CITY OF SACRAMENTO

Permit No: 9806955

1231 I Street, Sacramento, CA 95814

Insp Area: 1

Site Address: 6517 ELVAS AV SAC
Parcel No: 0080321013

Sub-Type: AOTHR
Housing (Y/N): N

CONTRACTOR
J P LEONARD CONCRETE
2531 WATSON ST
SACRAMENTO, CA 95821

OWNER
PALMISANO DOROTHY G
SACRAMENTO CA
00000

ARCHITECT

Nature of Work: CONSTRUCT 9FT HIGH MASONRY FENCE AT REAR OF PROPERTY.

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name_____ Lender'sAddress_____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class C 8 License Number 536207 Date 7/23/96 Contractor Signature _____

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars ($500.00);

_____ I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

_____ I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

_____ I am exempt under Sec._____ B & PC for this reason:_____

Date_____ Owner Signature_____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and herby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 7/23/96 Applicant/Agent Signature _____

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

_____ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

_____ I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier NIP Employ RD Policy Number_____

_____ (This section need not be completed if the permit is for $100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 7/23/96 Applicant Signature _____

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS ($100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

6001 ELVAS.
008 0321 013

City of Sacramento Development Services Division
Planning and Zoning Information Request

Project Address: 6079 ELVAS AVE

Assessor's Parcel Number: 008 - 0321 - 013

Current Land Use: industrial

Description of Request/Proposed Use:

new 9 foot masonry wall

Zoning Designation: M-1

Prior Applications for Project Site(P#,Z#,DRPB#):

Comments: not associated w/ commercial - industrial development (zoning) do not involve planning entitlements

Are There Any Planning Issues?: (Circle One) YES (NO)

Site Plan Check Required? (Circle One) YES NO

Design Review/ Preservation Required?: (Circle One) YES (NO)

Planning Review by/Date: [signature] 7.23.99

A list of items that must be reviewed by Planning is provided on the reverse side of this form.

CAPITOL ENGINEERING LABORATORIES, INC.

Materials Testing • Inspection • Crane Certification

CONCRETE SPECIMEN FIELD DATA (ASTM C39)

Client: AJM BODY SHOP, ATTN: JOHN McMURRAY
Address: 6525 ELVIS AVE.
SACRAMENTO, CA. 95819

Project:
Contract No.:
Project No: 4932
Sample Location: TOP OF WALL

Sample Date: 09-08-98
Time Cast: -
Receive Date: 09-10-98

By: M.C.
Slump(in.): 9.5"
Temp: Air-78 Mix-76
Entrained Air(%): -

Concrete Mix No.: 2500
Design Strength(psi): 2000
Supplier: LIVINGSTON
Truck/Ticket No.: -

Specimen Size: 4X8
Admixtures: -

Specimen	Test Date	Age	Load	Area	Strength	Type of Fracture
4A	09/15/98	7	14119	12.57	1120	
5C	10/06/98	28	26880	12.57	2140	
6C	10/06/98	28	26815	12.57	2130	

Meets Specified Strength ✓ Does not meet Specified Strength __

Remarks: GROUT

These tests were performed under the direction of the undersigned. If sampled by our representative, ASTM C172, C143, & C31 are utilized. Spare specimens will be retained only if strength is below specified.

Reported By Barry Lotz
Barry Lotz
Engineer

631 Commerce Drive • Roseville, CA 95678 • (916) 786-2488 • FAX (916) 786-9372

CAPITOL ENGINEERING LABORATORIES, INC.

Materials Testing • Inspection • Crane Certification

CONCRETE SPECIMEN FIELD DATA (ASTM C39)

Client: AJM BODY SHOP, ATTN: JOHN McMURRAY
Address: 6525 ELVIS AVE.
SACRAMENTO, CA. 95819

Project:
Contract No.:
Project No: 4932
Sample Location: LOWEST LIFT

Sample Date: 09-01-98
Time Cast: -
Receive Date: 09-09-98

By: OTHERS
Slump(in.): -
Temp: Air-- Mix--
Entrained Air(%): -

Concrete Mix No.: -
Design Strength(psi): 2000
Supplier: -
Truck/Ticket No.: -

Specimen Size: 6X12
Admixtures: -

Specimen	Test Date	Age	Load	Area	Strength	Type of Fracture
1B	09/15/98	14	33028	28.3	1340	A
2C	09/29/98	28	44989	28.3	1590	A
3X	10/27/98	56	57843	28.3	2040	A

Meets Specified Strength ✓ @56 Days Does not meet Specified Strength __

Remarks: GROUT
SAMPLED IN PLASTIC MOLDS

These tests were performed under the direction of the undersigned. If sampled by our representative, ASTM C172, C143, & C31 are utilized. Spare specimens will be retained only if strength is below specified.

Reported By Barry Lotz
Barry Lotz
Engineer

631 Commerce Drive • Roseville, CA 95678 • (916) 786-2488 • FAX (916) 786-9372

Trib Truss Engineering, Inc.
All bars require minimum 2.5" cover.

TITLE: Wall (no load above)
DESCR: 6525 Elvas, Sacto

JOB #: Dyn298 BY:ct @ 2/11/98

CANTILEVERED RETAINING WALL DESIGN

SOIL DATA

Allowable Bearing	=	1,000 psf
Active Lateral	=	30.0 psf
..Max. Active	=	0 "
...Slope Active	=	43.0 "
Backfill Slope (horiz:vert, 0=Level)	=	0.0:1
Passive Lateral	=	250 "
Soil Density	=	110.0 pcf
Soil Ht over Toe	=	6.0 in

ADDED VERTICAL LOADS

Axial DL on Stem	=	100 plf
Axial LL on Stem	=	360 plf
...Ecc. (Toe side +)	=	4.00 in
Surcharge over Toe	=	0.0 psf
Surcharge over Heel	=	0.0 "
Using Heel Surcharge to resist overturning ?		Yes
Using 1/3 vertical ?		Yes

ADDED LATERAL LOADS

Lateral Load Acting On Stem Above Soil	=	15.00 psf
Add'l Lateral Load	=	0.0 plf
...dist. to start	=	0.00 ft
dist. to end	=	0.00 ft

Adjacent Footing:

Vertical load	=	0 plf
Footing Width	=	0.00 ft
Ftg. CL to Wall	=	0.00 ft
Ftg. Base Above/Below Soil At Wall Face [+/-]	=	0.00 ft
Footing Type	:	Line

WALL & FOOTING DATA

Retained Height	=	0.50 ft
Wall Ht. above soil	=	8.00 ft
Key Depth	=	0.00 in
Key Width	=	0.00 in
Key Dist. to Toe	=	0.00 ft
Toe Width	=	1.00 ft
Heel Width	=	1.50 ft
Total Width	=	2.50 ft
Thickness	=	8.00 in

SUMMARY

Pressure @ Toe	=	863 psf	Factors of Safety:		
Pressure @ Heel	=	147 "	Overturning	=	2.93 :1
Allowable Press.	=	1,000 "	Sliding	=	4.26 :1
Eccentricity	=	3.54 in			
1-Way Shear @ Toe	=	10.8 psi	Allowable Shear	=	76.0 psi
1-Way Shear @ Heel	=	1.8 psi			

SLIDING CHECK

Ftg/Soil Friction	=	0.300	Lateral Pressure	=	128.8 #
Factor of Safety	=	4.26	(-)Passive Pressure	=	170.1
Add'l Force Req'D	=	0 #	(-)Friction	=	378.7

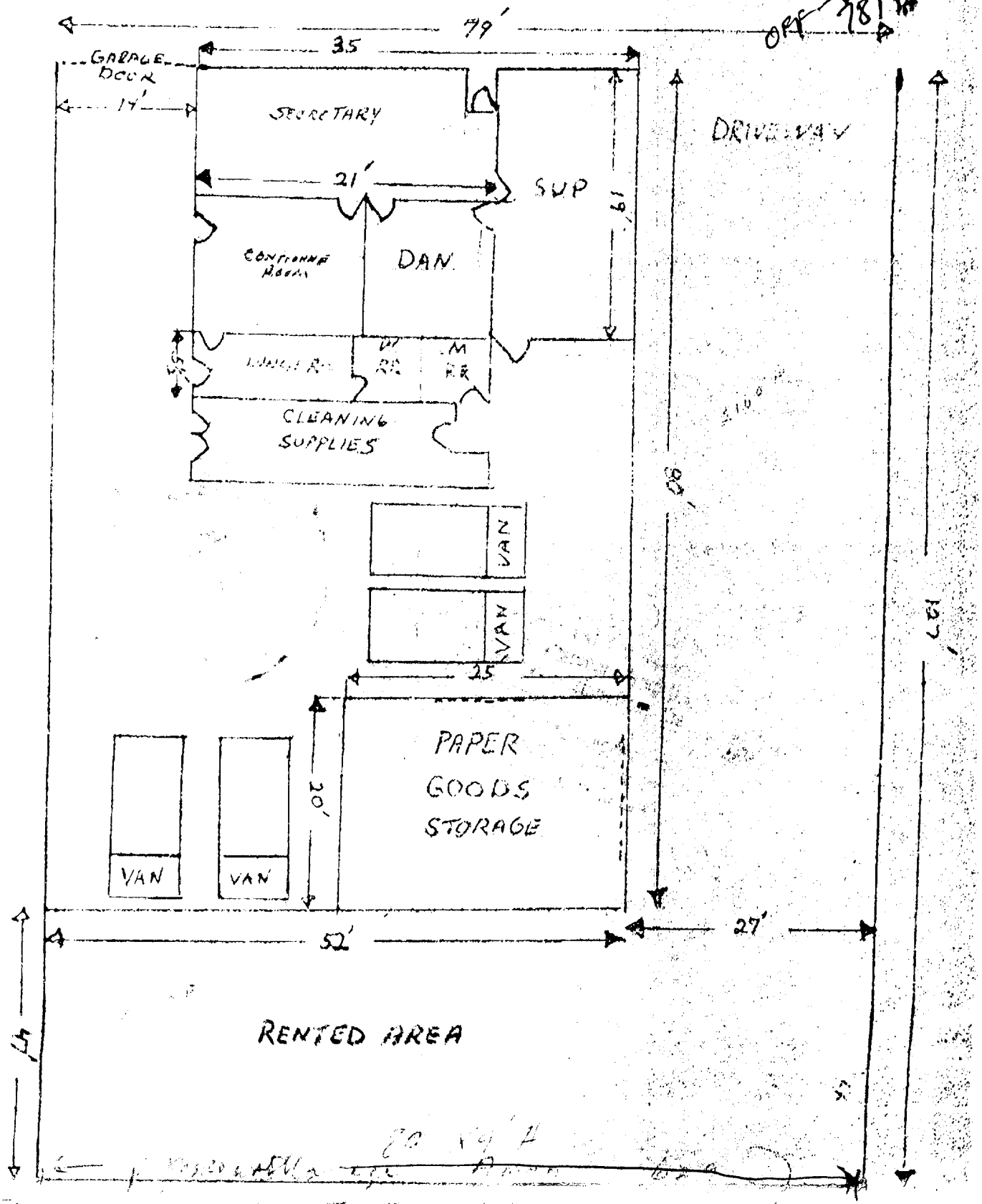
FOOTING DESIGN

		Toe	Heel				
ACI 9.1 Pressure	=	1,714	292 psf	f'c	=	2,000 psi	
Mu - Upward	=	762	0 ft-#	Fy	=	60,000 psi	
Mu - Downward	=	120	38	Using SP @ Heel ?		No	
Mu - Design	=	642	38	Rebar Choices			
One-Way Shear:				-- Toe --		-- Heel --	
Actual	=	10.83	1.83 psi	#4 @	20.20 in o.c.	20.20	
Allowable	=	76.03	76.03	#5 @	31.31	31.31	
Rebar Cover	=	2.50	2.50 in	#6 @	44.44	44.44	
Depth to steel	=	5.50	5.50	#7 @	48.00	48.00	
Ru = Mu/bd^2	=	23.6	1.4	#8 @	48.00	48.00	
Min Rebar %	=	0.0018		#9 @	48.00	48.00	
				#10@	48.00	48.00	

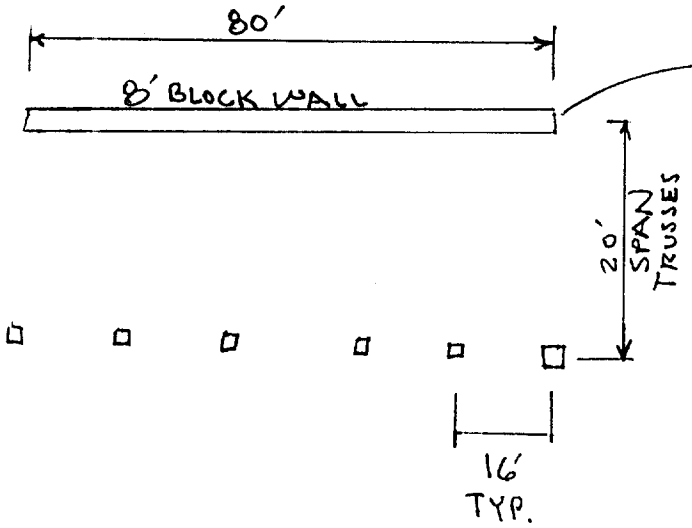
GST CLUB AUB

SACRAMENTO BRANCH

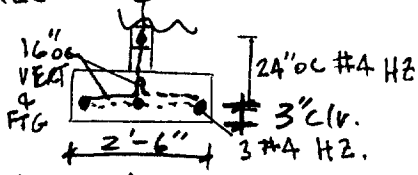
BLDG → 4160 ☐
LOT → 10033 ☐
OFF → 381 ☐



DYNAMIC CONST. 731-4005
6525 ELVAS, SACRAMENTO
1000 psf SOIL
16/7/0/10 PSF @ 1.25



$f_m' = 1{,}500$ psi
Grout strength 2,000 psi
2'6" x 8" deep footing
(see CALC)
16" oc #4 BAR VERTICAL
AND HEEL/TOE.
HORIZ THREE #4 FOOTING
24" OC WALL
16" LAPS



* solid Grout ALL cells

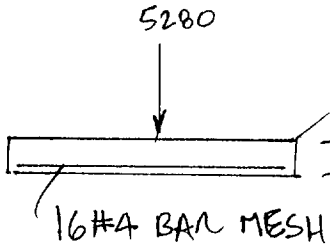
16' BEAM $\frac{20'}{2} \times 33$ PSF = 330 plf $M = 10560$ LB

USE 4x14 SSDF, 6x12 #1DF, $3\frac{1}{8} \times 13\frac{1}{2}$ 24F-V8
$5\frac{1}{8} \times 12$ 24F-V8

POST: $330 \times 16 = 5280$ LBS

4x4 #1DF $P_c = 1670$ $C_p = .2992$ $F_c' = 678$ 8' $P_{MAX} = 7651 > 5280$ OK.

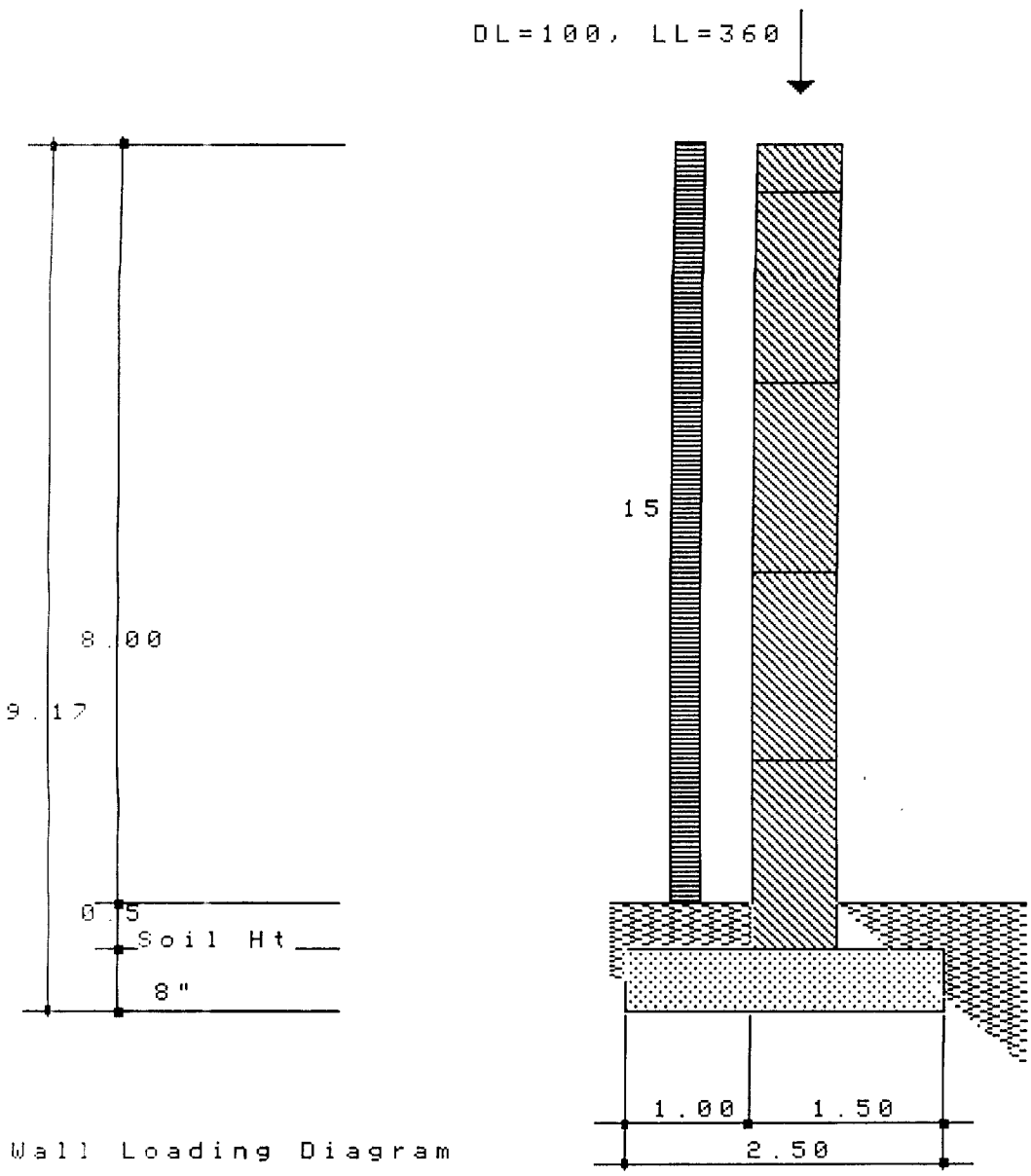
PAD FOOTING:



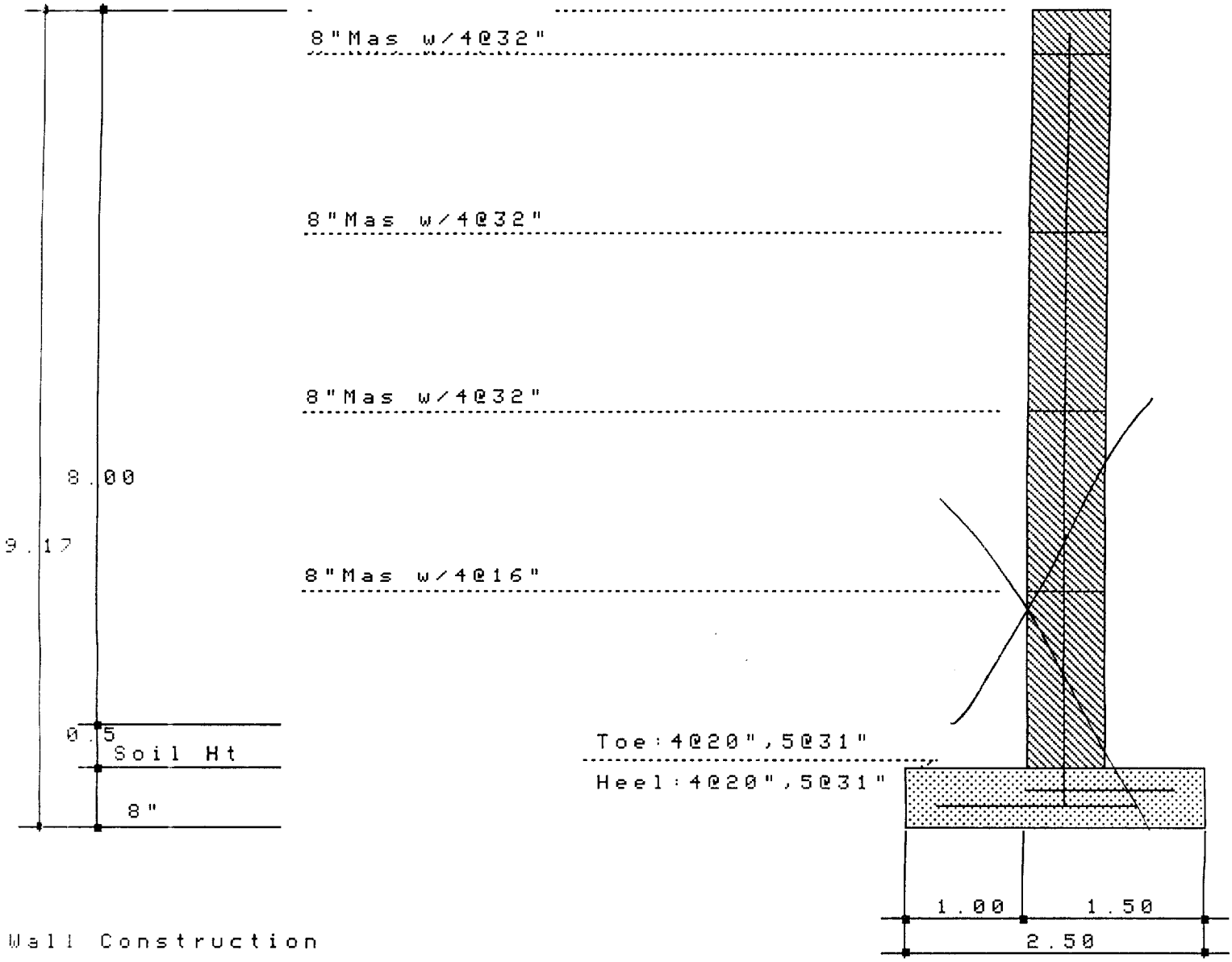
* $f_c' = 2{,}500$ psi
use Min. 5 sacks of cement

$f_y = 60{,}000$ psi for re bars

2/23/98
EXP 3/98



Wall Loading Diagram



Wall Construction

Tru-Truss Engineering, Inc.
All bars require minimum 2.5" cover.

TITLE: Wall (no load above)
DESCR: 6525 Elvas, Sacto

JOB #: Dyn298 BY:ct @ 2/11/98

STEM DESIGN

Wall Info....	< Stem Designs At Various Heights Above Ftg. >					
DESIGN HT. ABOVE FTG. =	8.00	6.00	4.00	2.00	0.00	ft
WALL TYPE ABOVE HT. :	Masonry	Masonry	Masonry	Masonry	Masonry	
Thickness (nominal) =	8.00	8.00	8.00	8.00	8.00	in
Rebar Size :	# 4	# 4	# 4	# 4	# 4	
Rebar Spacing =	32.00	32.00	32.00	32.00	16.00	in
Rebar Placed at :	Center	Center	Center	Center	Center	
DESIGN DATA						
Lateral Load @ Ht. =	468	498	528	558	582	#
MOMENT..... Capacity =	404	404	404	404	521	ft-#
Applied =	151	106	1	164	387	"
SHEAR..... Applied =	7.95	8.46	8.97	9.48	8.36	psi
Interaction Value =	0.351	0.232	0.035	0.450	0.786	
Wall Weight =	51.0	51.0	51.0	51.0	60.0	psf
Rebar Depth =	3.75	3.75	3.75	3.75	3.75	in
MASONRY DATA						
f'm =	1,500	1,500	1,500	1,500	1,500	psi
Fs =	24,000	24,000	24,000	24,000	24,000	psi
Grouting :	Part	Part	Part	Part	Part	
Special Inspection :	No	No	No	No	No	
n : Es / Em =	25.78	25.78	25.78	25.78	25.78	
Tension Embed=.004DbFs =	24.00	24.00	24.00	24.00	24.00	
Short Term Increase =	1.00	1.00	1.00	1.00	1.00	
CONCRETE DATA						
f'c =						psi
Fy =						psi

SUMMARY OF FORCES & MOMENTS

	Overturning Moments #	ft	ft-#	Resisting Moments #	ft	ft-#
Origin of Force:						
Heel Active Press. =	29	0.39	11			
Soil over Heel =				46	2.08	95
Toe Active Press. =	-20	0.39	-8			
Soil over Toe =				55	0.50	28
Sloped Soil @ Heel =					2.22	
Adjacent Ftg. Load =	0	0.00	0	0	0.00	0
Surcharge @ Heel =				0	0.00	0
Surcharge @ Toe =	0	0.00	0	0	0.00	0
Axial Load on Wall =				460	1.67	767
Load @ Proj. Wall =	120	5.17	620			
Averaged Stem Wts. =				452	1.33	602
Added Lateral Load =	0	0.00	0			
Ftg & Key Weight =				250	1.25	312
1/3 Active Pressure=				10	2.50	24
TOTALS =	129		623	1,262		1,829

---->> Distances from front edge of footing bottom (not key)

Tru-Truss Engineering, Inc.
All bars require minimum 2.5" cover.

TITLE: Wall (no load above)
DESCR: 6525 Elvas, Sacto

JOB #: Dyn298 BY:ct @ 2/11/98

CANTILEVERED RETAINING WALL DESIGN

SOIL DATA

Allowable Bearing	=	1,000	psf
Active Lateral	=	30.0	psf
....Max. Active	=	0	"
.....Slope Active	=	43.0	"
Backfill Slope (horiz:vert,0=Level)	=	0.0:1	
Passive Lateral	=	250	"
Soil Density	=	110.0	pcf
Soil Ht over Toe	=	6.0	in

ADDED VERTICAL LOADS

Axial DL on Stem	=	100	plf
Axial LL on Stem	=	360	plf
...Ecc. (Toe side +)	=	4.00	in
Surcharge over Toe	=	0.0	psf
Surcharge over Heel	=	0.0	"
Using Heel Surcharge to resist overturning ?		Yes	
Using 1/3 vertical ?		Yes	

ADDED LATERAL LOADS

Lateral Load Acting On Stem Above Soil	=	15.00	psf
Add'l Lateral Load	=	0.0	plf
...dist. to start	=	0.00	ft
dist. to end	=	0.00	ft

Adjacent Footing:

Vertical load	=	0	plf
Footing Width	=	0.00	ft
Ftg. CL to Wall	=	0.00	ft
Ftg. Base Above/Below Soil At Wall Face [+/-]	=	0.00	ft
Footing Type	:	Line	

WALL & FOOTING DATA

Retained Height	=	0.50	ft
Wall Ht. above soil	=	8.00	ft
Key Depth	=	0.00	in
Key Width	=	0.00	in
Key Dist. to Toe	=	0.00	ft
Toe Width	=	1.00	ft
Heel Width	=	1.50	ft
Total Width	=	2.50	ft
Thickness	=	8.00	in

SUMMARY

Pressure @ Toe	=	863	psf
Pressure @ Heel	=	147	"
Allowable Press.	=	1,000	"
Eccentricity	=	3.54	in
1-Way Shear @ Toe	=	10.8	psi
1-Way Shear @ Heel	=	1.8	psi
Allowable Shear	=	76.0	psi

Factors of Safety:

Overturning	=	2.93 :1
Sliding	=	4.26 :1

SLIDING CHECK

Ftg/Soil Friction	=	0.300	
Factor of Safety	=	4.26	
Add'l Force Req'D	=	0	#
Lateral Pressure	=	128.8	#
(-)Passive Pressure	=	170.1	
(-)Friction	=	378.7	

FOOTING DESIGN

		Toe	Heel	
ACI 9.1 Pressure	=	1,714	292	psf
Mu - Upward	=	762	0	ft-#
Mu - Downward	=	120	38	
Mu - Design	=	642	38	
One-Way Shear:				
Actual	=	10.83	1.83	psi
Allowable	=	76.03	76.03	
Rebar Cover	=	2.50	2.50	in
Depth to steel	=	5.50	5.50	
Ru = Mu/bd^2	=	23.6	1.4	
Min. Rebar %	=	0.0018		

f'c	=	2,000 psi
Fy	=	60,000 psi
Using SP @ Heel ?		No

Rebar Choices

	Toe	Heel
#4 @	20.20 in o.c.	20.20
#5 @	31.31	31.31
#6 @	44.44	44.44
#7 @	48.00	48.00
#8 @	48.00	48.00
#9 @	48.00	48.00
#10@	48.00	48.00

Job#>F0220 Live load = 16 psf
02-23-1998 Dead load = 17 psf

Maximum bearing capacity of soil = 1000 psf
Concrete f'c = 2000 psi
Steel reinforcement Fy = 36000 psi
Minimum bearing wall requirements (supporting 1 floors):
6"stem wall, 12"wide, 6"thick, 12"below undisturbed soil surface

Design Concentrated Live Load plus Dead Load (beam reaction) = 5280 Lbs

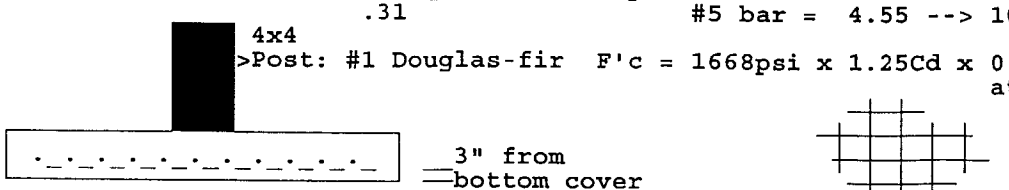
Weight of footing = 150 pcf x 12"deep / 12"/ft = 150 psf
Weight of 8' wall = 85 plf /12" x 12"/ft = 85 psf
Net allowable pressure = 1000 -150 -85 = 765 psf
Area of footing required = 5280 lbs/ 765 psf = 6.9 square ft
Calc a 30" x 30" footing beneath concentrated load;
Required Strength: (for LL = 16 psf DL = 17 psf)
5280 lbs ---> 2560 lbs LL 2720 lbs DL
U = 1.7 x 2560 + 1.4 x 2720 = 8160 lbs
Net pressure under footing = 8160 / 30 ^2 x 144 = 1306 psf

Shear stress effective depth = 12 - 3" cover - 1/2" bar diameter = 8.5"
Punching Shear = 1306 psf x (6.25 -1) = 6857
6857 / .85 / 4 / 12 / 8.5 = 20 psi less than 178 psi = 4(f'c)^.5 Ok
No shear reinforcement required.

Wide Beam Shear:
1306 x 2.5 x (15 -1.75 -8.5)/12 = 1292 lbs
1292 / .85 / 2.5 / 12 / 8.5 = 6 psi less than 89 psi = 2(f'c)^.5 Ok

Nominal Ultimate Bearing Stress:
8160 / .7 / 8.5 / 8.5 = 161 psi less than 1700 = .85 f'c Ok
Transfer by bearing alone is adequate.

Design for critical moment at face of column:
.5 x 1306 x 2.5 (15 -1.75)^2 / 144 = 1990 ft-lbs
1990 x 12 / .9 / Fy / 8.5 = .08 square inches
Minimum Steel Area = 200 / Fy x 30 x 8.5 = 1.41 square inches
Bar quantity = 1.41 / .2 square inches per #4 bar = 7.05 --> 16 bar mesh
.31 #5 bar = 4.55 --> 10 bar mesh



>Post: #1 Douglas-fir F'c = 1668psi x 1.25Cd x 0.21Cp = 432psi
at height = 131"

#1DF Post
Hgt Size
Use
16 #4 bar mesh for 30" square footing 12" deep supporting 5280 Lbs on 131" 4x4

All pad footings to be placed 24" into undisturbed soil

GENERAL NOTES: (Unless otherwise specified)
This individual building component is designed in accordance with the latest editions of NDS & TPI specifications and is to be used in a building system designed by others. Metal connectors are to be of prime quality galvanized sheet steel in accordance with Uniform Building Code Standard 25-17 and must be fully embedded into each truss face centered on the joint. Design assumes adequate drainage and a Dry-Condition use in a Non-Corrosive environment without the use of Fire-Retardant or Preservative-Treated Lumber. Top Chord is to be continuously sheated (or braced with purlins at the indicated spacing) and the Bottom chord is to be braced at a maximum spacing of 10'0" o.c. when no ceiling materials exist. Shim or wedge if necessary to provide full bearing area required. Indicated chord splice shall be ±6" within 1/4 of panel. Cut all members to bear tightly against each other. Installation is entirely the responsibility of the contractor. All bracing, temporary and permanent, is the responsibility of others. For additional information, refer to QUALITY CONTROL MANUAL, BRACING WOOD TRUSSES, and RECOMMENDED CODE OF STANDARD PRACTICE, all available from the TRUSS PLATE INSTITUTE, Madison, Wisconsin.

TRU-TRUSS ENGINEERING
1783 Arroyo Vista Way
El Dorado Hills, CA 95762

6579 ELVAS
008 C321 013

City of Sacramento Development Services Division
Planning and Zoning Information Request

Project Address: 6579 ELVAS AVE

Assessor's Parcel Number: 008 - C321 - 013

Current Land Use: [illegible]

Description of Request/Proposed Use:

9 foot masonary wall

Zoning Designation: M-1

Prior Applications for Project Site(P#,Z#,DRPB#): _____

Comments: [illegible] associated w/ Commercial development (zoning?) do not require planning entitlements

Are There Any Planning Issues?: (Circle One) YES (NO)

Site Plan Check Required? (Circle One) YES NO

Design Review/ Preservation Required?: (Circle One) YES (NO)

Planning Review by/Date: [illegible] 7-23-98

A list of items that must be reviewed by Planning is provided on the reverse side of this form.