

CITY OF SACRAMENTO

1231 I Street, Sacramento, CA 95814

Permit No: 0009234

Insp Area: 2

Site Address: 44 PARKSHORE CR SAC

Parcel No: 031-0220-044

Sub-Type: RES

Housing (Y/N): N

CONTRACTOR

MAK CHURK L.
1225 T ST
SACRAMENTO CA 95814

OWNER

YEE JOHN K/DORIS
44 PARKSHORE CR
SACRAMENTO CA 95831

ARCHITECT

Nature of Work: REROOF DURLITE TILE 35 SQS. RAIN GUTTERS, DRY ROT.

CONSTRUCTION LENDING AGENCY: I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

X License Class B License Number 617719 Date 4/31/01 Contractor Signature [Signature]

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code: any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00):

____ I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

____ I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code). The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law.

____ I am exempt under Sec. _____ B & PC for this reason: _____

Date _____ Owner Signature _____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

X Date 8/10/00 Applicant Agent Signature [Signature]

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

____ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

X CC I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier STATE FUND Policy Number 1271319-00 Exp Date 07/01/2001

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

X Date 8/10/00 Applicant Signature [Signature]

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.



SCHOEN ENGINEERING
 9524 BEDDINGTON WAY
 SACRAMENTO, CA 95827
 916-365-6666
 LIC: C042913

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May 11, 2000



ISSUED

AUG 16 2000

Churk Mak
 High Quality Construction
 1225 T Street
 Sacramento, CA 95814

SUBJECT: Roof structural inspection at 44 Park Shore Circle, Sacramento, CA 95831

Dear Churk:

On April 24th 2000 I inspected the roof structure of the residence at the above mentioned address. The roof was of standard construction with 2x6 Douglas fir No. 2 rafters @ 2' o.c.. The max spans in the house were 11' except for four rafters that framed down to the living room vault. In the garage the spans were 18'-6" from the front of the garage to the ridge. There were Full Dimension 6x12 support beams in the living room vault with a max span of 14'-6" with a cantilever to the front of 7'. Each of these beams supported approx 1/4 of the 31' wide living room vault. There was also a full dimension 6x12 ridge beam in the family room vault spanning 18'-6"

The following modifications should be made prior to reroofing:

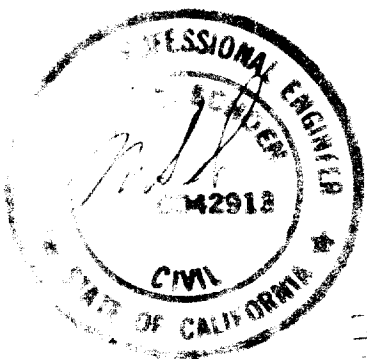
SUBJECT TO FIELD INSPECTION AND APPROVAL

- #1 * The rafters in the garage are overspan. Install two 1-3/4"x16" Microlam purlins in the garage one about 6' back from the garage door and one about 12' back(see attached sketch for details).
- #2 * At the intersection of the main house wing and the living room vault the main wing rafters are blind framed onto the vault. The first two longest valley jack rafters on both sides of the living room vault should be braced off of the central bearing wall or the vault beams to reduce the spans to 12'.
- #3 * Some of the purlin braces in the attic areas of the house are framed so as to run by the purlin and then rely on nailing and a 2x4 block nailed to the brace underneath to support the purlin. These are not adequate to support the proposed roof loads and should be redone. These braces should either be replaced by braces providing direct bearing for the supported member or modified to provide direct bearing(see sketch for details and plan for location).

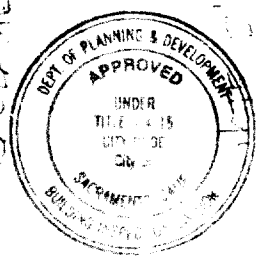
It is my finding that this structure is adequate for the following : 30 # tarred felt installed over the existing plywood sheathing, 1x2 batts; Concrete tile weighing 6 lbs./sq.ft. or less.

NOTE: it is possible when reroofing that the increased load to structural elements also supporting wall, ceiling and floor finishes could cause some minor cosmetic cracking of these finishes. This is typical of wood framed structures and does not of itself indicate structural inadequacy of these members.

Matt P 8/10/00

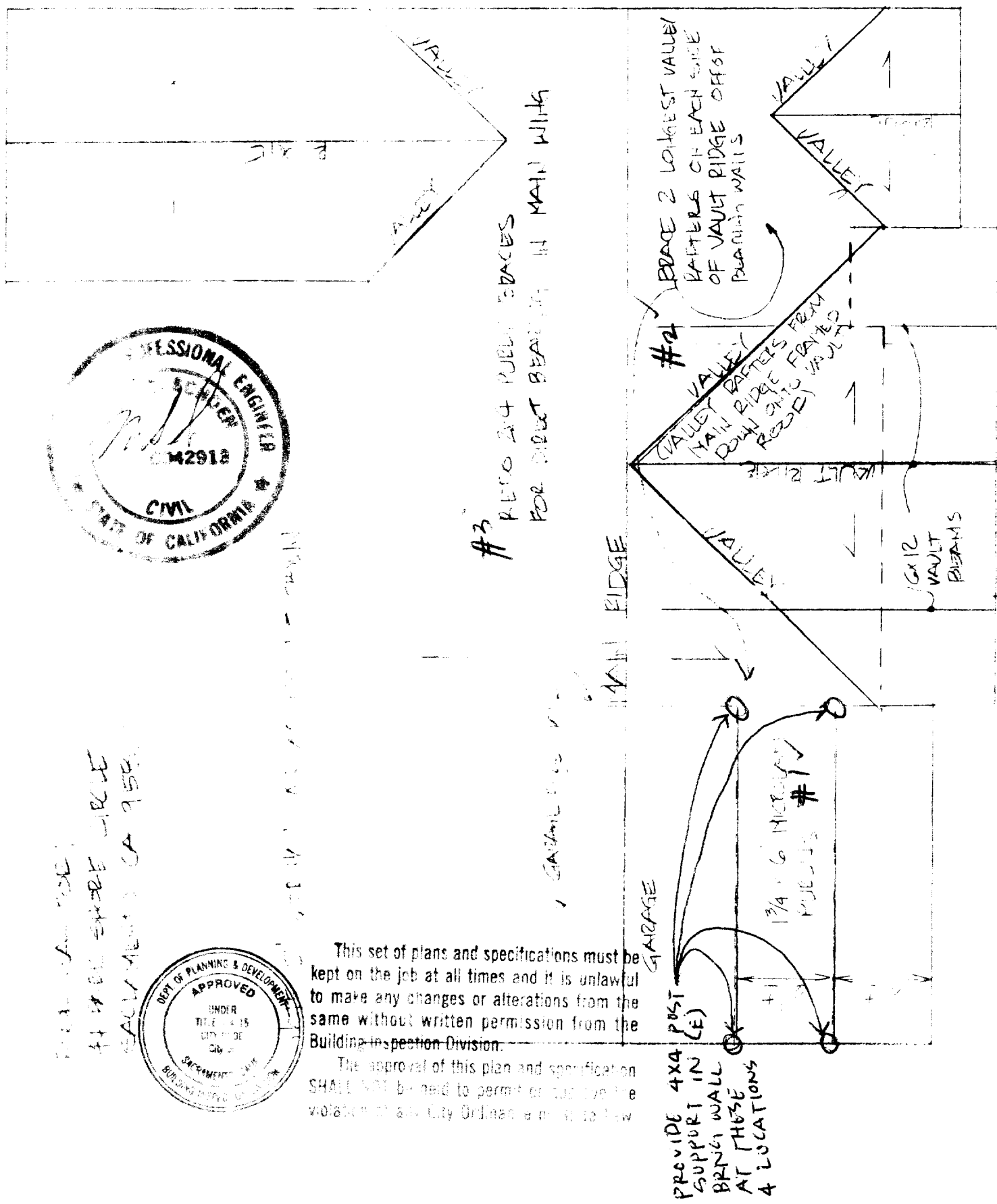


SEE PLAN SHEET
 #1 BEARING WALL
 EACH END OF 95'



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PROVIDE 4x4 POST SUPPORT IN BRICK WALL AT THESE 4 LOCATIONS

NOTE: SEE DRAWING

2x4 RAFTER

1 3/4" MIKROALM
PURLIN W/ 3-16d
TO BRACE
2x4 BRACE 4' O.C.
W/ 2-16d TO RAFTER
W/ PURLIN

DOUBLED 2x4
PURLIN BRACE
@ LOAD BEARING
WALLS NAIL EA.
2x4 TO W.P.W. W/
4-8d TIE NAILS

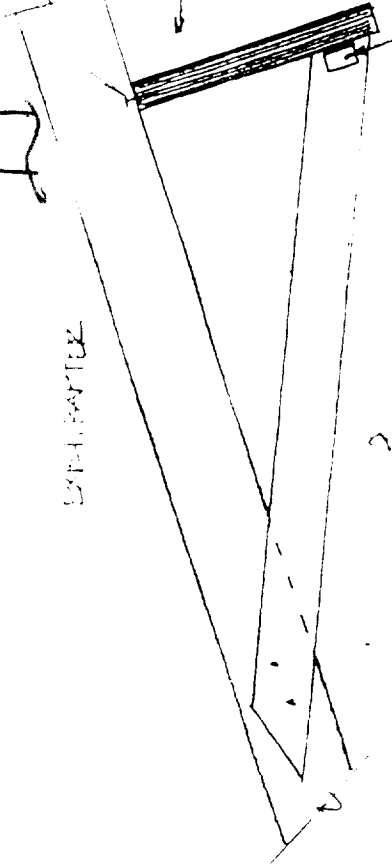
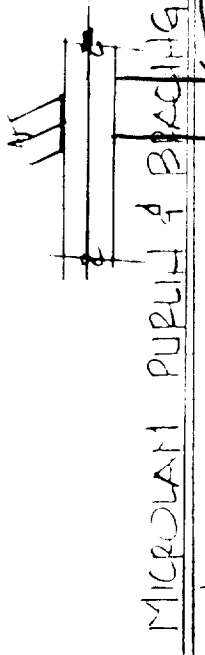
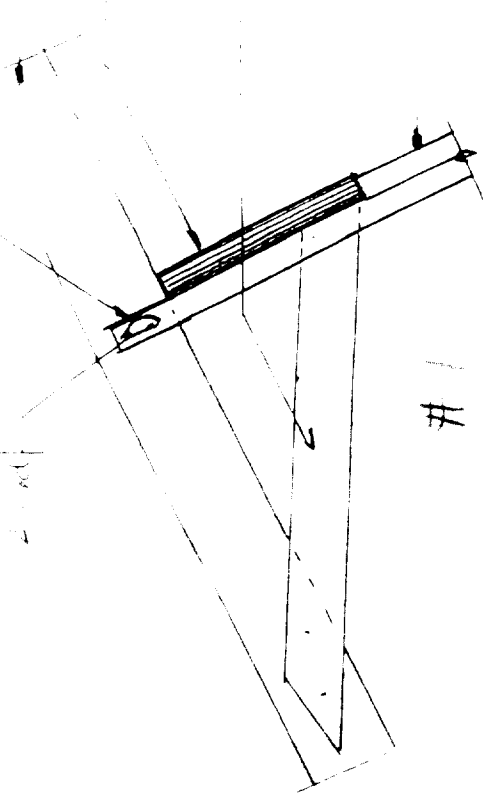
DOUBLE PLATE OF
LOAD BEARING WALL
BE TOP OF PURLIN

INSTALL (N) 4x4
POST AT CA
BRANCH LOCATION

2x4 TIE NAILS
TRAP TO
MIKROALM

MIKROALM

2x4 BRACE 9' O.C.
W/ 2-16d TO
RAFTER & A 3x4
W/ 2-8d TO
MIKROALM



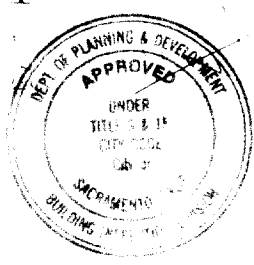
BRACING OF MIKROALM
PURLIN IN THE FIELD



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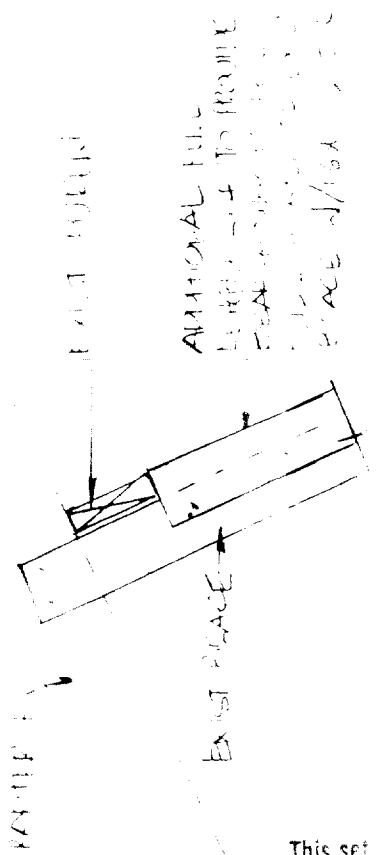
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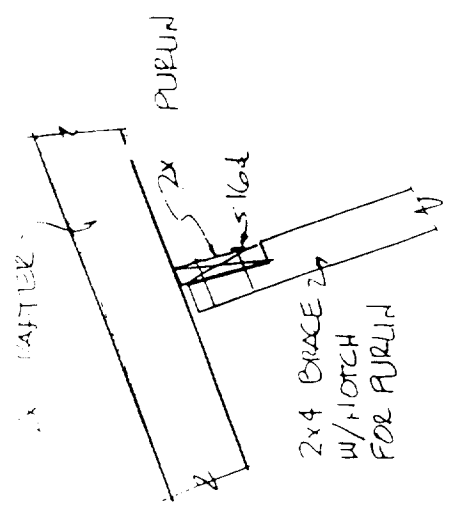
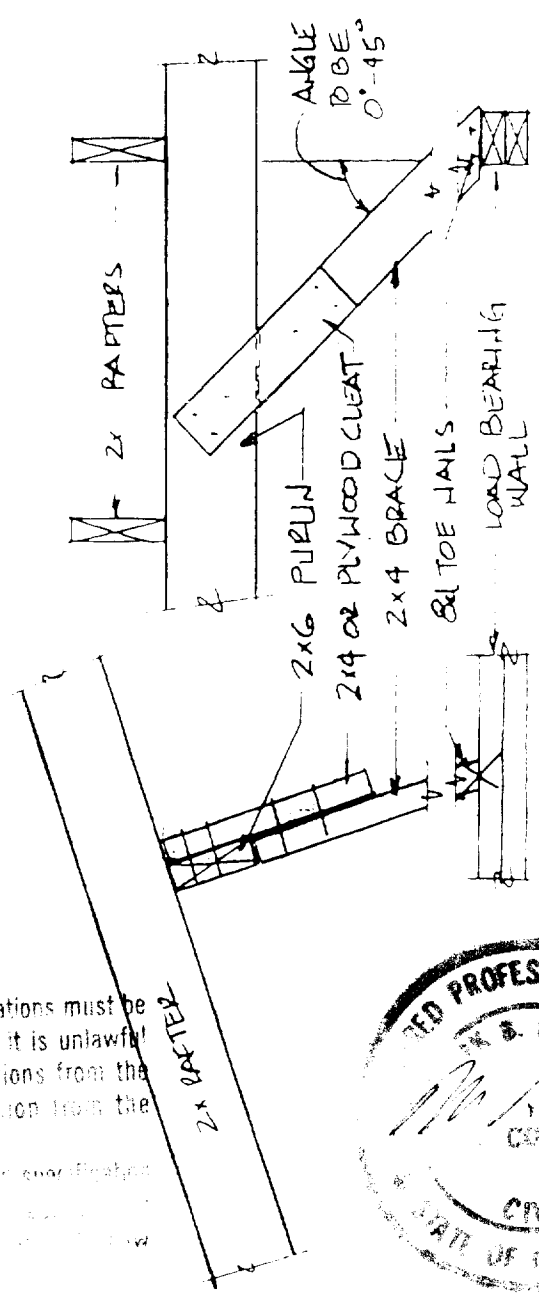


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BRACE FOR DIRECT BEARING



2x4 BRACE TO PURLIN CONNECTIONS



Calculation for the required area, section modulus and moment of inertia for simple span wood beams. Dead load (dl) and Live load (ll) are in pounds per square ft., Spans (l) and Tributary load length or spacing (sp) are in ft., Areas are in sq.in., Section moduli are in inches cubed and Moments of inertia are in inches to the 4th power. Allowable stresses (Fy),(Fb),(Fv) are in lbs./sq.in. per 1991 U.B.C.

WOOD HEADER BEAM

$$\begin{aligned}
 rdl &:= 15 & rll &:= 16 & rta &:= 16 \cdot 61 & l &:= 18.5 \\
 fdl &:= 9 & fl &:= 40 & fta &:= \frac{0}{2} & rta \cdot l &:= 180.56 \\
 wt &:= (rta \cdot (rdl + rll) + fta \cdot (fdl + fl)) \cdot 6 & wt &:= 318.56 & Cd &:= 1.25 & Cf &:= 1. \\
 Fb &:= 1250 & Cr &:= 1 & Fbp &:= Fb \cdot Cd \cdot Cf \cdot Cr & Fbp &:= 1562.5 & Ew &:= 1700000 & Fv &:= 95 \cdot Cd
 \end{aligned}$$

A min. required =

$$l \cdot \frac{wt}{2} \cdot \frac{3}{2} = 37.221$$

S min. required =

$$wt \cdot l^2 \cdot \frac{1.5}{Fbp} = 104.666$$

I min. required =

$$5 \cdot wt \cdot \frac{(l \cdot 12)^4}{12 \cdot 384 \cdot Ew \cdot l \cdot \frac{12}{240}} = 533.912$$

Check Beam properties:

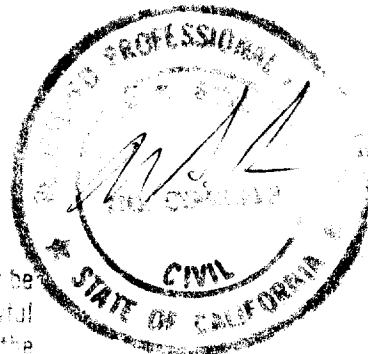
$$\begin{aligned}
 CF &:= \frac{12}{d} & A &:= w \cdot d & S &:= w \cdot CF \cdot \frac{d^2}{6} & I &:= w \cdot \frac{d^3}{12} & Stiffw &:= I \cdot Ew
 \end{aligned}$$

A = 72 > 37 S = 144 > 105 I = 864 > 534 therefore O.K.



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Calculation for the required section modulus and moment of inertia for simple span wood beams. Dead load(Dl) and Live load(Ll) are in pounds per square ft., Spans(l) and Tributary load length or spacing(sp) are in ft., Section moduli are in inches cubed and Moments of inertia are in inches to the 4th power. Allowable stress (Fy) is in lbs./sq.in. per Manufacturer's specifications. Section modulus shape factor reduction and load modification are per U.B.C. 1997 edition.

MICROLAM BEAM

$r_{dl} := 12$ $r_{ll} := 14$ $r_{ta} := 7$ $r_{fa} := 21.5$
 $f_{dl} := 8$ $f_{ll} := 40$ $f_{ta} := \frac{0}{2}$ $r_{fa} := 150.5$
 $W_{dl} := 16 \cdot \frac{1.75}{144} \cdot 35$ $W_{ll} = 6.806$ $E := 2000000$ $F_b := 2800 \cdot 1.25$

$w_t := r_{ta} \cdot (r_{dl} + r_{ll}) + f_{ta} \cdot (f_{dl} + f_{ll}) + W_{dl}$

S min. required = $(W_{dl} + w_t) \cdot l^2 \cdot \frac{1.5}{F_b} = 38.752$

I min. required = $5 \cdot (W_{dl} + w_t) \cdot \frac{(l \cdot 12)^4}{12 \cdot 384 \cdot E \cdot 1.25} = 437.413$

Use two 1-3/4"x16" MicroLam beams one back approx.6' and one back approx.12' from the front of the gargae.

$w = 1.75$ $d = 16$ $C_f := \frac{12^{1.9}}{d}$
 $S = C_f \cdot w \cdot \frac{d^2}{6}$ $I = w \cdot \frac{d^3}{12}$

S = 72.318 > 15.6 I = 597.333 > 125 therefore 1-3/4"x16" MLB is O.K.



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Calculation for the required area, section modulus and moment of inertia for simple span wood beams.
 Dead load(DL) and Live load(LL) are in pounds per square ft., Spans(L) and Tributary load length or spacing(sp) are in ft. Areas are in sq.in., Section moduli are in inches cubed and Moments of inertia are in inches to the 4th power. Allowable stresses (Fy),(Fb),(Fv) are in lbs./sq.in. per 1991 U.B.C.

WOOD HEADER BEAM

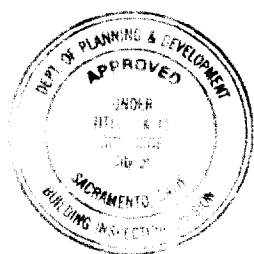
$rdl := 15$ $rl := 10$ $rta := 31 \cdot 25$ $l := 14.5$
 $fdl := 9$ $fl := 40$ $fta := \frac{0}{2}$ $rta \cdot l = 112.375$
 $wt := (rta \cdot (rdl + rl) + fta \cdot (fdl + fl)) + bc \cdot st = 256.25$ $Cd := 1.25$ $Cf := 1.$
 $Fb := 1250$ $Cr := 1.$ $Fbp = Fb \cdot Cd \cdot Cf \cdot Cr$ $Fbp = 1562.5$ $Ew := 1700000$ $Fv := 95 \cdot Cd$

$A \text{ min. required} = \frac{l \cdot \frac{wt}{2} \cdot \frac{3}{2}}{Fv} = 23.467$
 $S \text{ min. required} = \frac{wt \cdot l^2 \cdot \frac{1.5}{Fbp}}{6} = 51.721$
 $I \text{ min. required} = \frac{5 \cdot wt \cdot (l \cdot 12)^4}{12 \cdot 384 \cdot Ew \cdot l \cdot \frac{12}{240}} = 206.791$

Check Beam properties:

$w := 6$ $d := 12$
 $CF := \frac{12}{d}$ $s = w \cdot d$ $S = w \cdot CF \cdot \frac{d^2}{6}$ $I = w \cdot \frac{d^3}{12}$ $Stiffw := I \cdot Ew$

$A = 72 > \cancel{52.7}$ $S = 144 > \cancel{57.521}$ $I = 864 > \cancel{260.207}$ therefore O.K.



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SCHOEN ENGINEERING
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 9524 BEDDINGTON WAY
 SACRAMENTO, CA 95827
 (916) 369-6866
 LIC # C-002913

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 May 11, 2000



ISSUED

AUG 16 2000

Churk Mak
 High Quality Construction
 1225 T Street
 Sacramento, CA 95814

SUBJECT: Roof structural inspection at 44 Park Shore Circle, Sacramento, CA 95831
 DEPARTMENT OF PLANNING & DEVELOPMENT SERVICES

Dear Churk:

On April 24th 2000 I inspected the roof structure of the residence at the above mentioned address. The roof was of standard construction with 2x6 Douglas fir No. 2 rafters @ 2' o.c.. The max spans in the house were 11' except for four rafters that framed down to the living room vault. In the garage the spans were 18'-6" from the front of the garage to the ridge. There were Full Dimension 6x12 support beams in the living room vault with a max span of 14'-6" with a cantilever to the front of 7'. Each of these beams supported approx 1/4 of the 31' wide living room vault. There was also a full dimension 6x12 ridge beam in the family room vault spanning 18'-6"

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It is my finding that this structure is adequate for the following : 30 # tarred felt installed over the existing plywood sheathing. 1x2 batts; Concrete tile weighing 6 lbs./sq.ft. or less.

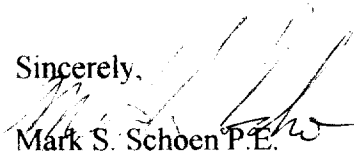
NOTE: it is possible when reroofing that the increased load to structural elements also supporting wall, ceiling and floor finishes could cause some minor cosmetic cracking of these finishes. This is typical of wood framed structures and does not of itself indicate structural inadequacy of these members.

Matt P. 8/10/00

This report deals with the structural adequacy of roof supporting members that were readily observable. It does not address any structure that was covered by wall finishes, buried in the ground or was otherwise not observable. Any such structures were assumed to conform to standard construction specifications in the Uniform Building Code. Also, it does not address any existing deflection or warping of roof surfaces, nor is it guaranteed that any structural modifications that may be listed in this report will remove such deflections or warping. The repair of such deflections or warping to improve architectural appearance is at the option of the building owner and the roofing contractor

I would like to thank you for allowing me to provide my services in this matter. Please let me know if I may be of further assistance

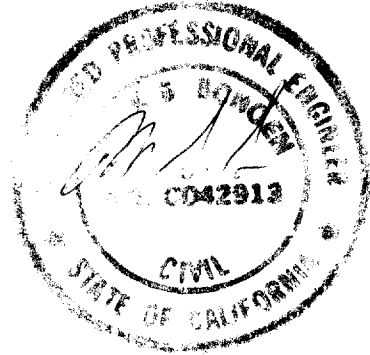
Sincerely,



Mark S. Schoen P.E.

MSS:mss
S-ENG-2000.HQC001.001

FOR MAIL ROOM MAY BE USED M.S.



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