

CITY OF SACRAMENTO

Permit No: 9807318

1231 I Street, Sacramento, CA 95814

Insp Area: 1

Site Address: 8309 BRIAR CLIFF WY SAC

Sub-Type: ASFR

Parcel No: 0790194016

Housing (Y/N): N

CONTRACTOR

D H CONSTRUCTION
4324 ORANGE GROVE AV
SACRAMENTO CA 95841

OWNER

DERN DOUGLAS J/LINDA A
2516 BELHAVEN WY
SACRAMENTO CA 95826

ARCHITECT

Nature of Work: FIRE REPAIR AND CHANGE ROOF TO VAULTED TRUSSES

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name None Lender's Address None

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class B1 License Number 362161 Date 9-30-99 Contractor Signature Ralph Amiruddin

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00):

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. _____ B & PC for this reason: _____
Date _____ Owner Signature _____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 8-5-98 Applicant/Agent Signature Ralph Amiruddin

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier State Fund Policy Number 046-98 Unit 0004903

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 8-5-98 Applicant Signature Ralph Amiruddin

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

*11/1/99
BT*

Sacramento Fire Department - Incident Report

Incident No : 980024684 Call# : 98053633 Date: 06/16/98 Time: 15:58
Address : 8309 BRIAR CLIFF WY
Type : 11 BUILDING FIRE
Action Taken: 13 EXTINGUISH,SALVAGE,OVERHAUL
Property : 1-2 FAMILY RESIDENTIAL: SINGLE FAMILY
UBC : DWELLINGS AND LODGING HOUSES

Weather : 80 Degrees / Clear
Resources : 1 Engine, 1 Truck
 : 1 Other Apparatus
 : 1 Fire Rescue Unit

Fire Casualties : None

Fire Damage : Extended beyond structure of origin
Smoke Damage : Confined to structure of origin
Property Loss : \$130,000 Contents Loss :
Property Value : \$150,000 Contents Value: \$30,000
Area of Origin : Concealed roof/ceiling space
Caused by : Equipment:Insufficient information
Form of Heat : Undetermined
Ignition Factor : Undetermined
Type of Material : Sawn woods,finished lumber
Form of Material : Structural member,framing
Type of Material : Sawn woods,finished lumber
Form of Material : Structural member,framing
Smoke Travel : Not classified
Other Factors : Roof covering
Extinguished by : Water from hydrant,draft,standpipe
Structure Type : Building with one specific property use
Structure Status : In use
 Occupied
Construction Type: Type V - Wood Frame
Roof Type : Wood shake - untreated
Number of Stories: 1

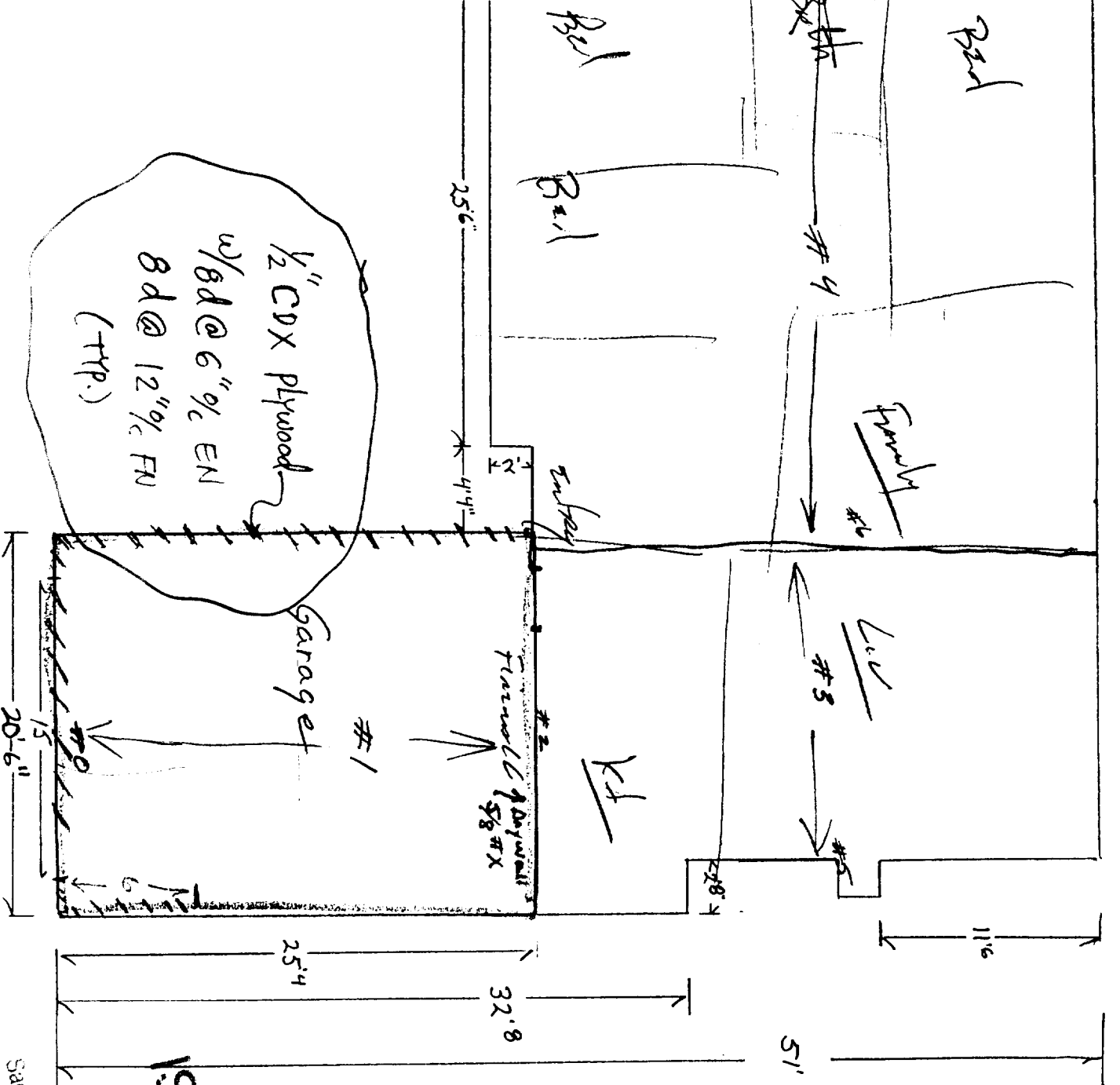
Level: A01

Detector Type : Undetermined/not reported

Extinguishing Sys: No extinguishing system

Report Author : F304

47'6"



1/2" CDX Plywood
 w/ 8d @ 6" o.c. EN
 8d @ 12" o.c. FN
 (TRP.)

Garage #1

Firewall #2
 5/8" Drywall

KL

Living #3

20'-6"

25'-4"

32'-8"

5'1"

11'-6"

Dem Job

Scope of Work

Garage Fire

Wall Frame 2x4 @ 16" o.c.
 Truss Roof 2' Top Plates

Elec Amplifier

Windows Replace same

Head's Air

PAINT 2nd 3rd.

Paints

Drywall

Floor Ceilings

Fire wall 5/8" Drywall

EXT Stucco

FRONT

Roofing Comp

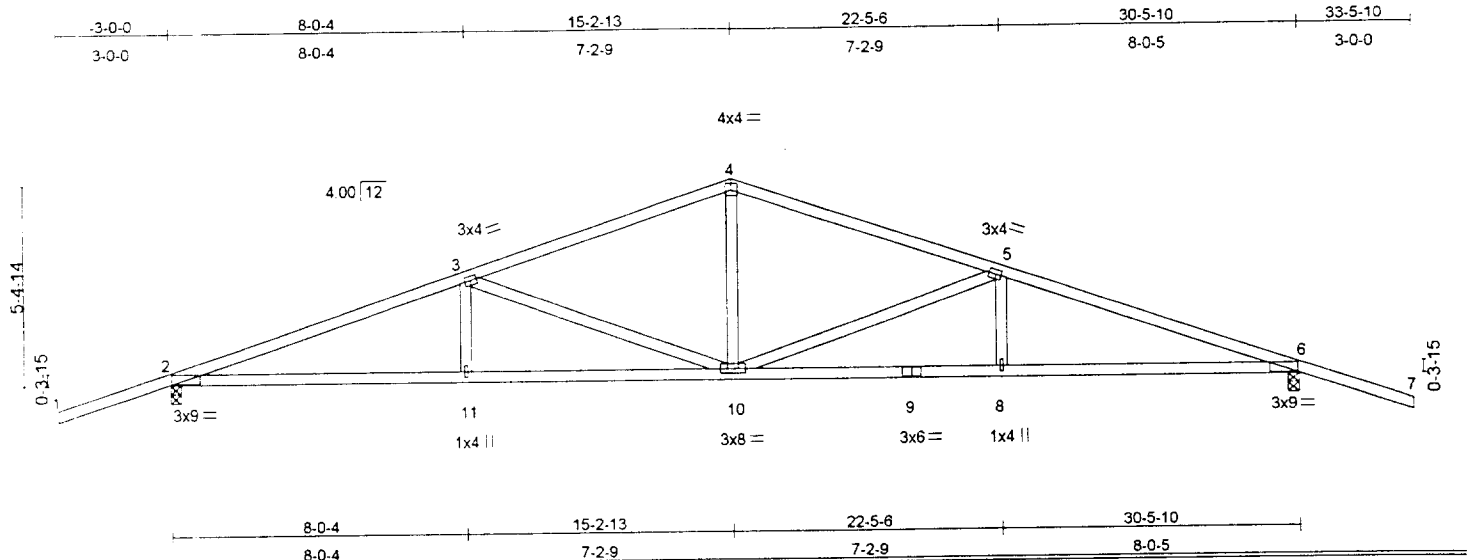
ISSUED

AUG 05 1998

4 63

Sacramento Building Division

1/8" scale



LOADING (psf)	SPACING	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 16.0	2-0-0	TC 0.69	Vert(LL) -0.14 10-11 >999	M20 186/148
TCDL 14.0	Plates Increase 1.00	BC 0.64	Vert(TL) -0.33 10-11 >999	
BCLL 0.0	Lumber Increase 1.25	WB 0.83	Horz(TL) 0.09 6 n/a	Weight: 127 lb
BDDL 7.0	Code UBC/CBO		Min Length / LL defl = 360	

LUMBER
TOP CHORD 2 X 4 DF No. 1&Btr-G
BOT CHORD 2 X 4 DF No. 1&Btr-G
WEBS 2 X 4 DF Std-G

BRACING
TOP CHORD Sheathed or 3-2-2 on center purlin spacing.
BOT CHORD Rigid ceiling directly applied or 9-3-7 on center bracing.

REACTIONS (lb/size) 2=1305/0-3-8, 6=1305/0-3-8
Max Horz 2=25(load case 3)
Max Uplift 2=429(load case 3), 6=429(load case 4)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=28, 2-3=-2542, 3-4=-1739, 4-5=-1739, 5-6=-2542, 6-7=28
BOT CHORD 2-11=2404, 10-11=2404, 9-10=2404, 8-9=2404, 6-8=2404
WEBS 3-11=106, 4-10=667, 5-8=106, 3-10=806, 5-10=806

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC/CBO. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
 - 3) All plates are M20 plates unless otherwise indicated.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 5) A plate rating reduction of 20% has been applied for the green lumber members.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 429 lb uplift at joint 2 and 429 lb uplift at joint 6.
 - 7) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

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Sacramento Building Division

REGISTERED PROFESSIONAL ENGINEER
EXPIRES 12/31/98
STATE OF CALIFORNIA
CIVIL

AUG 04 1998



NOTE: This design is valid for use with MITek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling, Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	D H CONST-DERN-
DH02	T-1A	SCISSORS	13	1	

General Truss Co. Sacramento, CA 95828

4-0-32 s Dec 16 1997 MiTek Industries, Inc. Tue Aug 04 11:05:26 1998 Page 1

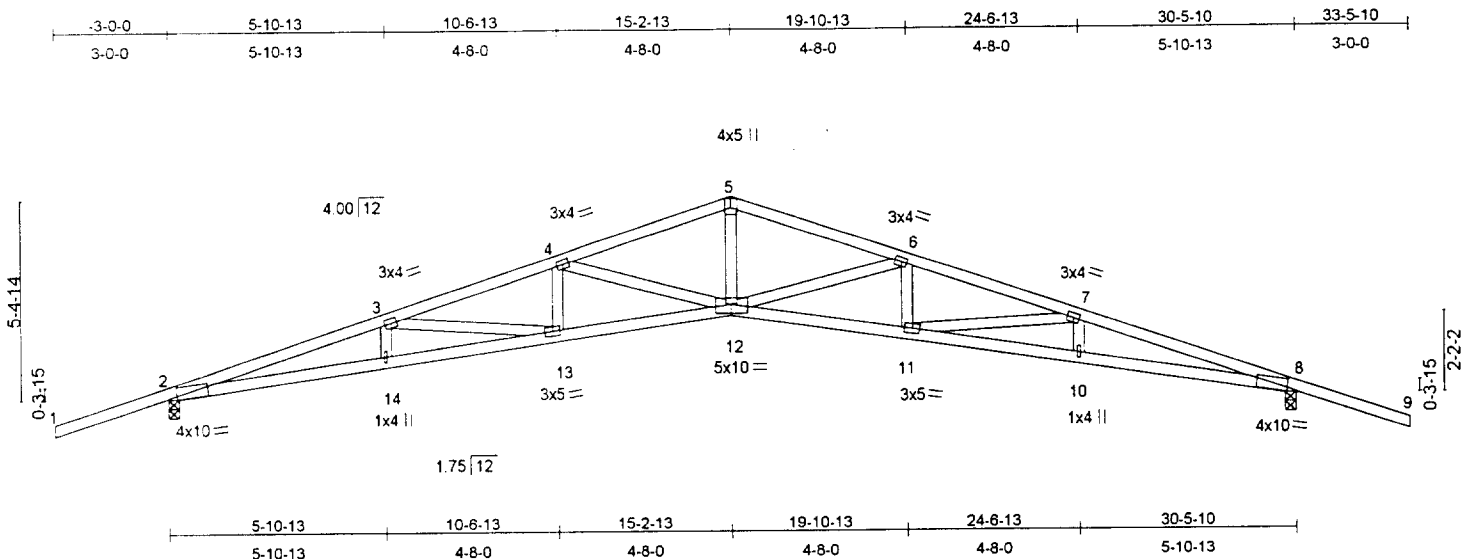


Plate Offsets (X, Y): [2:0-4-12,0-2-0], [5:0-2-0,0-2-12], [8:0-4-12,0-2-0], [12:0-5-0,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 16.0	Plates Increase 1.00	TC 0.90	Vert(LL) 0.40 12 >906	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.67	Vert(TL) -0.85 12 >426	
BCLL 0.0	Rep Stress Incr YES	WB 0.67	Horz(TL) 0.41 8 n/a	
BCDL 7.0	Code UBC/ICBO		Min Length / LL defl = 360	Weight: 129 lb

LUMBER
TOP CHORD 2 X 4 DF No. 1&Btr-G
BOT CHORD 2 X 4 DF No. 1&Btr-G
WEBS 2 X 4 DF Std-G

BRACING
TOP CHORD Sheathed.
BOT CHORD Rigid ceiling directly applied or 6-9-3 on center bracing.

REACTIONS (lb/size) 2=1306/0-3-8, 8=1306/0-3-8
Max Horz 2=25(load case 3)
Max Uplift 2=-429(load case 3), 8=-429(load case 4)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=27, 2-3=-4701, 3-4=-3924, 4-5=-3041, 5-6=-3041, 6-7=-3924, 7-8=-4701, 8-9=27
BOT CHORD 2-14=4484, 13-14=4484, 12-13=3762, 11-12=3762, 10-11=4484, 8-10=4484
WEBS 5-12=1643, 3-14=73, 4-13=245, 6-11=245, 7-10=73, 3-13=-718, 4-12=-873, 6-12=-873, 7-11=-718

#3

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC/ICBO. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
 - 3) All plates are M20 plates unless otherwise indicated.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 5) A plate rating reduction of 20% has been applied for the green lumber members.
 - 6) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 429 lb uplift at joint 2 and 429 lb uplift at joint 8.
 - 8) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

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NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	D H CONST--DERN--
DH02	T-1-GE	HOWE	1	1	

General Truss Co., Sacramento, CA 95828

4 0-32 s Dec 16 1997 MiTek Industries, Inc. Tue Aug 04 11:05:32 1998 Page 1

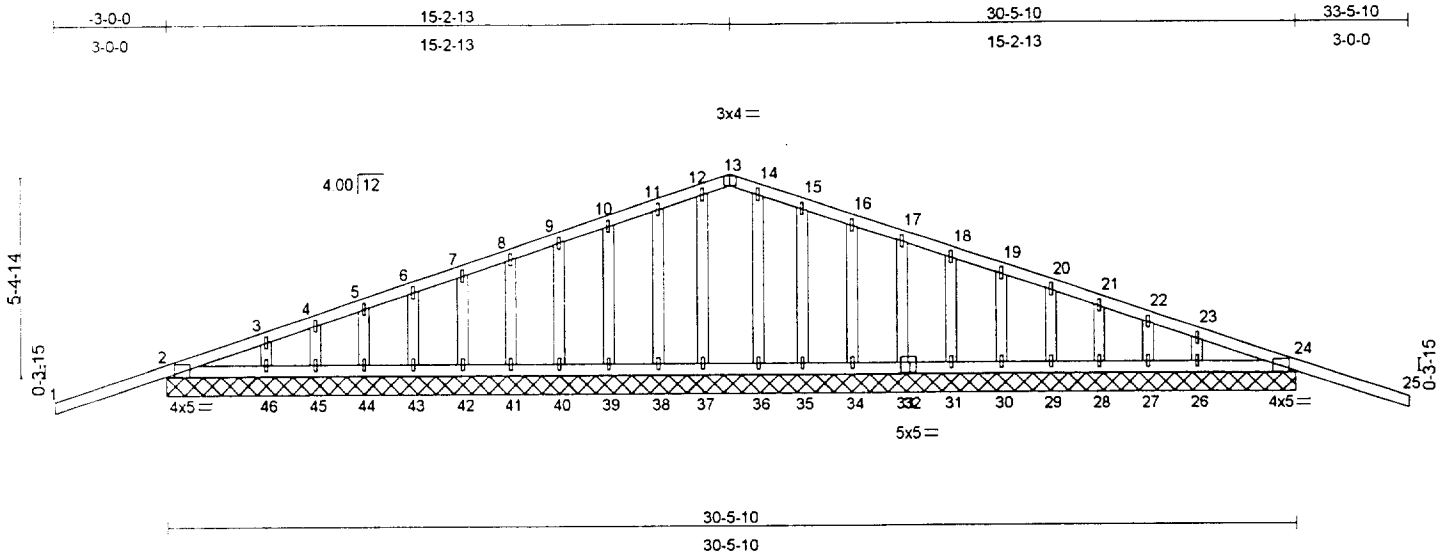


Plate Offsets (X, Y): [32, 0-2, 3, 0-0-4]				
LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 16.0	Plates Increase 1.00	TC 0.48	Vert(LL) n/a - n/a	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.22	Vert(TL) 0.18 24-25 >209	
BCLL 0.0	Rep Stress Incr YES	WB 0.04	Horz(TL) 0.00 n/a	
BCDL 7.0	Code UBC/ICBO	(Matrix)	Min Length / LL defl = 999	Weight: 166 lb

LUMBER
 TOP CHORD 2 X 4 DF No 1&Btr-G
 BOT CHORD 2 X 4 DF No 1&Btr-G
 OTHERS 2 X 4 DF Std-G

BRACING
 TOP CHORD Sheathed or 6-0-0 on center purlin spacing
 BOT CHORD Rigid ceiling directly applied or 0-2-6 on center bracing.

REACTIONS (lb/size) 2=426/30-5-10, 32=5/30-5-10, 24=426/30-5-10, 37=96/30-5-10, 36=96/30-5-10, 26=-62/30-5-10, 27=178/30-5-10, 28=79/30-5-10, 29=103/30-5-10, 30=98/30-5-10, 31=98/30-5-10, 33=94/30-5-10, 34=100/30-5-10, 35=90/30-5-10, 46=-62/30-5-10, 45=178/30-5-10, 44=79/30-5-10, 43=103/30-5-10
 Max Horz 2=171(load case 3), 24=-171(load case 4)
 Max Uplift 2=-300(load case 3), 24=-300(load case 4), 26=-62(load case 1), 27=-132(load case 4), 28=-6(load case 4), 29=-36(load case 4), 30=-29(load case 4), 31=-32(load case 4), 33=-34(load case 4), 34=-31(load case 4), 35=-33(load case 4), 46=-62(load case 1), 45=-132(load case 3), 44=-6(load case 3), 43=-36(load case 3)
 Max Grav 32=12(load case 2), 24=426(load case 1), 37=96(load case 1), 36=96(load case 1), 26=180(load case 4), 27=178(load case 7), 28=79(load case 1), 29=103(load case 1), 30=98(load case 1), 31=98(load case 7), 33=94(load case 1), 34=100(load case 7), 35=92(load case 7), 46=180(load case 3), 45=178(load case 6), 44=79(load case 1), 43=103(load case 1)

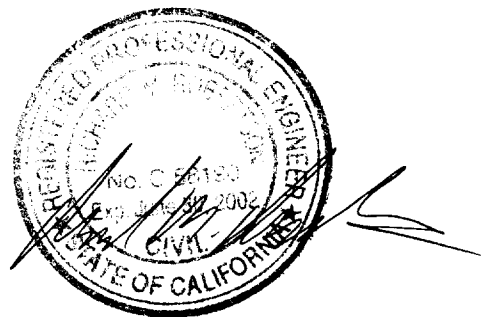
FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=56, 2-3=-59, 3-4=11, 4-5=-23, 5-6=-21, 6-7=-22, 7-8=-22, 8-9=-22, 9-10=-22, 10-11=-22, 11-12=2, 12-13=-22, 13-14=-22, 14-15=-20, 15-16=-22, 16-17=-22, 17-18=-22, 18-19=-22, 19-20=-22, 20-21=-21, 21-22=-23, 22-23=-11, 23-24=-59, 24-25=56
 BOT CHORD 2-46=0, 45-46=0, 44-45=0, 43-44=0, 42-43=0, 41-42=0, 40-41=0, 39-40=0, 38-39=0, 37-38=0, 36-37=0, 35-36=0, 34-35=0, 33-34=0, 32-33=0, 31-32=0, 30-31=0, 29-30=0, 28-29=0, 27-28=0, 26-27=0, 24-26=0
 WEBS 12-37=-77, 14-36=-77, 23-26=-21, 22-27=-110, 21-28=-74, 20-29=-81, 19-30=-80, 18-31=-80, 17-33=-80, 16-34=-81, 15-35=-74, 3-46=-21, 4-45=-110, 5-44=-74, 6-43=-81, 7-42=-80, 8-41=-80, 9-40=-80, 10-39=-81, 11-38=-74

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) All plates are M20 plates unless otherwise indicated.
 - 3) All plates are 1x4 M20 unless otherwise indicated.
 - 4) Gable requires continuous bottom chord bearing.
 - 5) Gable studs spaced at 1-4-0 on center.
 - 6) For studs exposed to wind, see MiTek "Standard Gable End Detail"
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 8) A plate rating reduction of 20% has been applied for the green lumber members.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 300 lb uplift at joint 2, 300 lb uplift at joint 24, 62 lb uplift at joint 26, 132 lb uplift at joint 27, 6 lb uplift at joint 28, 36 lb uplift at joint 29, 29 lb uplift at joint 30, 32 lb uplift at joint 31, 34 lb uplift at joint 33, 31 lb uplift at joint 34, 33 lb uplift at joint 35, 62 lb uplift at joint 46, 132 lb uplift at joint 45, 6 lb uplift at joint 44, 36 lb uplift at joint 43, 29 lb uplift at joint 42, 31 lb uplift at joint 41, 31 lb uplift at joint 40, 31 lb uplift at joint 39 and 32 lb uplift at joint 38.
 - 10) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

#5

AUG 04 1998



NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	D H CONST-DERN--
DH02	T-1SGE	HOWE	2	1	

General Truss Co., Sacramento, CA 95828

4 0-32 s Dec 16 1997 MiTek Industries, Inc. Tue Aug 04 11:05:40 1998 Page 1

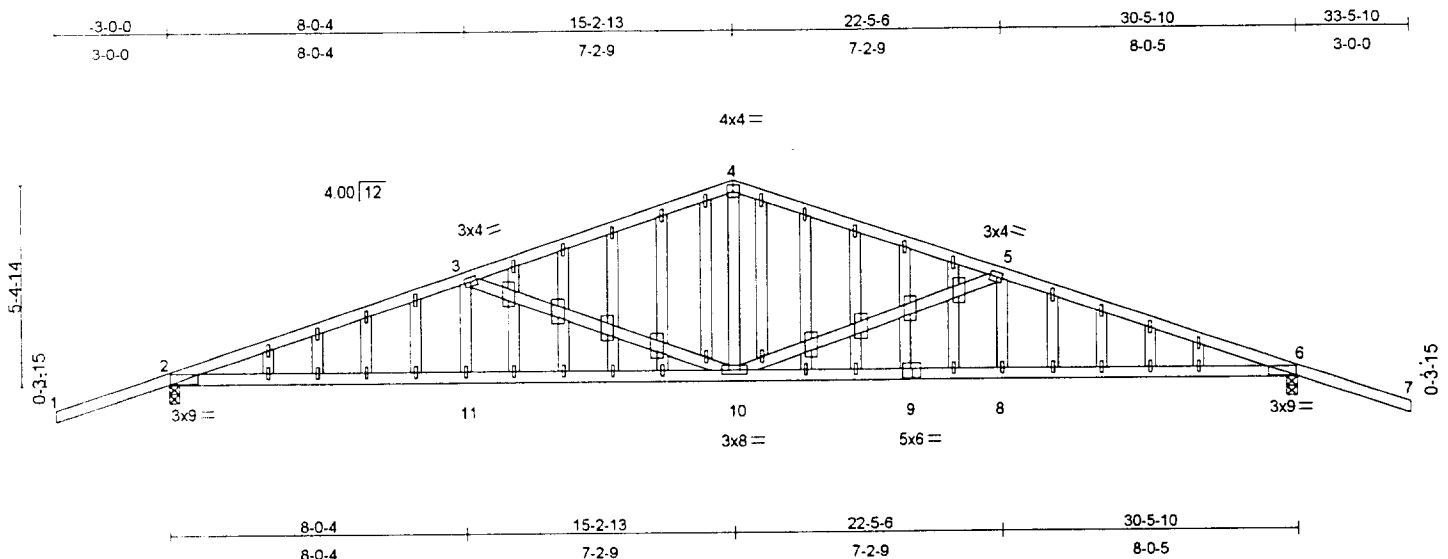


Plate Offsets (X, Y)	[9 0-2-11, 0-0-4], [29 0-1-12, 0-0-0]			
LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) / defl	PLATES GRIP
TCLL 16.0	Plates Increase 1.00	TC 0.69	Vert(LL) -0.14 10-11 >999	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.64	Vert(TL) -0.33 10-11 >999	
BCLL 0.0	Rep Stress Incr YES	WB 0.83	Horz(TL) 0.09 6 n/a	Weight: 189 lb
BCDL 7.0	Code UBC/ICBO		Min Length / LL defl = 360	

LUMBER
 TOP CHORD 2 X 4 DF No. 1&Btr-G
 BOT CHORD 2 X 4 DF No. 1&Btr-G
 WEBS 2 X 4 DF Std-G
 OTHERS 2 X 4 DF Std-G

BRACING
 TOP CHORD Sheathed or 3-2-2 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 9-3-7 on center bracing.

REACTIONS (lb/size) 2=1305/0-3-8, 6=1305/0-3-8
 Max Horz 2=25(load case 3)
 Max Uplift 2=-429(load case 3), 6=-429(load case 4)

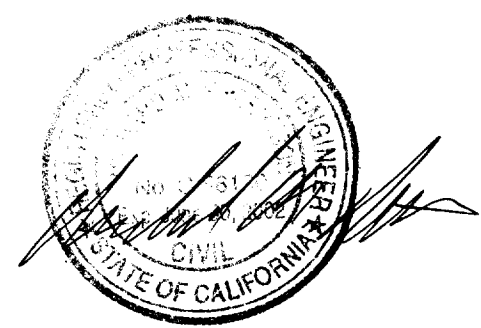
FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=28, 2-3=-2542, 3-4=-1739, 4-5=-1739, 5-6=-2542, 6-7=28
 BOT CHORD 2-11=2404, 10-11=2404, 9-10=2404, 8-9=2404, 6-8=2404
 WEBS 3-11=106, 4-10=667, 5-8=106, 3-10=-806, 5-10=-806

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC/ICBO if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 3) All plates are M20 plates unless otherwise indicated.
 - 4) All plates are 1x4 M20 unless otherwise indicated.
 - 5) Gable studs spaced at 1-4-0 on center.
 - 6) For studs exposed to wind, see MiTek "Standard Gable End Detail"
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 8) A plate rating reduction of 20% has been applied for the green lumber members.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 429 lb uplift at joint 2 and 429 lb uplift at joint 6.
 - 10) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

#6

AUG 04 1998



NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	D H CONST-DERN-
DH02	T-2	FINK	9	1	

General Truss Co. Sacramento, CA 95828

4 0-32 s Dec 16 1997 MITek Industries, Inc. Tue Aug 04 11:05:43 1998 Page 1

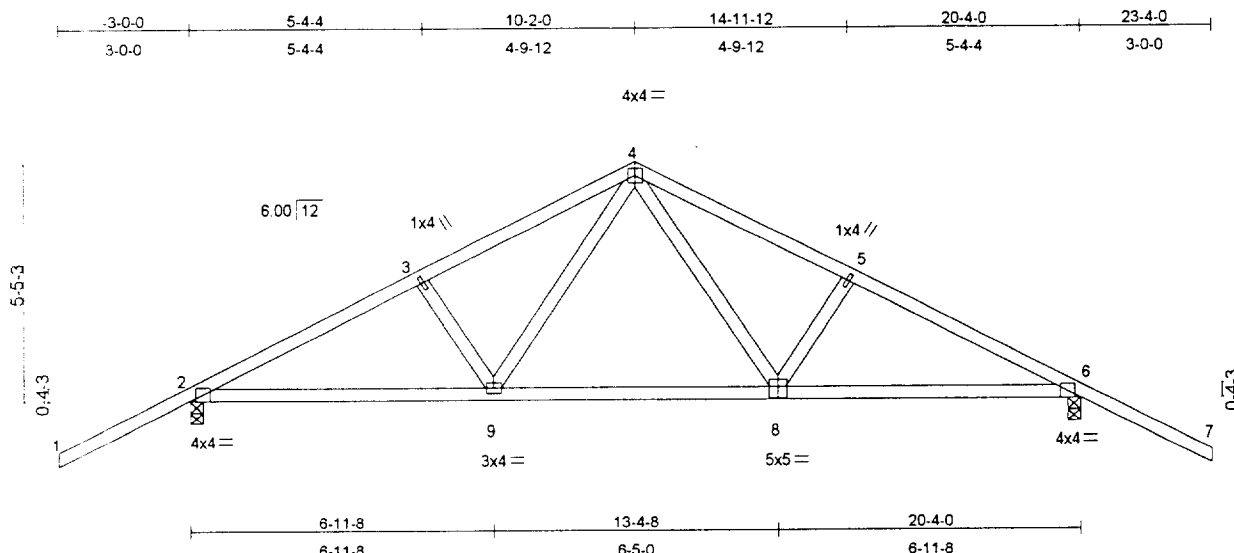


Plate Offsets (X,Y) [2-0-1-8, edge], [6-0-1-8, edge], [8-0-2-8, 0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 16.0	Plates Increase 1.00	TC 0.47	Vert(LL) -0.06 6-8 >999	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.40	Vert(TL) 0.13 6-7 >285	
BCLL 0.0	Rep Stress Incr YES	WB 0.15	Horz(TL) 0.02 6 n/a	Weight: 91 lb
BCDL 7.0	Code UBC/ICBO		Min Length / LL defl = 360	

LUMBER
 TOP CHORD 2 X 4 DF No 1&Btr-G
 BOT CHORD 2 X 4 DF No 1&Btr-G
 WEBS 2 X 4 DF Std-G

BRACING
 TOP CHORD Sheathed or 5-9-6 on center purlin spacing
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing

REACTIONS (lb/size) 2=930/0-3-8, 6=930/0-3-8
 Max Horz 2=-68(load case 3)
 Max Uplift 2=-298(load case 5), 6=-298(load case 5)

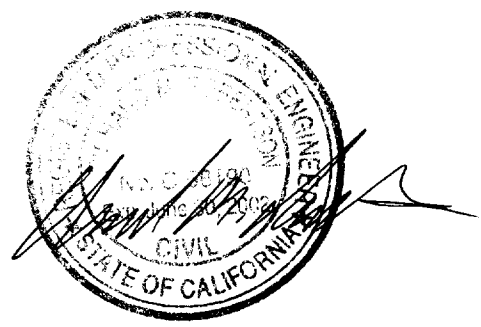
FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=40, 2-3=-1168, 3-4=-1008, 4-5=-1008, 5-6=-1168, 6-7=40
 BOT CHORD 2-9=1035, 8-9=711, 6-8=1035
 WEBS 3-9=-250, 4-9=359, 4-8=359, 5-8=-250

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC/ICBO if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 3) All plates are M20 plates unless otherwise indicated.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 5) A plate rating reduction of 20% has been applied for the green lumber members.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 298 lb uplift at joint 2 and 298 lb uplift at joint 6.
 - 7) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

1

AUG 04 1998



NOTE: This design is valid for use with MITek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling, Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	D H CONST-DERN-
OH02	T-2DH	FINK	1	1	

General Truss Co., Sacramento, CA 95828

4.0-32 s Dec 16 1997 MiTek Industries, Inc. Tue Aug 04 11:05:48 1998 Page 1

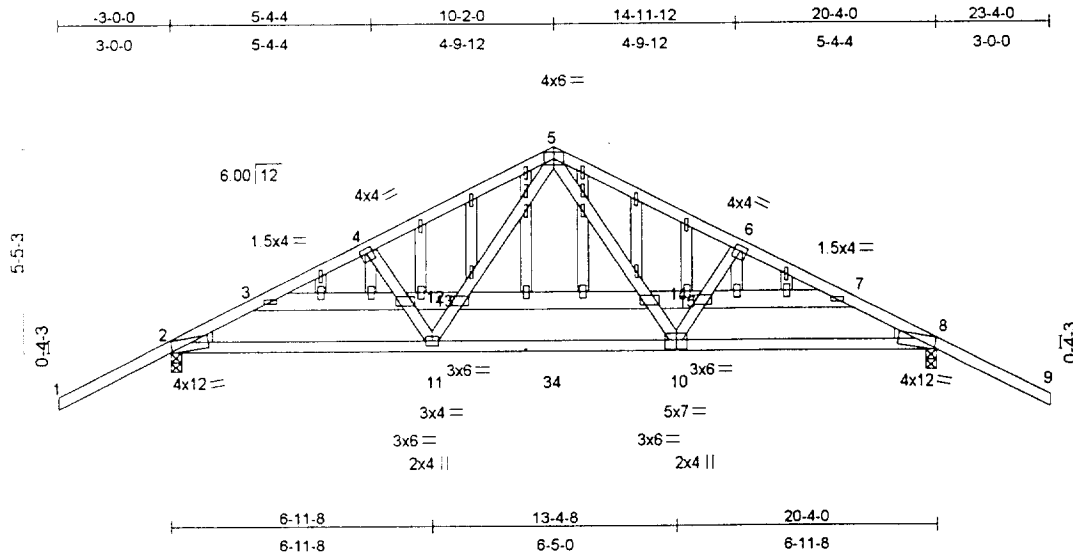


Plate Offsets (X, Y) [2 0-0-10 edge], [4 0-2-0, 0-1-8], [6 0-2-0, 0-1-8], [8 0-0-10 edge], [10 0-3-8, 0-3-0]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 16.0	Plates Increase 1.00		TC 0.63	Vert(LL) 0.12 10-11 >999	M20 186/148
TCDL 14.0	Lumber Increase 1.25		BC 0.69	Vert(TL) 0.23 8-9 >164	
BCLL 0.0	Rep Stress Incr NO		WB 0.31	Horz(TL) 0.07 8 n/a	
BCDL 7.0	Code UBC/ICBO		(Matrix)	Min Length / LL defl = 360	Weight: 150 lb

LUMBER
 TOP CHORD 2 X 4 DF No. 1&Btr-G
 BOT CHORD 2 X 4 DF No. 1&Btr-G
 WEBS 2 X 6 DF No 2-G "Except"
 4-11 2 X 4 DF Std-G, 5-11 2 X 4 DF Std-G, 5-10 2 X 4 DF Std-G, 6-10 2 X 4 DF Std-G
 OTHERS 2 X 4 DF Std-G
 WEDGE Left 2 X 4 DF Std, Right 2 X 4 DF Std

BRACING
 TOP CHORD Sheathed or 3-3-12 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 8-0-0 on center bracing.

REACTIONS (lb/size) 2=2055/0-3-8, 8=2055/0-3-8
 Max Horz 2=-68(load case 3)
 Max Uplift 2=-647(load case 5), 8=-647(load case 5)

2

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=80, 2-3=-3342, 3-4=-3437, 4-5=-3330, 5-6=-3330, 6-7=-3437, 7-8=-3342, 8-9=80
 BOT CHORD 2-11=2869, 11-34=2508, 10-34=2508, 8-10=2869
 WEBS 4-12=-227, 11-12=-197, 11-13=517, 5-13=526, 5-14=526, 10-14=517, 10-15=-197, 6-15=-227, 3-12=190, 12-13=147, 13-14=139, 14-15=147, 7-15=190

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC/ICBO if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 3) Except as shown below, special connection(s) required to support concentrated load(s). Design of connection(s) is delegated to the building designer.
 - 4) All plates are M20 plates unless otherwise indicated.
 - 5) Gable studs spaced at 1-4-0 on center.
 - 6) For studs exposed to wind, see MiTek "Standard Gable End Detail"
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94
 - 8) A plate rating reduction of 20% has been applied for the green lumber members.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 647 lb uplift at joint 2 and 647 lb uplift at joint 8.
 - 10) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

- LOAD CASE(S)**
- 1) Regular Lumber Increase=1.25, Plate Increase=1.00
 Uniform Loads (plf)
 Vert: 2-11=-41.1, 11-34=-41.1, 10-34=-41.1, 8-10=-41.1, 1-2=-60.0, 2-3=-60.0, 3-4=-60.0, 4-5=-60.0, 5-6=-60.0, 6-7=-60.0, 7-8=-60.0, 8-9=-60.0
 Concentrated Loads (lb)
 Vert: 5=-1708
 - 2) UBC Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 2-11=-61.1, 11-34=-61.1, 10-34=-61.1, 8-10=-61.1, 1-2=-28.0, 2-3=-28.0, 3-4=-28.0, 4-5=-28.0, 5-6=-28.0, 6-7=-28.0, 7-8=-28.0, 8-9=-28.0
 Concentrated Loads (lb)
 Vert: 5=-797
 - 3) Wind Left Lumber Increase=1.33, Plate Increase=1.33
 Uniform Loads (plf)
 Vert: 2-11=-29.3, 11-34=-29.3, 10-34=-29.3, 8-10=-29.3, 1-2=37.6, 2-3=14.3, 3-4=14.3, 4-5=14.3, 5-6=24.6, 6-7=24.6, 7-8=24.6, 8-9=48.0
 Horz: 1-2=-47.6, 2-3=-24.3, 3-4=-24.3, 4-5=-24.3, 5-6=34.6, 6-7=34.6, 7-8=34.6, 8-9=58.0
 Concentrated Loads (lb)
 Vert: 5=1085
 - 4) Wind Right Lumber increase=1.33, Plate Increase=1.33

Continued on page 2

AUG 04 1998



NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	D H CONST-DERN-
DH02	T-2DH	FINK	1	1	

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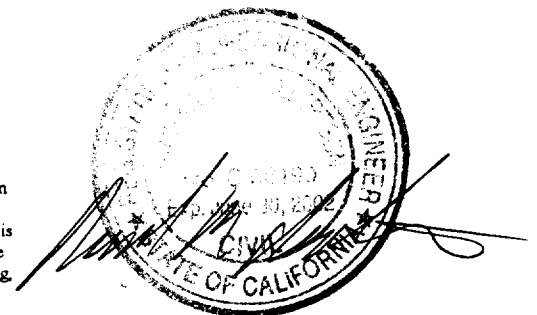
4 0-32 s Dec 16 1997 MITek Industries, Inc. Tue Aug 04 11:05:48 1998 Page 2

LOAD CASE(S)

- Uniform Loads (plf)
 - Vert: 2-11=-29.3, 11-34=-29.3, 10-34=-29.3, 8-10=-29.3, 1-2=48.0, 2-3=24.6, 3-4=24.6, 4-5=24.6, 5-6=14.3, 6-7=14.3, 7-8=14.3, 8-9=37.6
 - Horz: 1-2=-58.0, 2-3=-34.6, 3-4=-34.6, 4-5=-34.6, 5-6=24.3, 6-7=24.3, 7-8=24.3, 8-9=47.6
- Concentrated Loads (lb)
 - Vert: 5=1085
- 5) 1st Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
 - Uniform Loads (plf)
 - Vert: 2-11=-29.3, 11-34=-29.3, 10-34=-29.3, 8-10=-29.3, 1-2=48.0, 2-3=24.6, 3-4=24.6, 4-5=24.6, 5-6=24.6, 6-7=24.6, 7-8=24.6, 8-9=48.0
 - Horz: 1-2=-58.0, 2-3=-34.6, 3-4=-34.6, 4-5=-34.6, 5-6=34.6, 6-7=34.6, 7-8=34.6, 8-9=58.0
 - Concentrated Loads (lb)
 - Vert: 5=1085
- 6) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.00
 - Uniform Loads (plf)
 - Vert: 2-11=-41.1, 11-34=-41.1, 10-34=-41.1, 8-10=-41.1, 1-2=-60.0, 2-3=-60.0, 3-4=-60.0, 4-5=-60.0, 5-6=-28.0, 6-7=-28.0, 7-8=-28.0, 8-9=-28.0
 - Concentrated Loads (lb)
 - Vert: 5=-1708
- 7) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.00
 - Uniform Loads (plf)
 - Vert: 2-11=-41.1, 11-34=-41.1, 10-34=-41.1, 8-10=-41.1, 1-2=-28.0, 2-3=-28.0, 3-4=-28.0, 4-5=-28.0, 5-6=-60.0, 6-7=-60.0, 7-8=-60.0, 8-9=-60.0
 - Concentrated Loads (lb)
 - Vert: 5=-1708

AUG 04 1998

NOTE: This design is valid for use with MITek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



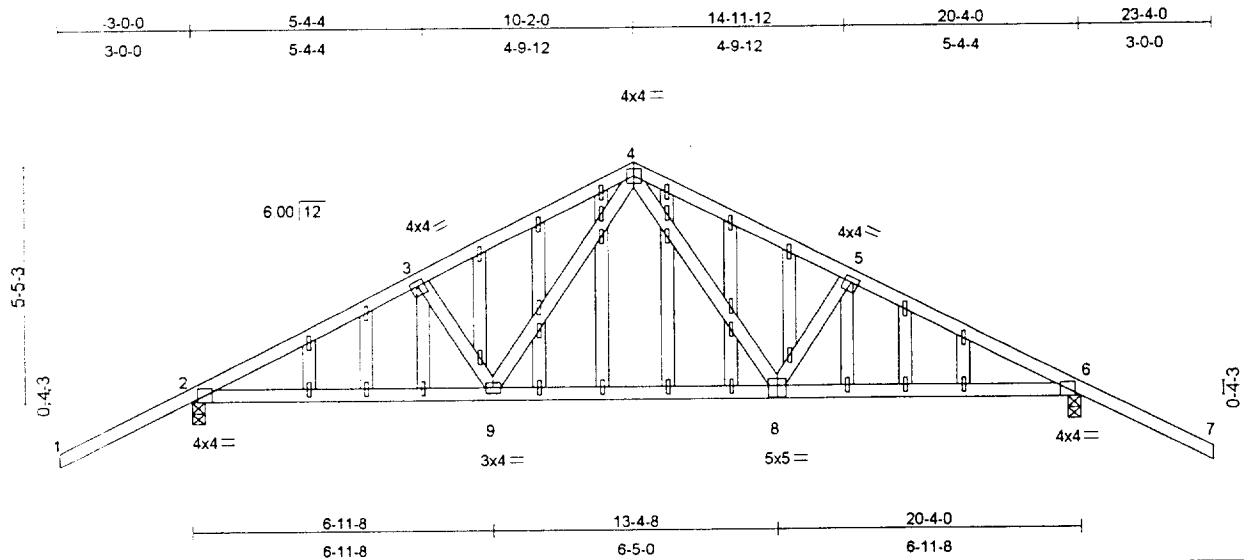


Plate Offsets (X, Y) [2 0-1-8, edge], [3 0-2-0, 0-1-8], [5 0-2-0, 0-1-8], [6 0-1-8, edge], [8 0-2-8, 0-3-0]				
LOADING (psf)	SPACING	CSI	DEFL (in) (loc)	PLATES GRIP
TCLL 16 0	Plates Increase 1.00	TC 0.47	Vert(LL) -0.06 6-8 >999	M20 186/148
TCDL 14 0	Lumber Increase 1.25	BC 0.40	Vert(TL) 0.13 6-7 >285	
BCLL 0 0	Rep Stress Incr YES	WB 0.15	Horz(TL) 0.02 6 n/a	Weight: 135 lb
BCDL 7 0	Code UBC/ICBO		Min Length / LL defl = 360	

LUMBER
 TOP CHORD 2 X 4 DF No. 1&Btr-G
 BOT CHORD 2 X 4 DF No. 1&Btr-G
 WEBS 2 X 4 DF Std-G
 OTHERS 2 X 4 DF Std-G

BRACING
 TOP CHORD Sheathed or 5-9-6 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=930/0-3-8, 6=930/0-3-8
 Max Horz 2=-68(load case 3)
 Max Uplift 2=-298(load case 5), 6=-298(load case 5)

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=40, 2-3=-1168, 3-4=-1008, 4-5=-1008, 5-6=-1168, 6-7=40
 BOT CHORD 2-9=1035, 8-9=711, 6-8=1035
 WEBS 3-9=-250, 4-9=359, 4-8=359, 5-8=-250

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC/ICBO. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
 - 3) All plates are M20 plates unless otherwise indicated.
 - 4) All plates are 1x4 M20 unless otherwise indicated.
 - 5) Gable studs spaced at 1-4-0 on center.
 - 6) For studs exposed to wind, see MiTek "Standard Gable End Detail".
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 8) A plate rating reduction of 20% has been applied for the green lumber members.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 298 lb uplift at joint 2 and 298 lb uplift at joint 6.
 - 10) This truss has been designed for both UBC-94 and ANSUTPI 1-1995 plating criteria.

LOAD CASE(S) Standard

AUG 04 1998

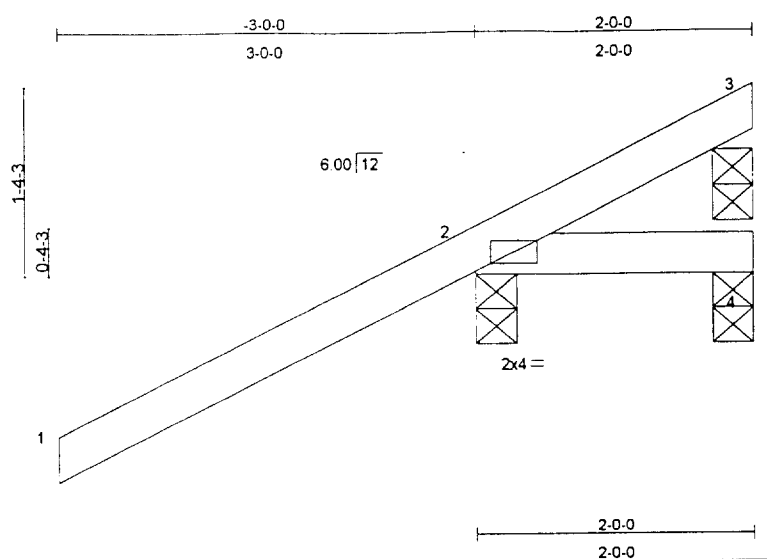


NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling, Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	D H CONST-DERN-
DH02	J2-2	JACK	10	1	

General Truss Co., Sacramento, CA 95828

4 0-32 s Dec 16 1997 MiTek Industries, Inc. Tue Aug 04 11:05:17 1998 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 16.0	Plates Increase 1.00	TC 0.45	Vert(LL) -0.00 2 >999	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.03	Vert(TL) 0.21 1-2 >171	
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a	
BCDL 7.0	Code UBC/ICBO	(Matrix)	Min Length / LL defl = 240	Weight: 10 lb

LUMBER
 TOP CHORD 2 X 4 DF No 1&Btr-G
 BOT CHORD 2 X 4 DF No 1&Btr-G

BRACING
 TOP CHORD Sheathed.
 BOT CHORD Rigid ceiling directly applied or 1-11-5 on center bracing.

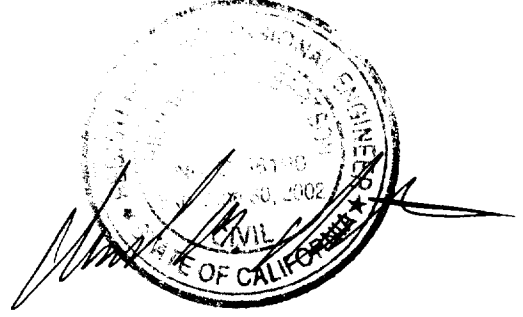
REACTIONS (lb/size) 3=-84/0-3-8, 2=395/0-3-8, 4=14/0-3-8, 4=14/0-3-8
 Max Horz 2=120(load case 5)
 Max Uplift 3=-84(load case 1), 2=-295(load case 5)
 Max Grav 3=125(load case 4), 2=395(load case 1), 4=33(load case 2), 4=14(load case 1)

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=79, 2-3=-93
 BOT CHORD 2-4=0

- NOTES**
- This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC/ICBO. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
 - All plates are M20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - A plate rating reduction of 20% has been applied for the green lumber members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 3 and 295 lb uplift at joint 2.
 - This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

AUG 04 1998

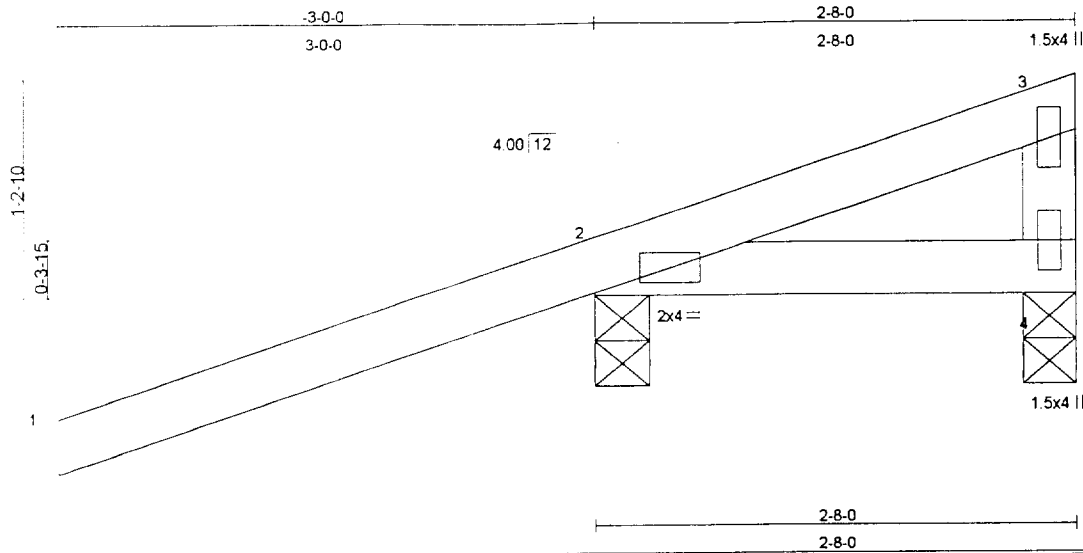


NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	D H CONST-DERN-
DH02	J28-2	MONO TRUSS	3	1	

General Truss Co. Sacramento, CA 95828

4 0-32 s Dec 16 1997 MiTek Industries, Inc. Tue Aug 04 11:05:19 1998 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 16.0	Plates Increase	1.00	TC 0.47	Vert(LL) -0.00 2-4 >999	M20 186/148
TCDL 14.0	Lumber Increase	1.25	BC 0.03	Vert(TL) 0.13 1-2 >287	
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL) 0.00 4 n/a	
BCDL 7.0	Code	UBC/ICBO		Min Length / LL defl = 240	Weight: 13 lb

LUMBER
TOP CHORD 2 X 4 DF No. 1&Btr-G
BOT CHORD 2 X 4 DF No. 1&Btr-G
WEBS 2 X 4 DF Std-G

BRACING
TOP CHORD Sheathed or 1-9-13 on center purlin spacing, except end verticals.
BOT CHORD Rigid ceiling directly applied or 1-8-8 on center bracing.

REACTIONS (lb/size) 2=277/0-3-8, 4=88/0-3-8
Max Horz 2=107(load case 5)
Max Uplift 2=186(load case 3), 4=27(load case 4)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=28, 2-3=0, 3-4=71
BOT CHORD 2-4=0

NOTES

- 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC/ICBO. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
- 2) All plates are M20 plates unless otherwise indicated.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
- 4) A plate rating reduction of 20% has been applied for the green lumber members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 186 lb uplift at joint 2 and 27 lb uplift at joint 4.
- 6) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

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NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

