CITY OF SACRAME	NTO	Permit No:	0506397
1231 I Street, Sacramento,	, CA 95814	Insp Area:	2
		Thos Bros:	337B3
机铸铁 医二甲基甲基甲基			
Site Address: 7574 RIVER R	RANCH WY SAC	Sub-Type:	RES
Parcel No: 031-0790-030		Housing (Y/N)	: N
CONTRACTOR	OWNER	ARCHITECT	
GUIRRE ROOFING	MARQUEZ MARCILINO H/SILVIA T	ARCHITECT	
515 BINGHAMPTON DR ACRAMENTO CA 95834	7574 RIVER RANCH WY SACRAMENTO, CA 95831		
ature of Work: T/O, RESHEE	ET, REROOF 40SQ LIGHTWEIGHT TILE		in the House Service
ONSTRUCTION LENDING AG	ENCY: I hereby affirm under penalty of perjury that there	is a construction lending agenc	y for the performance
e work for which this permit is issued (So	ec. 3097, Civ. C).		
ender's Name	Lender'sAddress		
ICENSED CONTRACTORS DE	ECLARATION: I hereby affirm under penalty of perju	ry that I am licensed under r	provisions of Chapte
ommencing with section 7000) of Division	on 3 of the Business and Professions Code and my license is in	full force and effect.	o chapte
cense Class <u>639</u> License Number	765880 Date 5-6-05 Contractor Signatur	flac la	
	ON: I hereby affirm under penalty of perjury that I am exem	_	
ason (Sec. 7031.5, Business and Professi	ions Code; any city or county which requires a permit to cons	truct, alter, improve, demolish	or renair any structu
or to its issuance, also requires the application	icant for such permit to file a signed statement that he or she is	s licensed nursuant to the provi	sions of the Contract
cense Law (Chapter 9 (commencing with	h Section 7000) of Division 8 of the Business and Profession	s Code) or that he or she is ex	empt therefrom and
and the aneged exemption. Any viola	ation of Section 7031.5 by any applicant for a permit subject	s the applicant to a civil penali	ty of not more than
ndred dollars (\$500.00);			
I, as a owner of the property, or my le (Sec. 7044, Business and Professional to does such work himself or herself or the second	y employees with wages as their sole compensation, will do the l Code: The Contractors License Law does not apply to an orthrough his/her own employees, provided that such improvent one year of completion, the owner builder will have the builder.	wher of property who builds or tents are not intended or offers	improves thereon,
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THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

GREGORIN DESIGN

9469 FORT WORTH WAY SACRAMENTO, CA 95827 DATE: 10-25-05



Prepared for:

Aguirre Roofing

Modifications

Sec

3515 Binghampton Drive

Sacramento, CA 95834



From the reactions of the the beam models included with these calculations the nails required for the ceiling joist tie/roof rafter system as exists can be determined by summation of moments about ridge point of a freebody diagram through the center of tied roof system. (see P3.2.)

 $\frac{1}{2}$ half span of tied roof system = 39.5'/2 = 19.75' Span to interior support = 19.5'/2 = 9.75'

Height @ center (exterior wall top plate to ridge bd.) = 19.75' x8"/12 =13'

Load case 1 - DL + LL all spans

Reaction exterior wall = 352#
Reaction interior support = 2035#

 Σ M = [352# x19.75' + 2035# x9.75' - 4' spacing(9.8+16)psf (19.75²/2)] = 6666 ft-lb

Load case 2 - DL all spans; LL over center span only (betw. Interior supports)

Reaction exterior wall = 123#
Reaction interior support = 1685#

 Σ M = [123# x19.75' + 1685# x9.75' - 4' spacing(9.8psf)(19.75²/2) - 4' spacing(16psf)(9.75²/2)]

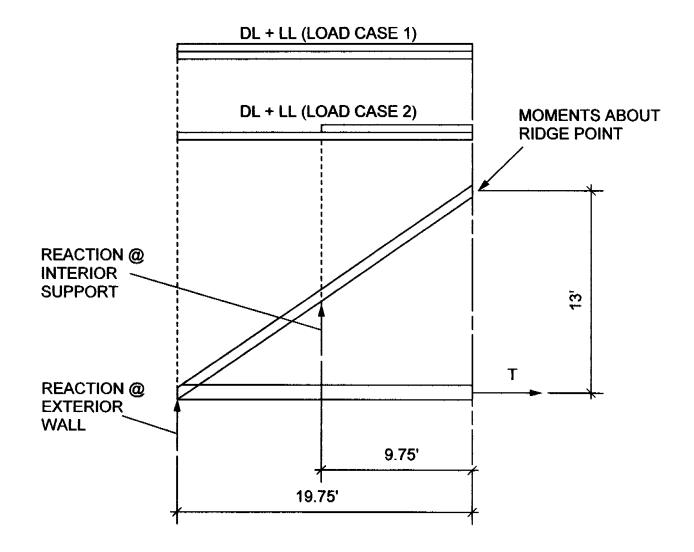
= 8170 ft-lb > 6666 ft-lb Therefore Load case 2 governs.

T = 8170 ft-lb / 13' = 629#

Number of 16d nails required = 629# / (1.25X141 #/nail) = 3.57 nails

OFFICE/CITY COPY

JKG 12.09.05



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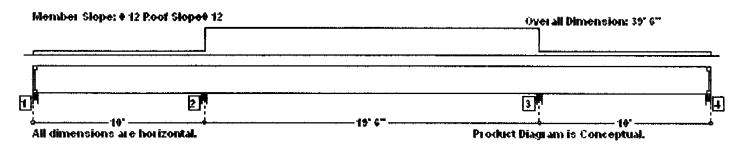
FREEBODY DIAGRAM

Page 1 Engine Version: 1.16.5

NAILING FOR TIED ROOF USING CONT. BEAM MODEL. DD ALL SPANS; LL CENTER SPAN ONLY

3 1/2" x 11 7/8" 2.0E Parallam® PSL

THIS PRODUCT MEETS OR EXCEEDS THE SET DESIGN **CONTROLS FOR THE APPLICATION AND LOADS LISTED**



Analysis is for a Header (Flush Beam) Member. Tributary Load Width: 4' Primary Load Group - Roof (psf): 0.0 Live at 125 % duration, 9.8 Dead Vertical Loads:

Live Dead Location **Application Comment** Type Uniform(plf) Roof(1.25) 64.0 0.0 10' To 29' 6" Adds To

SUPPORTS:

		Input Width	Bearing Length	Vertical Reactions (lbs) Live/Dead/Uplift/Total	Detail	Other
1	Stud wall	3.50"	1.50"	-154 / 123 / -33 / 123	L1: Blocking	1 Ply 1 1/4" x 11 7/8" 1.3E TimberStrand® LSL
2	Stud wall	3.50"	1.50"	778 / 907 / 0 / 1685	R7	None
3	Stud wall	3.50"	1.50"	778 / 907 / 0 / 1685	R7	None
4	Stud wall	3.50"	1.50"	-154 / 123 / -33 / 123	L1: Blocking	1 Ply 1 1/4" x 11 7/8" 1.3E TimberStrand® LSL

⁻See TJ SPECIFIER'S / BUILDERS GUIDE for detail(s): L1: Blocking,R7

DESIGN CONTROLS:

	Maximum	Design	Control	Control	Location
Shear (lbs)	1133	-1001	10044	Passed (10%)	Rt. end Span 2 under Roof loading
Moment (Ft-Lbs)	-2913	-2913	24878	Passed (12%)	MID Span 3 under Roof ADJACENT span loading
Live Load Defl (in)		0.090	0.650	Passed (L/999+)	MID Span 2 under Roof loading
Total Load Defl (in)		0.149	0.975	Passed (L/999+)	MID Span 2 under Roof loading

⁻Deflection Criteria: MINIMUM(LL:L/360,TL:L/240).

ADDITIONAL NOTES:

-IMPORTANT! The analysis presented is output from software developed by Trus Joist (TJ). TJ warrants the sizing of its products by this software will be accomplished in accordance with TJ product design criteria and code accepted design values. The specific product application, input design loads, and stated dimensions have been provided by the software user. This output has not been reviewed by a TJ Associate.

PROJECT INFORMATION:

7574 RIVER RANCH WAY

OPERATOR INFORMATION:

frank gregorin gregorin design \$ engineering 9469 fort worth wy sacramento, CA 95827 Phone: 916-362-3230 gregorin@sbcglobal.net

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⁻Bracing(Lu): All compression edges (top and bottom) must be braced at 14' 7" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

⁻The load conditions considered in this design analysis include alternate and adjacent member pattern loading.

⁻Design assumes adequate continuous lateral support of the compression edge.

⁻Not all products are readily available. Check with your supplier or TJ technical representative for product availability.

⁻THIS ANALYSIS FOR TRUS JOIST PRODUCTS ONLY! PRODUCT SUBSTITUTION VOIDS THIS ANALYSIS.
-Allowable Stress Design methodology was used for Building Code UBC analyzing the TJ Distribution product listed above.



3 1/2" x 11 7/8" 2.0E Parallam® PSL

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THIS PRODUCT MEETS OR EXCEEDS THE SET DESIGN CONTROLS FOR THE APPLICATION AND LOADS LISTED

Load Group: Primary Load Group

	^ 9' 10 00" ^
9' 10.00" ^ 19' 6.00"	
	1685 123
	778 –154
	50(W) 1.50(W)
Max. Unbraced Length (in) 175 175 175	175 175
Loading on all spans, LDF = 0.90 , 1.0 Dead	
Design Shear (lbs) 56 -339 449 -44	9 339 -56
-	9 398 -115
	907 115
	907 123
	1396 126
Loading on all spans, LDF = 1.25 , 1.0 Dead + 1.0 Floor + Design Shear (lbs) -98 -494 1001 -100	
• • • • • • • • • • • • • • • • • • • •	
, , , , , , , , , , , , , , , , , , , ,	1685 -40
	1685 -31 2913 N/A
Live Deflection (in) -0.019 0.090	·
	-0.019
Total Deflection (in) -0.025 0.149	-0.025
ALTERNATE span loading on odd # spans, LDF = 1.25 , 1.0 Dead +	1.0 Floor + 1.0 Roof
Design Shear (lbs) +21 -416 725 -72	5 416 21
Max Shear (lbs) 37 -476 821 -82	1 476 -37
Member Reaction (lbs) 37 1296	1296 37
Support Reaction (lbs) 46 1296	1296 46
Moment (Ft-Lbs) 13 -2155 1846 -	2155 13
Live Deflection (in) -0.010 0.045	-0.010
Total Deflection (in) -0.015 0.104	
Total Deflection (in) -0.015 0.104	-0.015
, ,	
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead +	1.0 Floor + 1.0 Roof
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs) -98 -494 1001 -100	1.0 Floor + 1.0 Roof 1 494 98
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs)	1.0 Floor + 1.0 Roof 1 494 98 3 553 40
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs)	1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs)	1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40 1685 -33
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs)	1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40 1685 -33 2913 N/A
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs) -98 -494 1001 -100 Max Shear (lbs) -40 -553 1133 -113 Member Reaction (lbs) -40 1685 Support Reaction (lbs) -33 1685 Moment (Ft-Lbs) N/A -2913 2608 - Live Deflection (in) -0.019 0.090	1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40 1685 -33 2913 N/A -0.019
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs)	1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40 1685 -33 2913 N/A
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs) -98 -494 1001 -100 Max Shear (lbs) -40 -553 1133 -113 Member Reaction (lbs) -40 1685 Support Reaction (lbs) -33 1685 Moment (Ft-Lbs) N/A -2913 2608 - Live Deflection (in) -0.019 0.090	1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40 1685 -33 2913 N/A -0.019 -0.025
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs)	1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40 1685 -33 2913 N/A -0.019 -0.025
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs)	1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40 1685 -33 2913 N/A -0.019 -0.025 1.0 Floor + 1.0 Roof 1 494 98
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs)	1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40 1685 -33 2913 N/A -0.019 -0.025 1.0 Floor + 1.0 Roof 1 494 98
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs)	1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40 1685 -33 2913 N/A -0.019 -0.025 1.0 Floor + 1.0 Roof 1 494 98 3 553 40
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs)	1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40 1685 -33 2913 N/A -0.019 -0.025 1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40
ALTERNATE span loading on even # spans, LDF = 1.25 , 1.0 Dead + Design Shear (lbs)	1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40 1685 -33 2913 N/A -0.019 -0.025 1.0 Floor + 1.0 Roof 1 494 98 3 553 40 1685 -40 1685 -33

PROJECT INFORMATION:

7574 RIVER RANCH WAY

OPERATOR INFORMATION:

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NAILING FOR TIED ROOF USING CONT. BEAM MODEL. DL + LL ALL SPANS

3 1/2" x 11 7/8" 2.0E Parallam® PSL

THIS PRODUCT MEETS OR EXCEEDS THE SET DESIGN **CONTROLS FOR THE APPLICATION AND LOADS LISTED**

Member Slop	pe: 0 12 Roof Slope0 12	Overall Dimension: 39° 6"
	2	[3] [*]
All dimension	ns are horizontal.	Product Diagram is Concentral

LOADS:

Analysis is for a Header (Flush Beam) Member. Tributary Load Width: 4' Primary Load Group - Roof (psf): 0.0 Live at 125 % duration, 9.8 Dead Vertical Loads:

Live Dead Location Application Comment Type Class 0 To 39' 6" Adds To Uniform(plf) Roof(1.25) 64.0 0.0

SUPPORTS:

<u> </u>	ironio.	Input Width	Bearing Length	Vertical Reactions (lbs) Live/Dead/Uplift/Total	Detail	Other
1	Stud wall	3.50"	1.50"	228 / 123 / 0 / 352	L1: Blocking	1 Ply 1 1/4" x 11 7/8" 1.3E TimberStrand® LSL
2	Stud wall	3.50"	1.50"	1128 / 907 / 0 / 2035	R7	None
3	Stud wall	3.50"	1.50"	1128 / 907 / 0 / 2035	R7	None
4	Stud wall	3.50"	1.50"	228 / 123 / 0 / 352	L1: Blocking	1 Ply 1 1/4" x 11 7/8" 1.3E TimberStrand® LSL

⁻See TJ SPECIFIER'S / BUILDERS GUIDE for detail(s): L1: Blocking,R7

DESIGN CONTROLS:

	Maximum	Design	Control	Control	Location
Shear (lbs)	-1143	-1011	10044	Passed (10%)	Rt. end Span 2 under Roof ADJACENT span loading
Moment (Ft-Lbs)	-3157	-3157	24878	Passed (13%)	Bearing 3 under Roof ADJACENT span loading
Live Load Defl (in)		0.081	0.650	Passed (L/999+)	MID Span 2 under Roof ALTERNATE span loading
Total Load Defl (in)		0.141	0.975	Passed (L/999+)	MID Span 2 under Roof ALTERNATE span loading

⁻Deflection Criteria: MINIMUM(LL:L/360,TL:L/240).

ADDITIONAL NOTES:

-IMPORTANT! The analysis presented is output from software developed by Trus Joist (TJ). TJ warrants the sizing of its products by this software will be accomplished in accordance with TJ product design criteria and code accepted design values. The specific product application, input design loads, and stated dimensions have been provided by the software user. This output has not been reviewed by a TJ Associate.

PROJECT INFORMATION:

7574 RIVER RANCH WAY

OPERATOR INFORMATION:

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⁻Bracing(Lu): All compression edges (top and bottom) must be braced at 14' 7" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability

⁻The load conditions considered in this design analysis include alternate and adjacent member pattern loading.

⁻Design assumes adequate continuous lateral support of the compression edge.

⁻Not all products are readily available. Check with your supplier or TJ technical representative for product availability.

⁻THIS ANALYSIS FOR TRUS JOIST PRODUCTS ONLY! PRODUCT SUBSTITUTION VOIDS THIS ANALYSIS.

⁻Allowable Stress Design methodology was used for Building Code UBC analyzing the TJ Distribution product listed above.



NAILING FOR TIED ROOF USING CONT. BEAM MODEL. DL + LL ALL SPANS

3 1/2" x 11 7/8" 2.0E Parallam® PSL

THIS PRODUCT MEETS OR EXCEEDS THE SET DESIGN **CONTROLS FOR THE APPLICATION AND LOADS LISTED**

Load Group: Primary Load Group

<u>^</u> 9	10.	.00"1	9' 6.	00"	^	9'	10.00"	^
Max. Vertical Reaction Total (1bs) 352		2035)		2035			352
Max. Vertical Reaction Live (1bs) 228		1128			1128			228
Required Bearing Length in 1.50(W)		1.50(W)		1.	50 (W	(1)		1.50(W)
Max. Unbraced Length (in)	175	175	175		175	1	.75	
•								

Loading on al.	l spans, LDF =	0.90 , 1.0 De	ad	
Design Shear (lbs)	56	-339 449	-449 339	-56
Max Shear (lbs)	115	-398 509	-509 398	-115
Member Reaction (lbs)	115	907	907	115
Support Reaction (lbs)	123	907	907	123
Moment (Ft-Lbs)	126	-1396 1084	-1396 126	

Loading on all spans,	LDF = 1.25 , 1	.0 Dead + 1.	.0 Floor + 1.	0 Roof
Design Shear (lbs)	126	-755 10	001 -1001	755 -126
Max Shear (lbs)	255	-887 11	133 -1133	887 -255
Member Reaction (lbs)	255	2020	202	0 255
Support Reaction (lbs)	274	2020	202	0 274
Moment (Ft-Lbs)	2	80 -3108	2413 -310	8 280
Live Deflection (in)	-	0.008	0.073	-0.008
Total Deflection (in)	-	0.014	0.132	-0.014

ALTERNATE span loading on odd #	spans, LDF =	1.25 , 1.0 0	Dead + 1.0 Floor	+ 1.0 Roof
Design Shear (lbs)	203	-678 725	-725 678	-203
Max Shear (1bs)	332	-810 821	-821 810	-332
Member Reaction (lbs)	332	1631	1631	332
Support Reaction (lbs)	352	1631	1631	352
Moment (Ft-Lbs)	475	-2349 1 6	551 -2349 47	5
Live Deflection (in)	0.00	5 0.	.028 0.0	005
Total Deflection (in)	-0.0	07 0.	.087 -0	. 007

ALTERNATE span loading on even #	spans, LDF =	1.25 , 1	.0 Dead + 1.0	Floor + 1.0 Roof
Design Shear (lbs)	14	-624 10	01 -1001 62	24 -14
Max Shear (lbs)	108	-720 11	33 -1133 72	-108
Member Reaction (lbs)	108	1853	1853	108
Support Reaction (lbs)	119	1853	1853	119
Moment (Ft-Lbs)	69	-3011	2511 -3011	69
Live Deflection (in)	-0.0	13	0.081	-0.013
Total Deflection (in)	-0.0	19	0.141	-0.019

ADJACENT span loading over support	# 2, LDF =	1.25, 1.	0 Dead + 1.0	Floor	+ 1.0 Roof
Design Shear (lbs)	121	-760 10	11 -991	619	-19
Max Shear (lbs)	250	-892 11	43 -1123	715	-113
Member Reaction (lbs)	250	2035	183	38	113
Support Reaction (lbs)	269	2035	183	88	124
Moment (Ft-Lbs)	269	-3157	2462 -296	52 75	
Live Deflection (in)	-0.0	08	0.077	-0.0	12
Total Deflection (in)	-0.0	14	0.136	-0.0	18

PROJECT INFORMATION: 7574 RIVER RANCH WAY

OPERATOR INFORMATION:

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