

CITY OF SACRAMENTO

Permit No: 9807871

1231 I Street, Sacramento, CA 95814

Insp Area: 2

Site Address: 2 JIB CT SAC
Parcel No: 0300130010

6698 Gloria Dr

Sub-Type: RES
Housing (Y/N): N

CONTRACTOR

ZIMMERMAN ROOFING
3560 RAMONA AV
SACRAMENTO, CA 95826

OWNER

ZAMPELLA MARK ALLEN
6698 GLORIA DR
SACRAMENTO CA 95831

ARCHITECT

Nature of Work: REROOF

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class C-39 License Number 557559 Date 8-14-98 Contractor Signature Billy Coy

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. _____ B & PC for this reason: _____
Date _____ Owner Signature _____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 8-14-98 Applicant/Agent Signature Billy Coy

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:
Carrier State Fund Policy Number 10-98 713 970002021 EXP. 10-1-98

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 8-14-98 Applicant Signature Billy Coy

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.



DEPARTMENT OF
PLANNING AND DEVELOPMENT

CITY OF SACRAMENTO
CALIFORNIA

1231 I STREET
ROOM 200
SACRAMENTO, CA
95814-2998

Permit Service
916-264-7619
FAX 916-264-7046

Mark Zampella
2 Tib Ct
95831

TILE ROOF WORKSHEET

This worksheet must be filled out whenever any type of tile roof is applied for.

If the answer to question #5 is yes, a written engineering report from a registered engineer must be provided with each application.

1. BRAND AND MODEL OF TILE Pioneer Shake tile
2. TILE WEIGHT PER SQUARE 1730
3. WEIGHT OF ROOF SYSTEM PER SQUARE 180
4. TOTAL WEIGHT OF ROOF SYSTEM 810
5. DOES TOTAL WEIGHT OF ROOF SYSTEM EXCEED 750# PER SQUARE? YES ~~NO~~
6. ROOF SLOPE _____

PLEASE PROVIDE A SEPARATE WORKSHEET FOR EACH APPLICATION INVOLVING A TILE ROOF.

all attached
engineering report

Zampella

Paul Zacher-Structural Engineers

4701 Lakeside Way
Fair Oaks, CA 95628

TEL: 916.961.3960
FAX: 916.961.3960
e-mail: pzacher@softcom.net

August 10, 1998

Zimmerman Roofing
3560 Ramona Avenue
Sacramento, CA 95826
TEL: 916.454.3667
FAX: 916.455.3784
TEL (Jeff): 916.392.1971
FAX (Jeff): 916.392.6853
FAX (Framer) : 916.383.5308

Attn.: Mr. Jeff Tucker,

re: Job 98173: ZAMPELLA

Subject: Structural Investigation Report of the Roofs for the Residences located at 2 Jib Court and 6698 Gloria Drive, Sacramento, CA 95831.

As requested by Mr. Jeff Tucker, this is a report to determine what needs should be addressed to correct any structural deficiencies of the roof. Paul Zacher visited the site August 3, 1998. The investigation was made to determine the existing condition of the structure. All information, data and analysis contained within this report is based on the 1994 Uniform Building Code.

The following is based on visual observations with no subsurface investigation being made.

DESCRIPTION:

Type of Facility: Residence.
Year Built: Estimated 1970's vintage.
Occupancy: Residential.
No. of Stories: Two.
Dimensions: Approximately 3500 square feet with a first story plate height of 8 feet.

CONSTRUCTION:

Roof:

The roof covering will consist of Pioneer Everwest Light Weight Concrete Tile over 1/2" solid sheathing. The living area is framed with both pre-engineered trusses spaced at 24" on center and with 2x6 rafters spaced at 24" on center with 2x6 purlins supported at no more than 6'-0" on center by 2x4 struts bearing on walls below. The garage area over hang was unaccessable and was not inspected.

1/14

John Tang

CONCLUSIONS:

Roof:

The living area lacks sufficient structural capacity for the applied live and dead loads. . The garage area over hang was unaccessable and therefore no conclusions are made..

RECOMMENDATIONS:

If any of the following recommendations do not correspond to actual field conditions, the engineer of record shall be notified for further investigation and evaluation before continuing work.

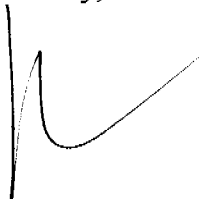
Living Area:

1. Add a 2x6 DF#2 x 8'-0" long purlin and support it to the bearing walls below with a 2x4 strut. The unbraced length of the struts shall not exceed 8'-0" and the minimum slope of the struts shall not be less than 45 degrees from the horizontal . See detail 1.
2. Provide additional 2x4 struts from the existing purlins or rafters to the bearing walls below. The maximum spacing between the new and existing struts shall not exceed 6'-0" on center. The unbraced length of the struts shall not exceed 8'-0" and the minimum slope of the struts shall not be less than 45 degrees from the horizontal.
3. Scab a 2x6 rafter to the existing 2x6 rafters with 16d's @ 12" on center where the span is greater than 12'-0".
4. Scab a 2x4 rafter to the existing 2x4 rafters with 16d's @ 12" on center where the span is greater than 8'-0".

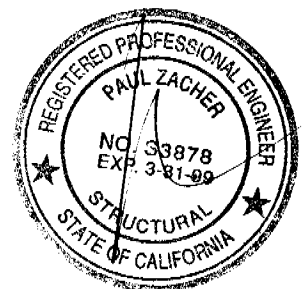
The inspection consisted of visual observation only, made solely to determine the structural capacity of the existing roof. Analysis does not determine any effects on the overall structure under lateral forces or effects on the foundation unless specifically noted in the calculations and in this document. No warranties, expressed or implied, are made or intended in conjunction with this report. The inspection was made only to the portions that were accessible. The specific items noted were those that were observable and there may be defects which are not observable, or are hidden by architectural and structural materials.

If you have any questions on the above, do not hesitate to call.

Sincerely,



Paul Zacher, P.E., S.E.
file



DESIGN LOADING:

| | | |
|-------------------------|------|-------|
| Roof Pitch | 4 | in 12 |
| Pitch Adjustment Factor | 1.05 | |

LOCATION: ROOF

| <u>MATERIAL</u> | | <u>WEIGHT</u> |
|-----------------------|-------------|---------------|
| Monier Villa Duralite | 5.80 | psf |
| Roofing felt | 0.30 | psf |
| 1x4 skip sht'g | 1.09 | psf |
| 1/2" OSB/ plywood | 1.50 | psf |
| 2x6 rafters @ 24" oc | <u>1.00</u> | psf |
| | Load | 9.7 psf |
| Roof Pitch Adjustment | <u>0.52</u> | psf |
| Total Load | 10.2 | psf |

LOCATION: VAULT

| <u>MATERIAL</u> | | <u>WEIGHT</u> |
|-----------------------|-------------|---------------|
| Monier Villa Duralite | 5.80 | psf |
| Roofing felt | 0.30 | psf |
| 1/2" OSB/ plywood | 1.50 | psf |
| 1x4 skip sht'g | 1.09 | psf |
| 2x8 rafters @ 24" oc | 1.32 | psf |
| Batt/blown insul | 0.50 | psf |
| 1/2" Gypboard | <u>2.50</u> | psf |
| | Load | 13.0 psf |
| Roof Pitch Adjustment | <u>0.70</u> | psf |
| Total Load | 13.7 | psf |

BEAM DESIGN FOR UNIFORM LOAD: 2-2x4

(Values for DF Larch #2)

| | |
|-------------------------------------|------------|
| Width, b | 3 inches |
| Depth, d | 3.5 inches |
| Length of beam | 9.75 feet |
| Dead load roof | 11.5 psf |
| Live load roof | 16 psf |
| Contributory width of roof load | 2 feet |
| Dead load floor | 0 psf |
| Live load floor | 0 psf |
| Contributory width of floor load | 0 feet |
| Dead load wall | 0 plf |
| Live load defl ratio | 240 |
| Total load defl ratio | 180 |
| Total dead load | 23 plf |
| Total live load | 32 plf |

Base design values:

| | |
|--------------------------|-------------|
| Shear, Fv | 95 psi |
| Bending, Fb | 875 psi |
| Comp. perp. to grain, Fc | 625 psi |
| Mod of Elasticity, E | 1600000 psi |
| Load duration factor, Cd | 1.25 |
| Size Factor, Cf | 1.50 |
| Repetitive factor, Cr | 1.15 |

| | |
|---------------------|---------|
| Dead load reaction | 112 lbs |
| Live load reaction | 156 lbs |
| Total load reaction | 268 lbs |

| | |
|---------------------------|-------------|
| Allowable shear, Fv' | 119 psi |
| Actual shear, fv | 36 psi |
| Allowable bending, Fb' | 1887 psi |
| Actual bending, fb | 1280 psi |
| Allowable live load defl | 0.49 inches |
| Actual live load defl | 0.38 inches |
| Allowable total load defl | 0.65 inches |
| Actual total load defl | 0.65 inches |

Bearing length req'd 0.14 inches

Horizontal Shear OK

Bending OK

Live Load Deflection OK

Beam Fails under Total Load Deflectio
OK

BEAM DESIGN FOR UNIFORM LOAD: 2x6

(Values for DF Larch #2)

| | |
|-------------------------------------|------------|
| Width, b | 1.5 inches |
| Depth, d | 5.5 inches |
| Length of beam | 12 feet |
| Dead load roof | 11.5 psf |
| Live load roof | 16 psf |
| Contributory width of roof load | 2 feet |
| Dead load floor | 0 psf |
| Live load floor | 0 psf |
| Contributory width of floor load | 0 feet |
| Dead load wall | 0 plf |
| Live load defl ratio | 240 |
| Total load defl ratio | 180 |
| Total dead load | 23 plf |
| Total live load | 32 plf |

Base design values:

| | |
|--------------------------|-------------|
| Shear, Fv | 95 psi |
| Bending, Fb | 875 psi |
| Comp. perp. to grain, Fc | 625 psi |
| Mod of Elasticity, E | 1600000 psi |
| Load duration factor, Cd | 1.25 |
| Size Factor, Cf | 1.30 |
| Repetitive factor, Cr | 1.15 |

| | |
|---------------------|---------|
| Dead load reaction | 138 lbs |
| Live load reaction | 192 lbs |
| Total load reaction | 330 lbs |

| | | | |
|---------------------------|-------------|-----------------------|----|
| Allowable shear, Fv' | 119 psi | Horizontal Shear | OK |
| Actual shear, fv | 55 psi | | |
| Allowable bending, Fb' | 1635 psi | Bending | OK |
| Actual bending, fb | 1571 psi | | |
| Allowable live load defl | 0.60 inches | Live Load Deflection | OK |
| Actual live load defl | 0.45 inches | | |
| Allowable total load defl | 0.80 inches | Total Load Deflection | OK |
| Actual total load defl | 0.77 inches | | |

| | |
|----------------------|-------------|
| Bearing length req'd | 0.35 inches |
|----------------------|-------------|

BEAM DESIGN FOR UNIFORM LOAD: 2-2x6

(Values for DF Larch #2)

| | |
|-------------------------------------|------------|
| Width, b | 3 inches |
| Depth, d | 5.5 inches |
| Length of beam | 15 feet |
| Dead load roof | 11.5 psf |
| Live load roof | 16 psf |
| Contributory width of roof load | 2 feet |
| Dead load floor | 0 psf |
| Live load floor | 0 psf |
| Contributory width of floor load | 0 feet |
| Dead load wall | 0 plf |
| Live load defl ratio | 240 |
| Total load defl ratio | 180 |
| Total dead load | 23 plf |
| Total live load | 32 plf |

Base design values:

| | |
|--------------------------|-------------|
| Shear, Fv | 95 psi |
| Bending, Fb | 875 psi |
| Comp. perp. to grain, Fc | 625 psi |
| Mod of Elasticity, E | 1600000 psi |
| Load duration factor, Cd | 1.25 |
| Size Factor, Cf | 1.30 |
| Repetitive factor, Cr | 1.15 |

| | |
|---------------------|---------|
| Dead load reaction | 173 lbs |
| Live load reaction | 240 lbs |
| Total load reaction | 413 lbs |

| | | | |
|---------------------------|-------------|-----------------------|----|
| Allowable shear, Fv' | 119 psi | Horizontal Shear | OK |
| Actual shear, fv | 35 psi | | |
| Allowable bending, Fb' | 1635 psi | Bending | OK |
| Actual bending, fb | 1227 psi | | |
| Allowable live load defl | 0.75 inches | Live Load Deflection | OK |
| Actual live load defl | 0.55 inches | | |
| Allowable total load defl | 1.00 inches | Total Load Deflection | OK |
| Actual total load defl | 0.94 inches | | |
| Bearing length req'd | 0.22 inches | | |

BEAM DESIGN FOR UNIFORM LOAD:

(Values for DF Larch #1)

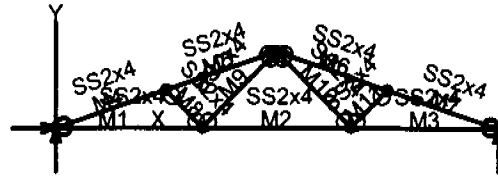
| | |
|-------------------------------------|-------------|
| Width, b | 3.5 inches |
| Depth, d | 9.25 inches |
| Length of beam | 12.5 feet |
| Dead load roof | 1.5 psf |
| Live load roof | 16 psf |
| Contributory width of roof load | 6 feet |
| Dead load floor | 0 psf |
| Live load floor | 0 psf |
| Contributory width of floor load | 0 feet |
| Dead load wall | 0 plf |
| Live load defl ratio | 360 |
| Toal load defl ratio | 240 |
| Total dead load | 9 plf |
| Total live load | 96 plf |

Base design values:

| | |
|--------------------------|-------------|
| Shear, Fv | 95 psi |
| Bending, Fb | 1000 psi |
| Comp. perp. to grain, Fc | 625 psi |
| Mod of Elasticity, E | 1600000 psi |
| Load duration factor, Cd | 1.25 |
| Size Factor, Cf | 1.20 |

| | |
|---------------------|---------|
| Dead load reaction | 56 lbs |
| Live load reaction | 600 lbs |
| Total load reaction | 656 lbs |

| | | | |
|---------------------------|-------------|-----------------------|----|
| Allowable shear, Fv' | 119 psi | Horizontal Shear | OK |
| Actual shear, fv | 27 psi | | |
| Allowable bending, Fb' | 1500 psi | Bending | OK |
| Actual bending, fb | 493 psi | | |
| Allowable live load defl | 0.42 inches | Live Load Deflection | OK |
| Actual live load defl | 0.14 inches | | |
| Allowable total load defl | 0.63 inches | Total Load Deflection | OK |
| Actual total load defl | 0.16 inches | | |
| Bearing length req'd | 0.30 inches | | |



VisualAnalysis 3.12.c Report

August 10, 1998 10:01 AM

Project:

File: D:\Paul\d_and_d\untitled folder\Zampella\truss.VAP

Engineer: Paul Zacher

Default Units: Feet, Pounds, Degrees, °Fahrenheit, Seconds.

Nodes

| Node | X ft | Y ft | Fix | DX Fix | DY Fix | RZ Fix |
|------|---------|---------|-----|-----------|-----------|-----------|
| N1 | 0.00 | 0.00 | Yes | Yes | No | |
| N2 | 8.00 | 0.00 | No | No | " | |
| N3 | 16.00 | 0.00 | " | " | " | |
| N4 | 24.00 | 0.00 | " | Yes | " | |
| N5 | 6.00 | 2.00 | " | No | " | |
| N6 | 18.00 | 2.00 | " | " | " | |
| N7 | 12.00 | 4.00 | " | " | " | |

Spring Elements

This item is empty. Check the selection state, or report properties.

Member Elements

| Member | Section | Material | Length ft | Weight lbs | Theta deg |
|--------|---------|----------|--------------|---------------|--------------|
| M1 | SS2x4 | Wood | 8.00 | 11.80 | 0.00 |
| M2 | " | " | 8.00 | 11.80 | 0.00 |
| M3 | " | " | 8.00 | 11.80 | 0.00 |
| M4 | " | " | 6.32 | 9.33 | 0.00 |
| M5 | " | " | 6.32 | 9.33 | 0.00 |
| M6 | " | " | 6.32 | 9.33 | 0.00 |
| M7 | " | " | 6.32 | 9.33 | 0.00 |
| M8 | " | " | 2.83 | 4.17 | 0.00 |
| M9 | " | " | 5.66 | 8.35 | 0.00 |
| M10 | " | " | 5.66 | 8.35 | 0.00 |
| M11 | " | " | 2.83 | 4.17 | 0.00 |

Section Properties

| Category | Section | Ax in ² | Iz in ⁴ | Sy+ in ³ | Sy- in ³ |
|----------|---------|-----------------------|-----------------------|------------------------|------------------------|
| Wood Sha | SS2x4 | 5.25 | 5.36 | 3.06 | 3.06 |

Material Properties

| Material | Strength ksi | Elasticity ksi | Poisson | Density lb/ft ³ | Therm. /F |
|----------|-----------------|-------------------|---------|-------------------------------|--------------|
| Wood | -NA- | 1700.00 | 0.36 | 40.47 | 0.00 |

Plate Elements

This item is empty. Check the selection state, or report properties.

VisualAnalysis 3.12.c Report

August 10, 1998 10:01 AM

Project:

File: D:\Paul\d and d\untitled folder\Zampella\truss.VAP

Engineer: Paul Zacher

Default Units: Feet, Pounds, Degrees, °Fahrenheit, Seconds.

Load Cases

| Load Case | Strength Service Results | | |
|---------------------|--------------------------|-----|---------|
| (1)Service Case 1 | Yes | Yes | 1st Ord |
| (2)Service Case 2 | " | " | " |
| (3)Equation Case 1 | " | " | " |

Service Load Cases

| Load Case | Load Source | Self Weight | Loads |
|----------------|-------------|-------------|-------|
| Service Case 1 | Dead loads | None | |
| Service Case 2 | Roof Live 1 | " | |

Load Combination Summary

Equation Case: Equation Case 1

Combination: +1D+1L+1Lr+1R+1W+1S+1E+1H+1F+1TS+1T+1TC+1I+1U+1LE

Contributing Cases & Source

Service Case 1 (Dead loads)

Service Case 2 (Roof Live loads)

Equation Case Combinations

| Load Case | Cases Equation | |
|-----------------|----------------|------|
| Equation Case 1 | 0.00 | 0.00 |

Factored Case Combinations

This item is empty. Check the selection state, or report properties.

Nodal Loads

This item is empty. Check the selection state, or report properties.

Member Point Loads

This item is empty. Check the selection state, or report properties.

Member Uniform Loads

| Load Case | Member | Direction | Offset ft | End Off ft | Magnitude |
|----------------|--------|-----------|--------------|---------------|------------|
| Service Case 1 | M1 | DY proj. | 0.00 | 8.00 | -0.01 K/ft |
| " | M2 | " | 0.00 | 8.00 | -0.01 K/ft |
| " | M3 | " | 0.00 | 8.00 | -0.01 K/ft |
| " | M4 | " | 0.00 | 6.32 | -0.02 K/ft |
| " | M5 | " | 0.00 | 6.32 | -0.02 K/ft |
| " | M6 | " | 0.00 | 6.32 | -0.02 K/ft |
| " | M7 | " | 0.00 | 6.32 | -0.02 K/ft |
| Service Case 2 | M4 | " | 0.00 | 6.32 | -0.03 K/ft |
| " | M5 | " | 0.00 | 6.32 | -0.03 K/ft |
| " | M6 | " | 0.00 | 6.32 | -0.03 K/ft |
| " | M7 | " | 0.00 | 6.32 | -0.03 K/ft |

Member Linear Loads

This item is empty. Check the selection state, or report properties.

Member Temperature Changes

This item is empty. Check the selection state, or report properties.

Member Gradient Temperatures

This item is empty. Check the selection state, or report properties.

VisualAnalysis 3.12.c Report

August 10, 1998 10:02 AM

Project:

File: D:\Paul\d_and_d\untitled folder\Zampella\truss.VAP

Engineer: Paul Zacher

Default Units: Feet, Pounds, Degrees, °Fahrenheit, Seconds.

Load Cases

| Load Case | Strength Service Results | | |
|---------------------|--------------------------|-----|---------|
| (1)Service Case 1 | Yes | Yes | 1st Ord |
| (2)Service Case 2 | " | " | " |
| (3)Equation Case 1 | " | " | " |

Member Extreme Results

| Member | Fx(lc) K | Fy(lc) K | Mz(lc) K-ft | fc max(lc) ksi | fc min(lc) ksi | Dx(lc) in | Dy(lc) in |
|--------|------------------|------------------|------------------|-------------------|-------------------|------------------|------------------|
| M1 | 0.87(1) | -0.03(1) | -0.04(1) | 0.17(1) | 0.01(1) | 0.00(3) | -0.17(3) |
| " | 1.80(3) | 0.03(3) | 0.04(3) | 0.51(3) | 0.34(3) | 0.02(3) | 0.00(1) |
| M2 | 0.55(1) | -0.03(1) | -0.04(1) | 0.11(1) | -0.04(1) | 0.01(1) | -0.20(3) |
| " | 1.12(3) | 0.03(3) | 0.03(3) | 0.33(3) | 0.21(3) | 0.03(3) | -0.08(1) |
| M3 | 0.87(1) | -0.03(3) | -0.04(1) | 0.17(1) | 0.01(1) | 0.02(1) | -0.17(3) |
| " | 1.80(3) | 0.03(1) | 0.04(3) | 0.51(3) | 0.34(3) | 0.05(3) | 0.00(1) |
| M4 | -1.93(3) | -0.19(3) | -0.22(3) | -0.37(3) | -1.21(3) | -0.02(3) | -0.18(3) |
| " | -0.89(1) | 0.12(3) | 0.15(3) | 0.52(3) | -0.18(1) | 0.00(1) | 0.00(1) |
| M5 | -1.64(3) | -0.12(3) | -0.22(3) | -0.30(3) | -1.18(3) | -0.03(3) | -0.26(3) |
| " | -0.76(1) | 0.19(3) | 0.15(3) | 0.55(3) | -0.15(1) | -0.01(1) | -0.08(1) |
| M6 | -1.64(3) | -0.19(3) | -0.22(3) | -0.30(3) | -1.18(3) | 0.03(1) | -0.24(3) |
| " | -0.76(1) | 0.12(3) | 0.15(3) | 0.55(3) | -0.15(1) | 0.08(3) | -0.07(1) |
| M7 | -1.93(3) | -0.12(3) | -0.22(3) | -0.37(3) | -1.21(3) | 0.02(1) | -0.16(3) |
| " | -0.89(1) | 0.19(3) | 0.15(3) | 0.52(3) | -0.18(1) | 0.06(3) | 0.02(3) |
| M8 | -0.43(3) | 0.00(2) | 0.00(1) | -0.08(3) | -0.08(3) | 0.07(1) | -0.11(3) |
| " | -0.18(1) | 0.00(3) | 0.00(3) | -0.03(1) | -0.03(1) | 0.13(3) | -0.04(1) |
| M9 | 0.25(2) | 0.00(1) | 0.00(1) | 0.05(2) | 0.05(2) | -0.11(3) | -0.14(3) |
| " | 0.52(3) | 0.00(3) | 0.00(3) | 0.10(3) | 0.10(3) | -0.05(1) | -0.07(1) |
| M10 | 0.25(2) | 0.00(3) | 0.00(3) | 0.05(2) | 0.05(2) | 0.07(1) | -0.10(3) |
| " | 0.52(3) | 0.00(1) | 0.00(1) | 0.10(3) | 0.10(3) | 0.14(3) | -0.05(1) |
| M11 | -0.43(3) | 0.00(3) | 0.00(3) | -0.08(3) | -0.08(3) | -0.10(3) | -0.14(3) |
| " | -0.18(1) | 0.00(2) | 0.00(1) | -0.03(1) | -0.03(1) | -0.05(1) | -0.06(1) |

BENDING & COMP: TRUSS 1; MEMBER 4Grading:

2x or 4x Doug-fir larch: No. 2

Assumptions:

Lateral support at points of bearing

SPS or gypboard attached to compression face

Maximum center-center spacing = 24"

| | |
|-----------------------|-----------------------|
| Width, b | 1.5 inches |
| Depth, d | 3.5 inches |
| Length | 6.32 feet |
| Max Axial Comp, C | 1930 lbs |
| Max Reaction, R | 190 lbs |
| Max Moment, M | 220 ft-lbs |
| Max LL Deflection | 0.09 inches |
| Max TL Deflection | 0.18 inches |
| LL Defl Criteria = L/ | 240 |
| TL Defl Criteria = L/ | 180 |
| Duration factor, Cd | 1.25 |
| Repetitive Factor, Cr | 1.15 |
| fc = | 368 psi |
| Fce= | 1597 psi |
| Fc*= | 1094 psi |
| F'c= | 879 psi |
| fb= | 72 psi |
| F'b= | 1258 psi |
| Shear D/C ratio | 0.46 < 1.0, Member OK |
| Interaction equation: | |
| (fc/F'c)^2 + | |
| fb/ (F'b(1-fc/Fce)) = | 0.25 < 1.0, Member OK |
| Live Load defl ratio | 0.28 < 1.0, Member OK |
| Total Load defl ratio | 0.43 < 1.0, Member OK |

6698 GLORIA
DRIVE 2 JIB CT



SCAB 2x4 TO EXISTING
2x4 RAFTERS
(TOTAL 10)

5°
OVER
HANG

NO ACCESS
FLAT 2

2x4 c24

TRUSS ①

9°

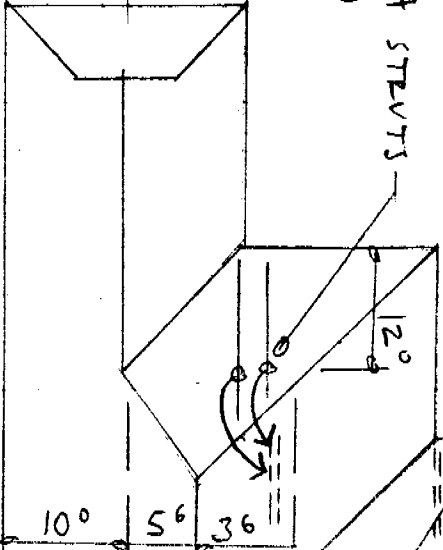
8°

12'6"
4x10

ADD 2x6x8' PURLIN
W/ 2x4 STUB

ADD 2x4 STUBS
(TOTAL 2)

6°



1 ROOF PLAN-ZAMPPELLA
N.T.S.

2x6 c24

5'
12°

SCAB 2x6 TO
EXISTING 2x6
RAFTERS
(TOTAL 6)

ADD STUB
(TOT 1)

SCAB 2x6 TO EXISTING
2x6 RAFTERS (TOT 2)

