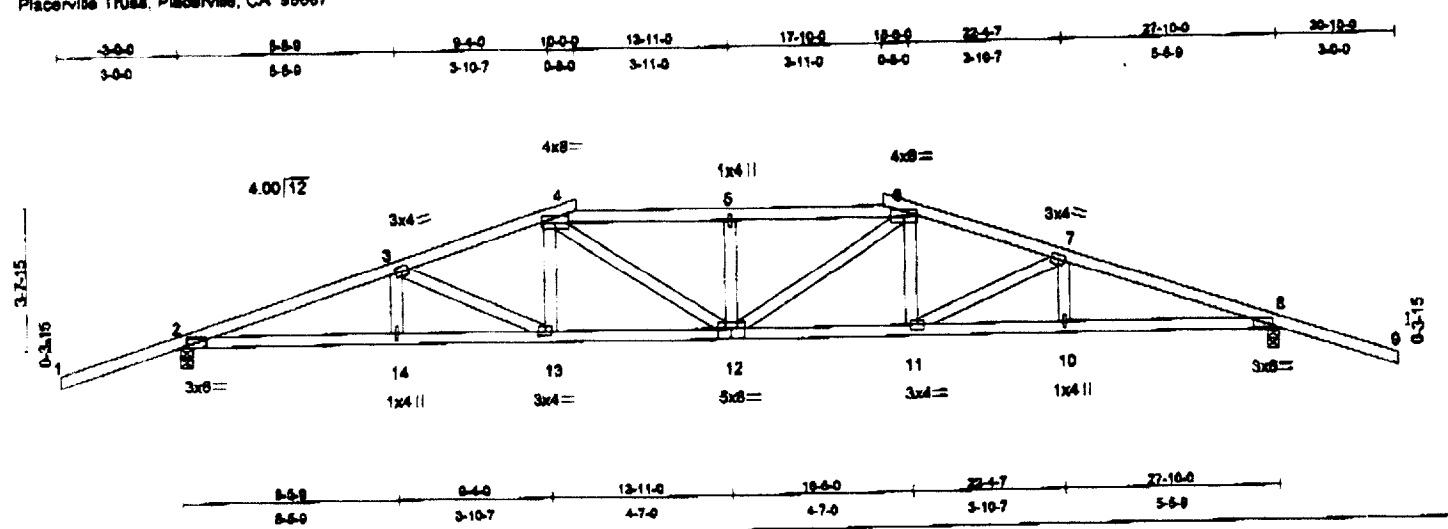


Plate Offsets (X, Y): [12;0-4;0,0-3;0]

LOADING (pcf)	SPACING	2-0-0	CSI	DEFL (in)	(loc)	Vdefl	PLATES GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.51	Vert(LL)	-0.18	12	M20 185/144
TCOL 10.0	Lumber Increase	1.15	BC 0.33	Vert(TL)	-0.28	12	
BCLL 0.0	Rep Stress Incr	NO	WB 0.25	Horz(TL)	0.08	8	Weight: 108 lb
BCOL 6.0	Code	UBC/ANSI95		1st LC LL Min Vdefl	= 360		

**LUMBER**  
 TOP CHORD 2 X 4 SPF M 19  
 BOT CHORD 2 X 4 SPF M 19  
 WEBS 2 X 4 HF Std

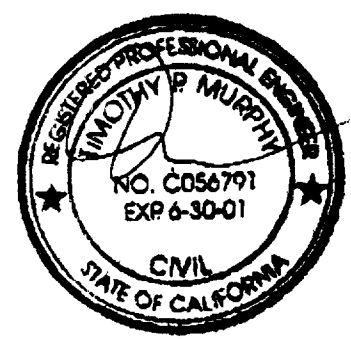
**BRACING**  
 TOP CHORD Sheathed or 4-3-4 on center purlin spacing.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

**REACTIONS (lb/size)** 2=1153/0-3-8, 8=1153/0-3-8

**FORCES (lb) - First Load Case Only**  
 TOP CHORD 1-2=28, 2-3=2367, 3-4=1871, 4-5=2169, 5-6=2169, 6-7=1871, 7-8=2367, 8-9=28  
 BOT CHORD 2-14=2238, 13-14=2238, 12-13=1853, 11-12=1853, 10-11=2238, 8-10=2238  
 WEBS 3-14=46, 3-13=406, 4-13=218, 4-12=382, 5-12=423, 6-12=382, 5-11=218, 7-11=406, 7-10=48

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
  - 2) Provide adequate drainage to prevent water ponding.
  - 3) All plates are M20 plates unless otherwise indicated.
  - 4) This truss has been designed for a 10.0 pcf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-04.
  - 5) This truss has been designed with ANSI/TPI 1-1995 criteria.

**LOAD CASE(S)** Standard



April 1, 1999

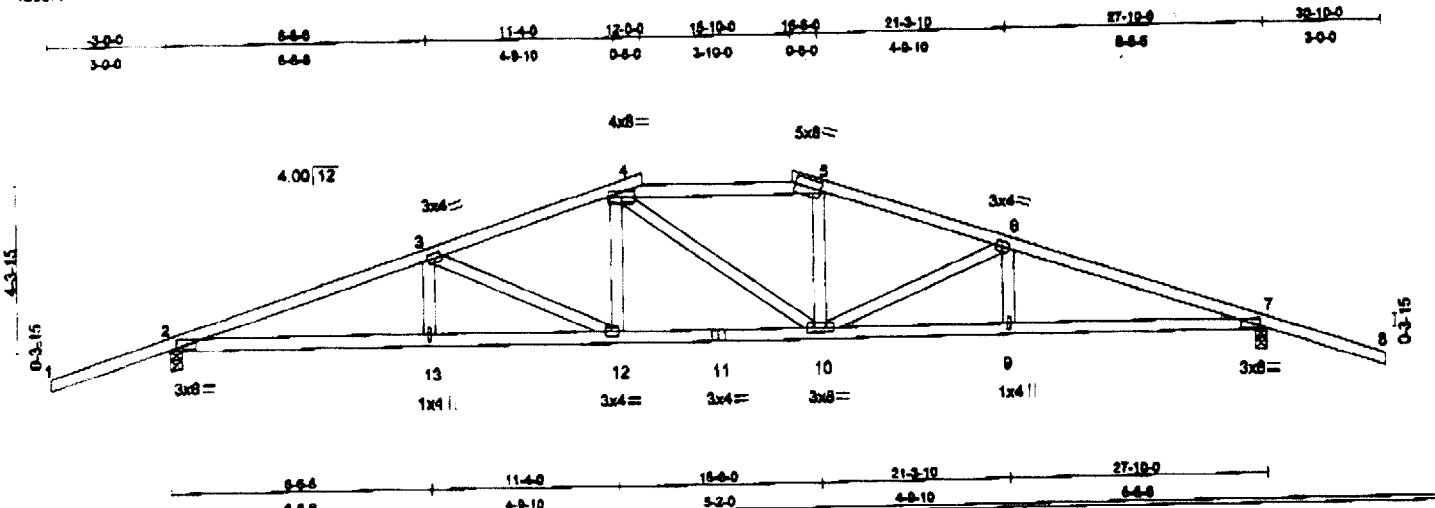
Design void for use only with MITEK connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult GST-88 Quality Standard, DBB-89 Bracing Specification, and HSB-81 Handling, Installing and Bracing Recommendation available from Truss Plate Institute, 563 D'Onofrio Drive, Madison, WI 53718

**MITEK Industries, Inc.**



Job	Truss	Truss Type	Qty	Ply	Neubeuer / Hagen	R1327148
3172GS	B-12	CAL HIP	1	1		

Placerville Truss, Placerville, CA 95667 4.0-32 & Jan 20 1999 MITek Industries, Inc. Thu Apr 01 12:43:28 1999 Page 1



LOADING (pcf)	SPACING	2'-0-0	CSI	DEFL (in)	(loc)	Vdef	PLATES GRIP
TCLL 20.0	Plates Increase	1.16	TC 0.81	Vert(LL)	-0.14	12	M20 155/144
TCDL 10.0	Lumber Increase	1.16	BC 0.36	Vert(TL)	-0.25	10-12	>999
BCLL 0.0	Rep Stress Incr	NO	WB 0.28	Horz(TL)	0.08	7	n/a
BCDL 6.0	Code	UBC/ANSI95		1st LC LL Min Vdef	=	360	Weight: 106 lb

**LUMBER**  
 TOP CHORD 2 X 4 SPF M 19  
 BOT CHORD 2 X 4 SPF M 19  
 WEBS 2 X 4 HF Std

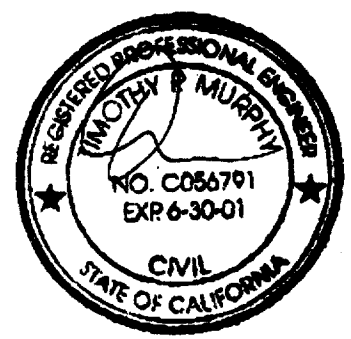
**BRACING**  
 TOP CHORD Sheathed or 4-2-11 on center purlin spacing.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

**REACTIONS (lb/size)** 2=1153/0-3-8, 7=1163/0-3-8

**FORCES (lb) - First Load Case Only**  
 TOP CHORD 1-2=28, 2-3=2288, 3-4=1763, 4-5=1702, 5-6=1781, 6-7=2288, 7-8=28  
 BOT CHORD 2-13=2144, 12-13=2144, 11-12=1655, 10-11=1655, 9-10=2144, 7-8=2144  
 WEBS 3-13=66, 3-12=616, 4-12=267, 4-10=59, 6-10=222, 6-10=500, 6-9=66

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
  - 2) Provide adequate drainage to prevent water ponding.
  - 3) All plates are M20 plates unless otherwise indicated.
  - 4) This truss has been designed for a 10.0 pcf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
  - 5) This truss has been designed with ANSI/TPI 1-1985 criteria.

**LOAD CASE(9) Standard**



April 1, 1999

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-88 Bracing Specification, and MIB-91 Handling, Installing and Bracing Recommendation available from Truss Plate Institute, 563 D'Onofrio Drive, Madison, WI 53719.

**MITek Industries, Inc.**

Job	Truss	Truss Type	Qty	Ply	Neubauer / Hagen	R1327147
3172GS	B	ATTIC	4	1		

Placerville Truss, Placerville, CA 95667 4.0-32 s Jan 20 1999 MITek Industries, Inc. Thu Apr 01 12:43:14 1999 Page 1

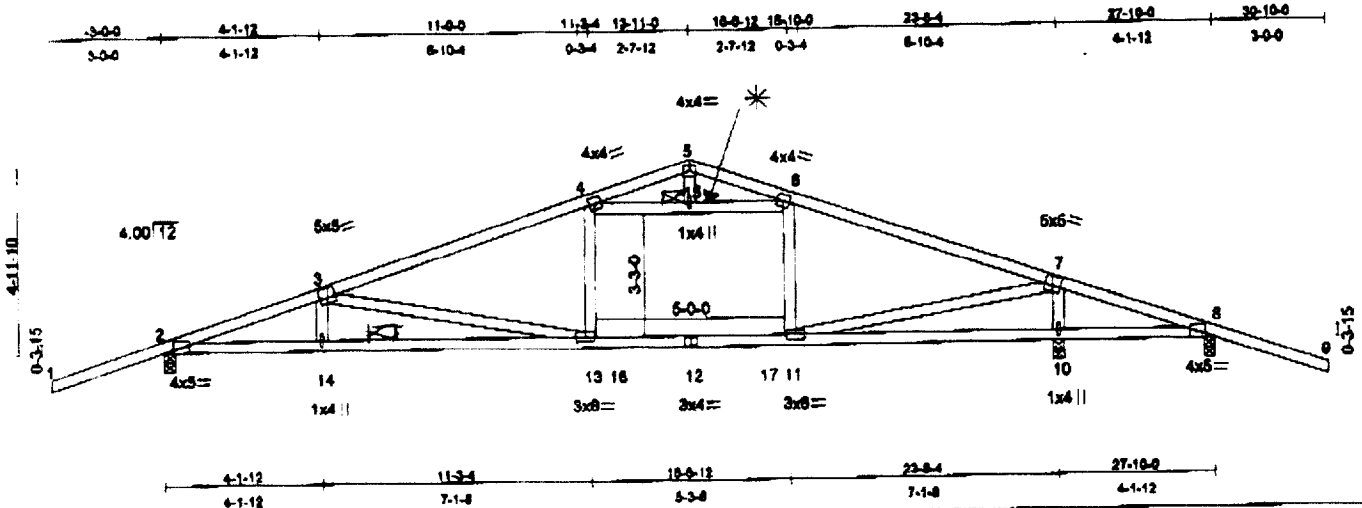


Plate Effects (X,Y): [2:0-2-14,edge], [3:0-2-8,0-3-0], [7:0-2-8,0-3-0], [8:0-2-14,edge]

LOADING (perf)	SPACING	2-0-0	CSI	DEFL (in)	(in)	(loc)	Vdef	PLATES GRIP
TCLL 20.0	Plates Increase	1.16	TC 0.80	Vert(LL)	-0.31	13-14	>999	M20 185/144
TCDL 18.0	Lumber Increase	1.16	BC 0.59	Vert(TL)	-0.62	13-14	>644	
BCLL 0.0	Rep Stress Incr	NO	WB 0.76	Horz(TL)	0.08	8	n/a	Weight: 107 lb
BCDL 5.0	Code	UBC/ANSI95	(Matrix)	1st LC LL Min Vdef	= 360			

**LUMBER**  
 TOP CHORD 2 X 4 SPF M 19  
 BOT CHORD 2 X 4 SPF M 19  
 WEBS 2 X 4 HF Std

**BRACING**  
 \* TOP CHORD Sheathed or 4-3-15 on center purlin spacing.  
 BOT CHORD Rigid ceiling directly applied or 8-0-0 on center bracing.

**REACTIONS (lb/size)** 8=608/0-3-8, 10=680/0-3-8, 2=1091/0-3-8  
 Max Grav 10=864(load case 5), 2=1091(load case 1)

**FORCES (lb) - First Load Case Only**  
 TOP CHORD 1-2=88, 2-3=2227, 3-4=1578, 4-5=348, 5-6=378, 6-7=1585, 7-8=723, 8-9=66  
 BOT CHORD 2-14=2084, 13-14=2081, 13-18=1431, 12-18=1431, 12-17=1431, 11-17=1431, 10-11=712, 8-10=642  
 WEBS 4-15=1142, 6-15=1142, 4-13=170, 6-11=31, 3-14=29, 7-10=706, 3-13=644, 7-11=734, 6-15=221

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
  - 2) 75lb AC unit load placed on the bottom chord, 13-11-0 from left end, supported at two points, 4-0-0 apart.
  - 3) Except as shown below, special connection(s) required to support concentrated load(s). Design of connection(s) is delegated to the building designer.
  - 4) All plates are M20 plates unless otherwise indicated.
  - 5) This truss has been designed for a 10.0 perf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
  - 6) Ceiling dead load (5.0 perf) on member(s), 4-15, 6-15
  - 7) This truss has been designed with ANSI/TPI 1-1985 criteria.

**LOAD CASE(S)** Standard



April 1, 1999

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult Q&T-88 Quality Standard, DBB-88 Bracing Specification, and HB-91 Handling, Installing and Bracing Recommendation available from Truss Plate Institute, 863 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	Neubauer / Hagen	R1327151
9172GS	C	ATTIC	4	1		

Pleasanton Truss, Pleasanton, CA 94567

4.0-32 & Jan 20 1999 MITek Industries, Inc. Thu Apr 01 12:43:32 1999 Page 1

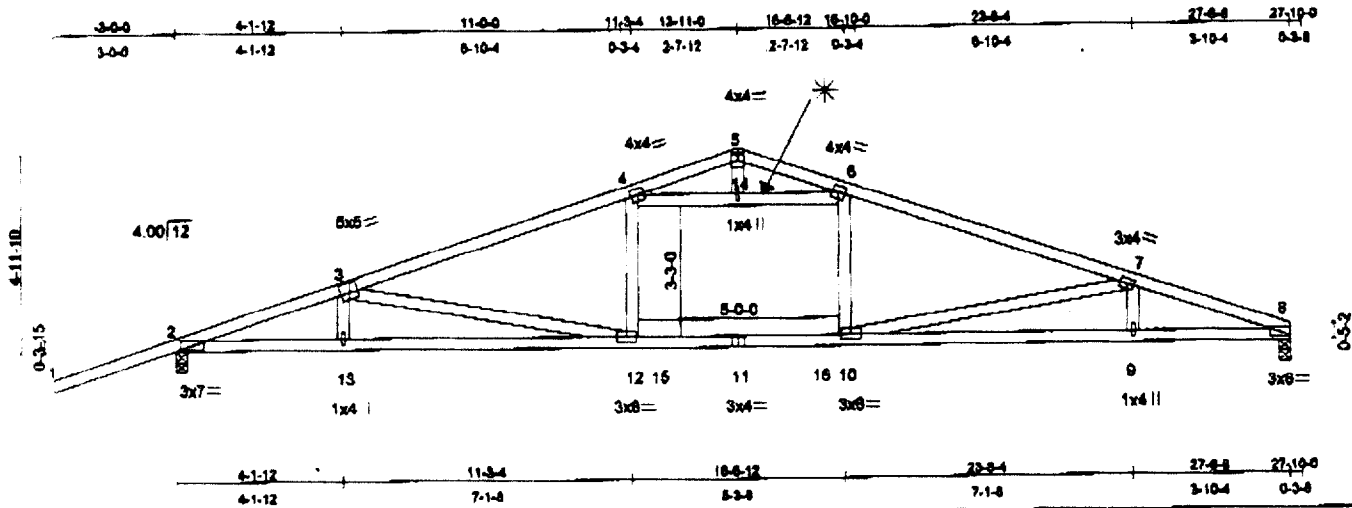


Plate Offsets (X,Y): [3-0-2-8,0-3-0]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL (in) (loc)	Vdefl	PLATES GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.51	Vert(LL) -0.36 12-13	>910	M20 185/144
TCDL 10.0	Lumber Increase	1.15	BC 0.37	Vert(TL) -0.49 12-13	>963	
BCLL 0.0	Rep Stress Incr	NO	WB 0.81	Horz(TL) 0.09 8	n/a	Weight: 102 lb
BCDL 5.0	Code	UBC/ANSI96	(Matrix)	1st LC LL Min Vdefl = 360		

**LUMBER**  
 TOP CHORD 2 X 4 SPF M 19  
 BOT CHORD 2 X 4 SPF M 19  
 WEBS 2 X 4 HF Std

**BRACING**  
 TOP CHORD \* Sheathed or 4-1-8 on center purlin spacing.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

**REACTIONS (lb/size)** 8=981/0-3-8, 2=1180/0-3-8

**FORCES (lb) - First Load Case Only**  
 TOP CHORD 1-2=56, 2-3=2480, 3-4=1854, 4-5=443, 5-6=444, 6-7=1956, 7-8=2509  
 BOT CHORD 2-13=2286, 12-13=2281, 12-15=1784, 11-15=1784, 11-16=1784, 10-16=1784, 9-10=2330, 8-9=2330  
 WEBS 4-14=1433, 6-14=1433, 4-12=218, 6-10=218, 3-13=10, 7-8=34, 3-12=497, 7-10=546, 5-14=277

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
  - 2) 75lb AC unit load placed on the bottom chord, 13-11-0 from left end, supported at two points, 4-0-0 apart.
  - 3) Except as shown below, special connection(s) required to support concentrated load(s). Design of connection(s) is delegated to the building designer.
  - 4) All plates are M20 plates unless otherwise indicated.
  - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-84.
  - 6) Ceiling dead load (5.0 psf) on member(s), 4-14, 6-14
  - 7) This truss has been designed with ANSI/TPI 1-1995 criteria.

**LOAD CASE(S)** Standard

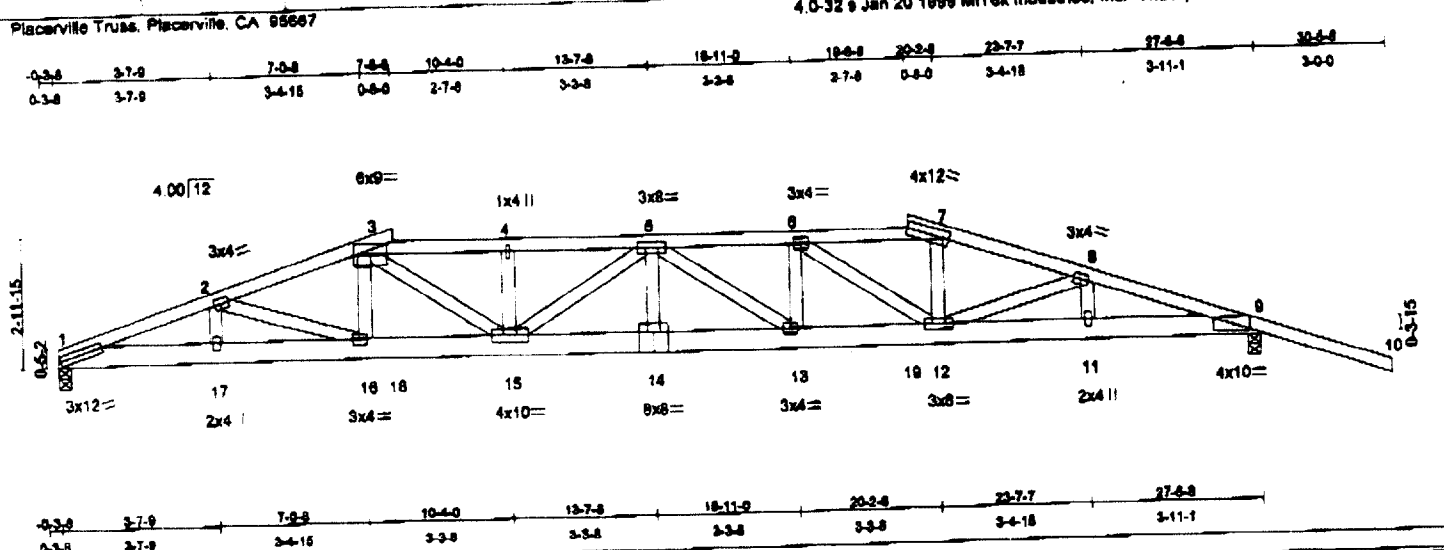


April 1, 1999

*Please design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE U.S.*

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-88 Bracing Specification, and HIB-81 Handling, Installing and Bracing Recommendation available from Truss Plate Institute, 583 D Onafria Drive, Medford, WI 53719

**MITek Industries, Inc.**



LOADING (psf)	SPACING	CSI	DEFL (in)	(loc)	Vdef	PLATES GRIP
TCLL 20.0	2-0-0	TC 0.69	Vert(LL) -0.46	14	>705	M20 185/144
TCOL 10.0	Plates Increase 1.15	BC 0.95	Vert(TL) -0.81	14	>408	
BCLL 0.0	Lumber Increase 1.15	WB 0.92	Horz(TL) 0.16	9	n/a	
BCOL 5.0	Rep Stress Incr NO	(Matrix)	1st LC LL Min Vdef = 350			Weight: 125 lb
	Code UBC/ANSI95					

**LUMBER**  
 TOP CHORD 2 X 4 SPF M 19  
 BOT CHORD 2 X 6 SPF 1650F 1.5E  
 WEBS 2 X 4 HF Std

**BRACING**  
 TOP CHORD Sheathed or 2-1-13 on center purlin spacing.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

**REACTIONS (lb/size)** 1=2194/0-3-8, 9=2405/0-3-12 (input 0-3-8)

**FORCES (lb) - First Load Case Only**  
 TOP CHORD 1-2=5729, 2-3=5866, 3-4=6882, 4-5=6878, 5-6=6930, 6-7=5789, 7-8=6016, 8-9=6891, 9-10=60  
 BOT CHORD 1-17=5361, 18-17=5361, 18-18=5821, 19-18=5821, 14-18=7374, 13-14=7374, 13-18=6930, 12-18=6930, 11-12=5643, 9-11=5643  
 WEBS 2-17=165, 2-18=211, 3-16=280, 3-15=1782, 4-15=595, 5-15=606, 6-14=342, 6-13=542, 6-13=507, 6-12=1395, 7-12=1091, 8-12=187, 8-11=230

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
  - 2) Except as shown below, special connection(s) required to support concentrated load(s). Design of connection(s) is delegated to the building designer.
  - 3) Provide adequate drainage to prevent water ponding.
  - 4) All plates are M20 plates unless otherwise indicated.
  - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
  - 6) WARNING: Required bearing size at joint(s) 9 greater than input bearing size.
  - 7) This truss has been designed with ANSUTPI 1-1995 criteria.

**LOAD CASE(S) Standard**  
 1) Regular: Lumber Increase=1.15, Plate Increase=1.15  
 Uniform Loads (psf)  
 Vert: 1-2=60.0, 2-3=60.0, 3-4=67.8, 4-5=67.8, 5-6=67.8, 6-7=67.8, 7-8=60.0, 8-9=60.0, 9-10=60.0, 1-17=24.3, 16-17=24.3, 18-18=24.3, 15-18=84.3, 14-15=84.3, 13-14=84.3, 13-18=84.3, 12-18=24.3, 11-12=24.3, 8-11=24.3  
 Concentrated Loads (lb)  
 Vert: 3=520 7=520



April 1, 1999

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**MITek Industries, Inc.**

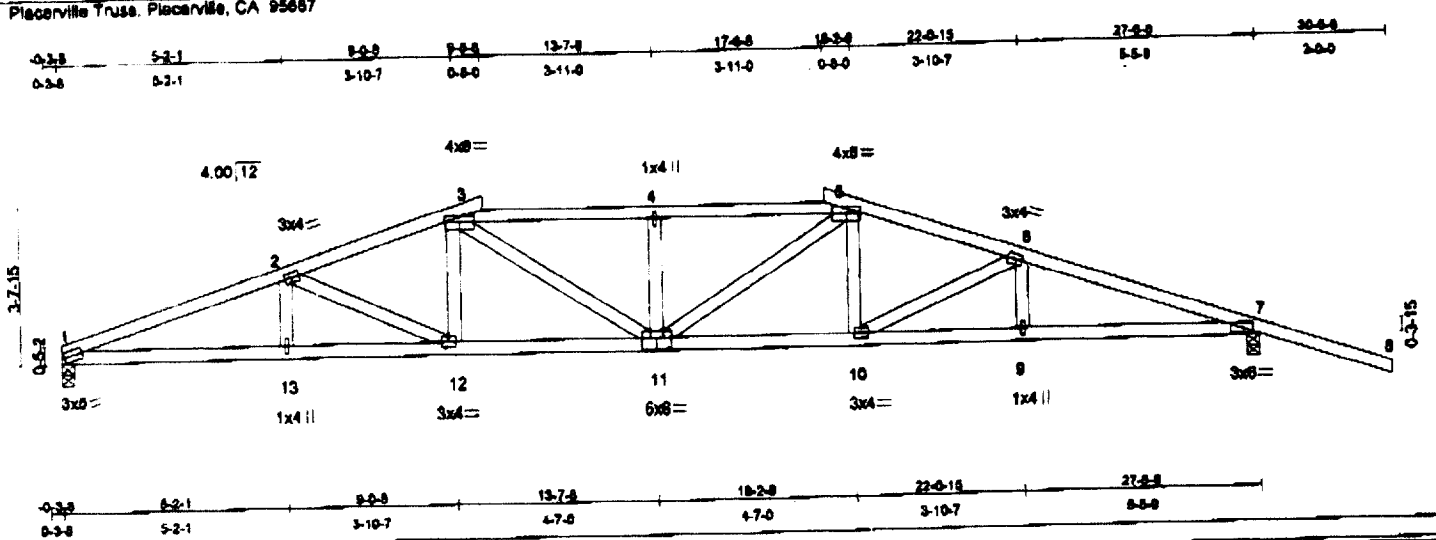


Plate Offsets (X,Y): [11:0-4-0-0-3-0]

LOADING (pcf)	SPACING 2-0-0	CSI	DEFL (in) (loc)	Vdef	PLATES GRIP
TCLL 20.0	Plates Increase 1.15	TC 0.51	Vert(LL) -0.15 11	>999	M20 155/144
TCDL 10.0	Lumber Increase 1.15	BC 0.33	Vert(TL) -0.27 11	>999	
BCLL 0.0	Rep Stress Incr NO	WB 0.25	Horz(TL) 0.08 7	n/a	Weight 104 lb
BCDL 6.0	Code UBC/ANSI96		1st LC LL Min Vdef = 380		

**LUMBER**  
 TOP CHORD 2 X 4 SPF M 19  
 BOT CHORD 2 X 4 SPF M 19  
 WEBS 2 X 4 HF Std

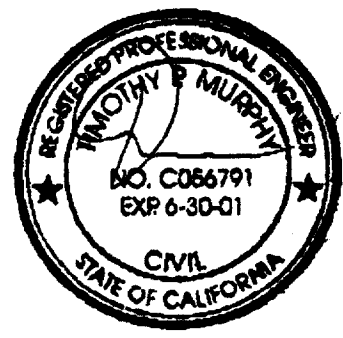
**BRACING**  
 TOP CHORD Sheathed or 4-3-10 on center purlin spacing.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 1=954/0-3-8, 7=1142/0-3-8

FORCES (lb) - First Load Case Only  
 TOP CHORD 1-2=2232, 2-3=1906, 3-4=2123, 4-5=2123, 5-6=1940, 6-7=2336, 7-8=28  
 BOT CHORD 1-13=2113, 12-13=2113, 11-12=1796, 10-11=1823, 9-10=2207, 7-8=2207  
 WEBS 2-13=44, 2-12=336, 3-12=188, 3-11=406, 4-11=420, 6-11=372, 5-10=218, 6-10=406, 6-9=48

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
  - 2) Provide adequate drainage to prevent water ponding.
  - 3) All plates are M20 plates unless otherwise indicated.
  - 4) This truss has been designed for a 10.0 pcf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
  - 5) This truss has been designed with ANSI/TPI 1-1996 criteria.

LOAD CASE(S) Standard



April 1, 1999

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component a responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult GST-88 Quality Standard, DSB-88 Bracing Specification, and HIB-91 Handling, Installing and Bracing Recommendation available from Truss Plate Institute, 543 O'Donohue Drive, Madison, WI 53719

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Job	Truss	Truss Type	Qty	Qty	Neubauer / Hagan	R1327150
3172GS	C-12	CAL HIP	1	1		

Placerville Truss, Placerville, CA 95667 4.0-32 © Jan 20 1999 MITek Industries, Inc. Thu Apr 01 12:43:47 1999 Page 1

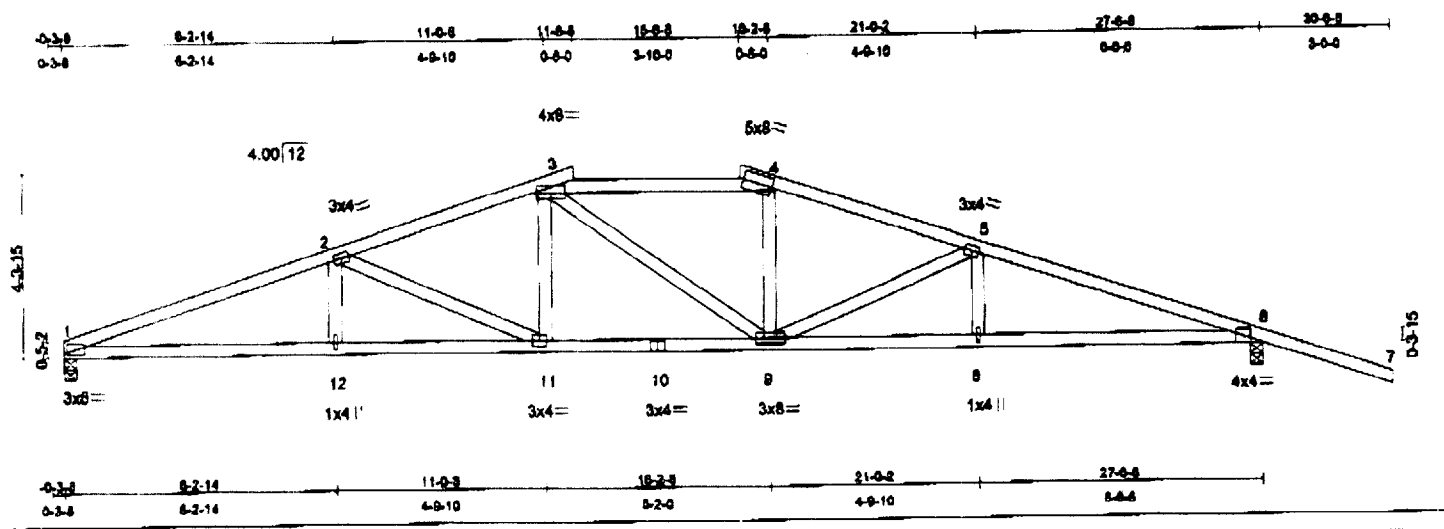


Plate Offsets (X,Y): [8:0-3-0,edge]

<b>LOADING</b> (pcf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> (in) (loc) /defl	<b>PLATES GRIP</b>
TCLL 20.0	Plates Increase 1.16	TC 0.51	Vert(LL) -0.13 9 >999	M20 185/144
TCDL 10.0	Lumber Increase 1.16	BC 0.36	Vert(TL) -0.24 8-11 >999	
BCLL 0.0	Rep Stress Incr NO	WB 0.28	Horz(TL) 0.05 6 n/a	Weight: 101 lb
BCDL 5.0	Code UBC/ANSI95		1st LG LL Min /defl = 360	

**LUMBER**  
 TOP CHORD 2 X 4 SPF M 19  
 BOT CHORD 2 X 4 SPF M 19  
 WEBS 2 X 4 HF Std

**BRACING**  
 TOP CHORD Sheathed or 4-3-9 on center purlin spacing.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

**REACTIONS** (lb/size) 1=954/D-3-8, 8=1142/D-3-8

**FORCES** (lb) - First Load Case Only  
 TOP CHORD 1-2=2176, 2-3=1717, 3-4=1671, 4-5=1749, 5-6=2237, 6-7=28  
 BOT CHORD 1-12=2046, 11-12=2046, 10-11=1613, 9-10=1613, 8-9=2114, 6-8=2114  
 WEBS 2-12=54, 2-11=469, 3-11=242, 3-8=74, 4-6=213, 5-9=500, 5-8=55

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
  - 2) Provide adequate drainage to prevent water ponding.
  - 3) All plates are M20 plates unless otherwise indicated.
  - 4) This truss has been designed for a 10.0 pcf bottom chord live load nonconcurrent with any other live loads per Table No. 18-B, UBC-94.
  - 5) This truss has been designed with ANSI/TPI 1-1995 criteria.

**LOAD CASE(S)** Standard



April 1, 1999

Design vailo for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling, Installing and Bracing Recommendation available from Truss Plate Institute, 883 O' Onofrio Drive, Medison, WI 53719

Job	Truss	Truss Type	Qty	Ply	Neubauer / Hagen	R1327153
3172GS	D-07	CAL HIP	1	3		

Placerville Truss, Placerville, CA 95887 4.0-32 s Jan 20 1999 MITek Industries, Inc. Thu Apr 01 12:43:55 1999 Page 1

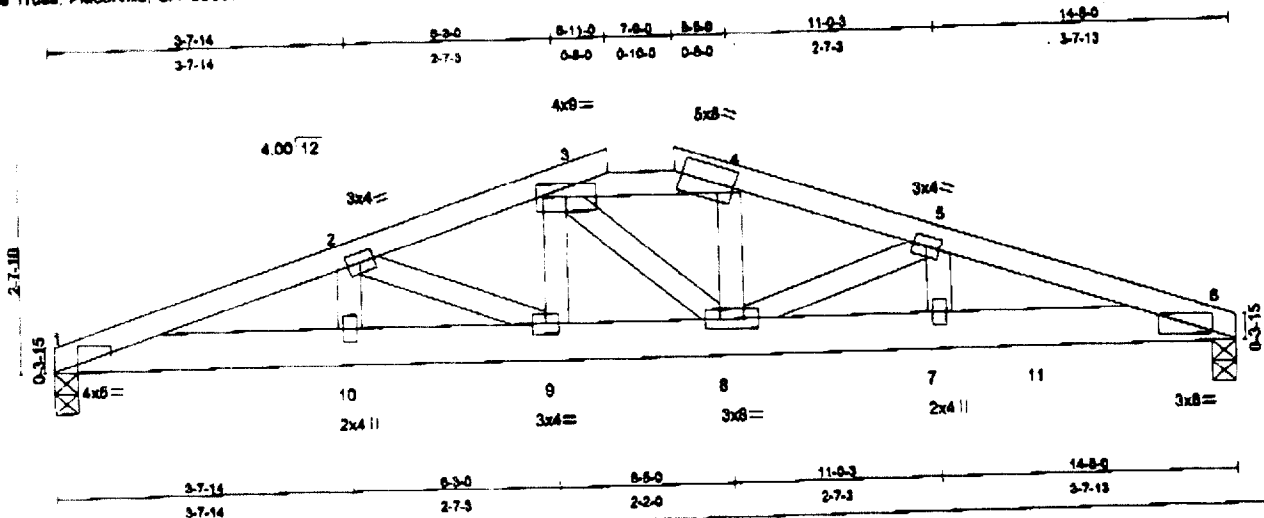


Plate Offsets (X,Y): [1:0-3-8,0-0-2], [8:0-3-8,0-0-14]					PLATES GRIP	
LOADING (psf)	SPACING	2-0-0	CSI	DEFL (in)	(loc)	Vdefl
TCLL 20.0	Plates Increase	1.15	TC 0.28	Ver(LL)	-0.08	8 >999
TCDL 10.0	Lumber Increase	1.15	BC 0.83	Ver(TL)	-0.16	8 >999
BCLL 0.0	Rep Stress Incr	NO	WB 0.34	Horz(TL)	0.04	8 n/a
BCDL 5.0	Code	UBC/ANSI95	(Matrix)	1st LC LL Min Vdefl = 350		Weight 180 lb

**LUMBER**  
 TOP CHORD 2 X 4 SPF M 19  
 BOT CHORD 2 X 6 SPF 1850F 1.6E  
 WEBS 2 X 4 HF Std

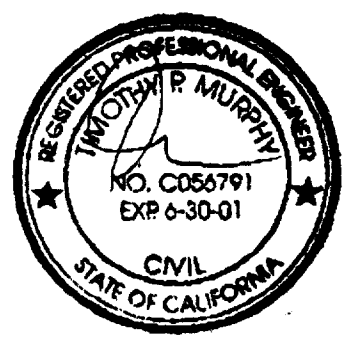
**BRACING**  
 TOP CHORD Sheathed or 6-0-0 on center purlin spacing.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

**REACTIONS (lbs/size)** 1=3841/0-3-8, 8=4190/0-3-8

**FORCES (lb) - First Load Case Only**  
 TOP CHORD 1-2=8860, 2-3=7324, 3-4=7298, 4-5=7494, 5-6=10203  
 BOT CHORD 1-10=8466, 9-10=6466, 8-9=6704, 7-8=9657, 7-11=9657, 6-11=9657  
 WEBS 2-10=1058, 2-9=1884, 3-9=2004, 3-8=849, 4-8=1648, 5-8=2788, 5-7=1883

- NOTES**
- 3-ply truss to be connected together with 0.131"x3" Nails as follows: Top chords connected with 1 row(s) at 0-9-0 on center. Bottom chords connected with 3 row(s) at 0-4-0 on center. Webs connected as follows: 2 X 4 - 1 row(s) at 0-8-0 on center.
  - This truss has been checked for unbalanced loading conditions.
  - Except as shown below, special connection(s) required to support concentrated load(s). Design of connection(s) is delegated to the building designer.
  - Provide adequate drainage to prevent water ponding.
  - All plates are M20 plates unless otherwise indicated.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 18-B, UBC-94.
  - This truss has been designed with ANSI/TPI 1-1995 criteria.

**LOAD CASE(S) Standard**  
 1) Regular: Lumber Increase=1.16, Plate Increase=1.16  
 Uniform Loads (psf)  
 Vert: 1-2=-80.0, 2-3=-80.0, 3-4=-80.0, 4-5=-80.0, 5-8=-80.0, 1-10=-452.6, 8-10=-452.6, 8-9=-452.6, 7-8=-452.6  
 7-11=-10.0, 6-11=-10.0  
 Concentrated Loads (lb)  
 Vert: 11=-2194



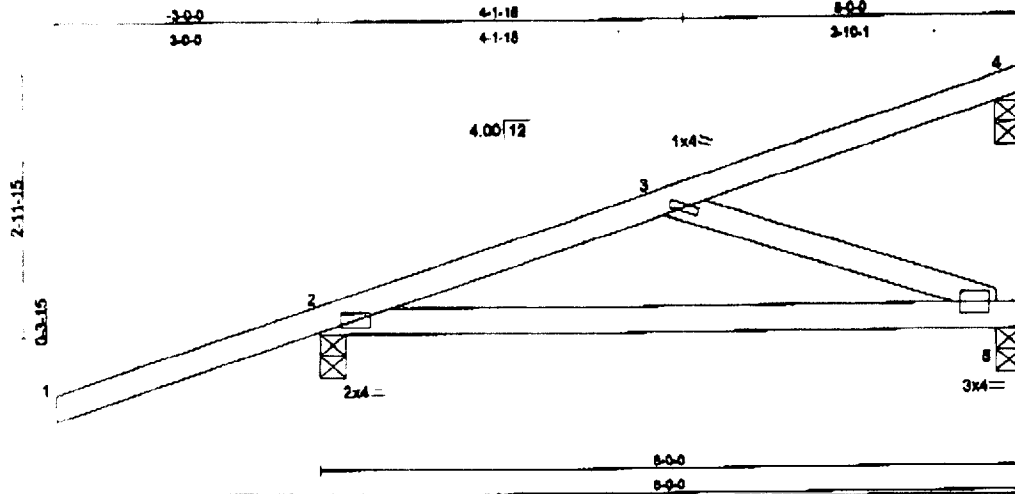
April 1, 1999

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult Q&T-88 Quality Standard, OSB-88 Bracing Specification, and MS-81 Handling, Installing and Bracing Recommendation available from Truss Plate Institute, 583 O'Donofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	Neubauer / Hagen	R1327162
3172GS	CHJ1	JACK	5	1		

Pacerville Truss, Pacerville, CA 95867

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<b>LOADING (pcf)</b>	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> (in) (loc) Udefl	<b>PLATES GRIP</b>
TCLL 20.0	Plates Increase 1.15	TC 0.61	Vert(LL) -0.17 2-3 >529	M20 185/144
TCDL 10.0	Lumber Increase 1.15	BC 0.36	Vert(TL) 0.31 1-2 >123	
BCLL 0.0	Rep Stress Incr NO	WB 0.08	Horz(TL) 0.00 5 n/a	
BCDL 5.0	Code UBC/ANSI95	(Matrix)	1st LC LL Min Udefl = 350	Weight: 27 lb

**LUMBER**  
 TOP CHORD 2 X 4 SPF M 19  
 BOT CHORD 2 X 4 SPF M 19  
 WEBS 2 X 4 HF Std

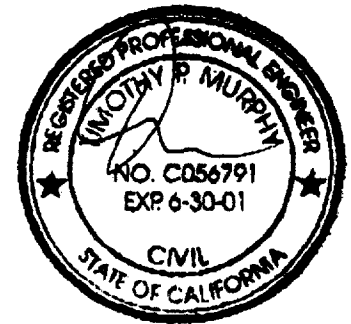
**BRACING**  
 TOP CHORD Sheathed or 6-0-0 on center purlin spacing.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

**REACTIONS (lb/size)** 4=110/0-3-8, 2=405/0-3-8, 5=122/0-3-8  
 Max Grav 5=141(load case 2)

**FORCES (lb) - First Load Case Only**  
 TOP CHORD 1-2=58, 2-3=314, 3-4=27  
 BOT CHORD 2-5=238  
 WEBS 3-5=253

- NOTES**
- 1) All plates are M20 plates unless otherwise indicated.
  - 2) This truss has been designed for a 10.0 pcf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
  - 3) This truss has been designed with ANSI/TPI 1-1995 criteria.

**LOAD CASE(S)** Standard



April 1, 1999

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult GST-88 Quality Standard, DSB-88 Bracing Specification, and HIB-91 Handling, Installing and Bracing Recommendation available from Truss Plate Institute, 585 D Onafrio Drive, Madison, WI 53719

MiTek Industries, Inc.