

CITY OF SACRAMENTO
1231 I Street, Sacramento, CA 95814

Permit No: 0011600

Insp Area: 1

Site Address: 640 34TH ST SAC
Parcel No: 004-0271-006

Sub-Type: REM
Housing (Y/N): N

CONTRACTOR
HER LAND ENT
2604 21ST ST
SACRAMENTO CA 95818

OWNER
DAWSON GREG/MATTINGLY JENNIFER
640 34TH ST
SACRAMENTO CA 95816

ARCHITECT

Nature of Work: INTERIOR REMODEL OF FIRST FLOOR BATH, KITCHEN, LAUNDRY AND CONNECTING AREAS, +65 SF KTCHN ADD'N, +913 SF 2ND FLR ADD'N, -147 SF CVRD PATIO, -147 2ND FLR DECK

CONSTRUCTION LENDING AGENCY: I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C.)

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class _____ License Number _____ Date _____ Contractor Signature _____

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00):

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code. The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code. The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. _____ B & PC for this reason: _____

Date 10/20/00 Owner Signature _____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 10/20/00 Applicant/Agent Signature _____

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:
I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier STATE FUND Policy Number 0714906-99 Exp Date 11/01/2000

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 10/20/00 NEIGHBORHOODS PLANNING AND DEVELOPMENT SERVICES _____

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

Date of Request: _____
By: _____

CITY OF SACRAMENTO DEVELOPMENT SERVICES DIVISION
PLANNING AND ZONING INFORMATION REQUEST

Project Address: 640 34th St

Assessor's Parcel Number: 004-0271-006

Previous Use: _____

Description of Request/Proposed Use: Add second story

Is This a Change of Use? NO

Zoning Designation: R-1

Prior Applications for Project Site(P#, Z#, DRPB#): _____

Comments: In Alhambra Corridor - must have Design Review, NO Issuance of permits until D.R. application appeal period over

Are There Any Planning Issues?: (circle one) YES NO

- * Staff Site Plan Check Required? (Circle one) YES NO
- * Field Inspection Required? (Circle one) YES NO
- * Design Review/Preservation Required?: (Circle one) YES NO

Planning Review by/Date: S. J. R. 14 Sep 00

A list of items that must be reviewed by Planning is provided on the reverse side of this form.

MICROFILM AFTER FINAL

OWNER-BUILDER VERIFICATION

ATTENTION PROPERTY OWNERS

An owner-builder building permit has been applied for in your name and bearing your signature.

Please complete and return this information in the envelope provided at your earliest opportunity to avoid unnecessary delay in processing and issuing your building permit. No building permit will be issued until this verification is received.

1. I personally plan to provide the major labor and materials for construction of the proposed Improvement (yes or no) _____
2. I (have have not) _____ signed an application for A building permit for the proposed work.

3. I have contracted with the following person (firm) to provide the proposed construction:

Name HER LAND ENT Address 2604 21ST ST

City SAC Telephone 979-9309

Contractors License No. 462840

4. I plan to provide portions of the work, but I have hired the following person to coordinate, Supervise, and provide the major work.

Name _____ Address _____

City _____ Telephone _____

Contractors License No. _____

5. I will provide some of the work but I have contracted (hired) the following to provide the Work indicated:

| Name | Address | Phone | Type of work |
|------|---------|-------|--------------|
|------|---------|-------|--------------|

| | | | |
|--|--|--|--|
| | | | |
|--|--|--|--|

X Signed [Signature]

Job Address 640 34th ST X DATE 10/20/00

Permit No: 0011600

Certification of Compliance
School District Development

Part I - To be completed by the APPLICANT

Owner's Name/Address GREG DAWSON / JENNIFER MATTINGLY
Project Address 640 34TH ST.
Parcel Number 004-0271-006 Lot No. _____
Subdivision Name _____ No. of Units _____
Applicant's Signature _____ Title _____
Phone No. (916) 930-0201 Date 10/9/00

Notice to Applicant: Pursuant to Government Code Section 66020(d), this will serve to notify you that the 90-day approval period in which you may protest the fees or other payment identified above will begin to run on the date in which the building or installation permit for this project is issued or on which they are paid to the district(s) or to another public entity authorized to collect them on behalf of the district(s), whichever is earlier.

Part II - To be completed by the BUILDING DEPARTMENT

Plan Identification Number 0011600 R
Building Type (check one) Residential Apartment/Condominium Commercial/Industrial
Square Feet of Chargeable Building Area 978 #
Signature/Title Will S. Jones Date 10-5-00

Part III - To be completed by the SCHOOL DISTRICT

School District GLUSD Certificate No. 6892
 Exempt Comments _____
Residential/Apartment/etc. 978 Square ft. x \$ 1.72 = \$ 1682.16
Commercial/Industrial _____ Square ft. x \$ _____ = \$ _____
Total fees collected CK # 1050 = \$ 1682.16

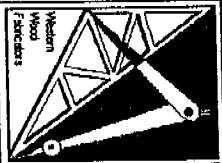
10-06-00P01:52 RCVD

This certification covers only the amount of square footage indicated above. Any additions or corrections to the square footage for this project will require an amendment to the Certificate of Compliance.

As the authorized school official, I hereby certify that the requirements of Government Code Section 65995 and any other authorized requirements have been complied with by the above signed applicant.

Signature [Signature] Date 10/16/00

White & Canary - School District • Pink - Building Department • Goldenrod - Applicant



WESTERN WOOD

FABRICATORS

3700 Riego Rd., Elverta, CA 95626

PH (916) 991-4400

FAX (916) 991-4461

TRUSS ENGINEERING DRAWING PACKAGE

Customer: HERLAND CONSTRUCTION Project: 640 34 STREET Plan: _____ Elev: _____

Sale Personnel: BILL V. Designer: DUN N. Submittal No. _____ Date: 12/18/00 Job No. 00-605

NOTES:

- ALL LATERAL BRACING SPECIFIED IS FOR BRACING COMPRESSION WEB MEMBERS, AND MUST BE INSTALLED. TOP CHORDS ARE ASSUMED TO BE Laterally RESTRAINED BY PLYWOOD, OR SPACED SHEATHING. WHERE NO RIGID CEILING IS APPLIED DIRECTLY TO THE BOTTOM CHORDS, IT SHALL BE BRACED AT INTERVAL OF NOT EXCEEDING 10'-0".
- VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.
- ALL FLOOR TRUSSES RECOMMEND 2X6 STRONGBACKS AT 10'-0" O.C.
- DUE TO THE VARIATIONS IN WEATHER, LUMBER DIMENSIONS AND MOISTURE CONTENT, AT THE TIME OF FABRICATION, WESTERN WOOD FABRICATORS CANNOT BE RESPONSIBLE FOR ANY TRUSS DIMENSION VARIANCE OF + - 1/4 " OR LESS

DO NOT CUT OR ALTER TRUSSES



WESTERN WOOD
CERTIFIED INSPECTION
IN STRICT ACCORDANCE
WITH U.B.C. 2311.6
PREFABRICATED

Job Name: 640 34 STREET

Truss ID: B1DG

Drwg: B1DG

| REQ'D | REACT | SIZE | REQ'D |
|-------|--------|-------------|-------|
| 1 | 0-1-12 | 1276 3.50" | 1.50" |
| 2 | 18-1-4 | 1281149.50" | 1.50" |

plating spec : ANSI/AISC - 1995
THIS DESIGN IS THE COMPOSITE RESULT OF
MULTIPLE LOAD CASES.
SEPARATE REQUIREMENTS shown are based ONLY
on the truss material at each bearing.
Loaded for 10 PSF non-occupant BIL.
Gable floors are 2x 4 @ 16.0 in. o.c. MAX.
<It is assumed that one face of this truss
< is sheathed with plywood, OSB, wood board
< siding or horizontal slating. If not,
< additional loads must be considered on
< non-continuous bearing gables.
May use adequate straps for gable blocks.
Gable blocks may require lateral bracing
due to wind applied to the face. See
Trussals suggested gable bracing detail(s).
Lateral loads in line with the chords have
not been considered unless noted otherwise.
These loads and their connections are the
responsibility of the building designer.

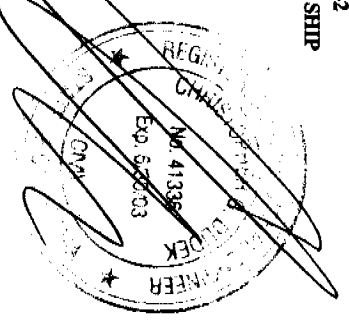
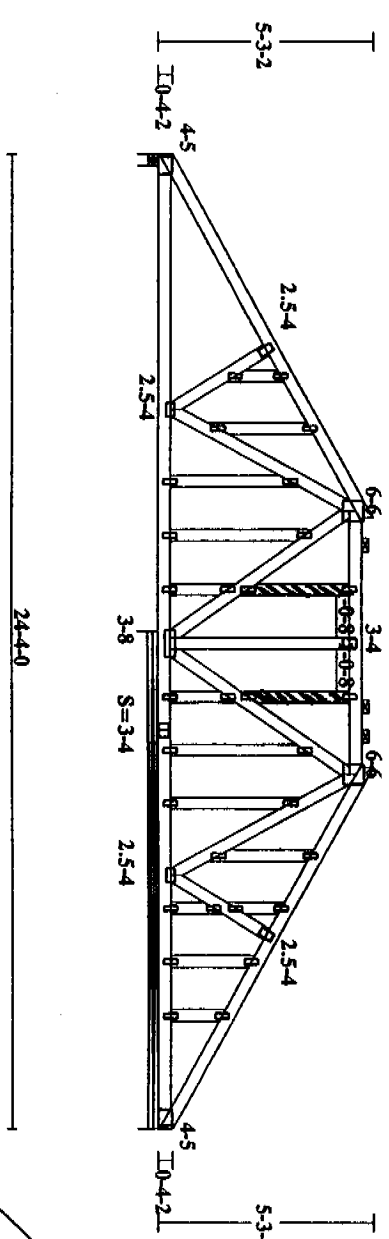
This design based on chord bracing applied
per the following schedule:
max o.c. from to
TC 48.0' 9-0-15 15-2-0
UP/LIFT REACTION(S) :
support 1 -258#
support 2 -253#

MAX DEFLECTION (span) :
L/999 IN MEM 11-12 (LIVE)
L = .04" D = .06" T_e = .10"

| MEM | FORCE | CST | MEM | FORCE | CST |
|------|-------|-----|------|-------|-----|
| 2-9 | -683 | .18 | 5-10 | 1078 | .47 |
| 3-9 | 785 | .30 | 5-12 | 794 | .30 |
| 3-10 | 1070 | .47 | 6-12 | -687 | .19 |
| 4-10 | -228 | .09 | | | |

THE TWO VERTICALS THAT
ARE SHADED ARE NON STRUCTURAL
AND MAY BE ALTERED OR ELIMINATED

| Joint Locations | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
|-----------------|----|----|---|---|---|---|---|
| 0-0-0 | 0 | 0 | 8 | 0 | 0 | 0 | 0 |
| 4-10-2 | 9 | 6 | 4 | 2 | 0 | 0 | 0 |
| 8-10-2 | 10 | 12 | 4 | 2 | 0 | 0 | 0 |
| 12-10-2 | 11 | 14 | 4 | 4 | 0 | 0 | 0 |
| 16-10-2 | 12 | 17 | 4 | 4 | 0 | 0 | 0 |
| 20-10-2 | 13 | 24 | 4 | 4 | 0 | 0 | 0 |



All plates are 20 gauge Trusswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.
TYPICAL PLATE : 15-4

Truss ID: B1DG

Western Wood Fabricators
3700 Riego Road, Elverta, CA 95626

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: TRUSSCOM MANUAL, by Trusswal, QUALITY CONTROL STANDARD FORMERLY PLATE CONNECTED WOOD TRUSSES - (HIB-91) and HIB-91 SUMMARY - (HIB-89), HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES - (HIB-91) and HIB-91 SUMMARY SHEET by TPI. The Truss Plate Institute (TPI) is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, NW, Ste 200, Washington, DC 20036.

| | |
|------------------|-----------------------|
| Eng. Job: EJ. | Job#: J0605 |
| Chk: CM | Date: 12/14/2000 |
| Dsgnr: DN | DurFacs L=1.25 P=1.25 |
| TC Live 16.0 psf | Rep Mbr Bnd 1.15 |
| TC Dead 14.0 psf | O.C.Spacing 2-0-0 |
| BC Live .0 psf | Design Spec UBC |
| BC Dead 7.0 psf | |
| TOTAL 37.0 psf | Segn T6.1.2 0 |

Job Name: 640 34 STREET

Truss ID: CIDG

Drwg: CIDG

| REQ | X-LOC | REACT | SIZE | REQ'D |
|-----|--------|-------|-------|-------|
| 1 | 0-1-12 | 1087 | 3.50" | 1.50" |
| 2 | 20-3-4 | 1235 | 3.50" | 1.50" |

TC 2x4 DEL #1
2x4 DEL 2100P-1, BE 3-7
BC 2x4 DEL #1
WB3 2x4 DEL S1000P
CEL BLK 2x4 DEL S1000P
Lumber shear allowances are per NDS-97.
Drainage must be provided to avoid panning.
1/8" min. continuous lateral bracing attached to flat TC where indicated w/ 2-10d nails each.
Lumber must be structural grade.
BRACE @ 24" O.C. UNLESS NOTED OTHERWISE.
DESIGNED FOR 250 PLS DRY LOAD APPLIED EVENLY ALONG THE TOP CHORD TO THE BOTTOM CHORD CONTINUOUSLY, CONCENTRICALLY WITH DEAD LOADS ONLY. Duration=1.33 Continuous bearing reaction = 0 PLS. (**)
Connection (by others) must transfer equal load to each ply (or add-on) span.

Plating spec: ANSI/APA - 1995
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOW CROSSES.
BEARING REQUIREMENTS shown are based ONLY on the truss material at each bearing.
PLATE VALUES PER ICCB RESEARCH REPORT #1607.
Ledged for 10 HSP non-connection BCL.
Cable blocks are 2x 4 @ 16.0 in. o.c. MAX.
<It is assumed that one face of this truss <is sheathed with plywood, OSB, wood board <slating or hardwood siding. If not, <additional loads must be considered on <non-continuous bearing gables.
May use adequate stragles for gable blocks.
Gable blocks may require lateral bracing due to wind applied to the face. See Trussals suggested gable bracing detail (S). Lateral loads in line with the chords have not been considered unless noted otherwise. These loads and their connections are the responsibility of the building designer.

This design based on chord bracing applied per the following schedule:
TC max o.c. from 7-9-7 to 12-6-8
TC DELTY REACTION(S): support 1 -223# support 2 -81#

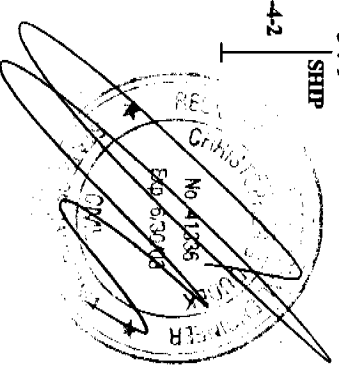
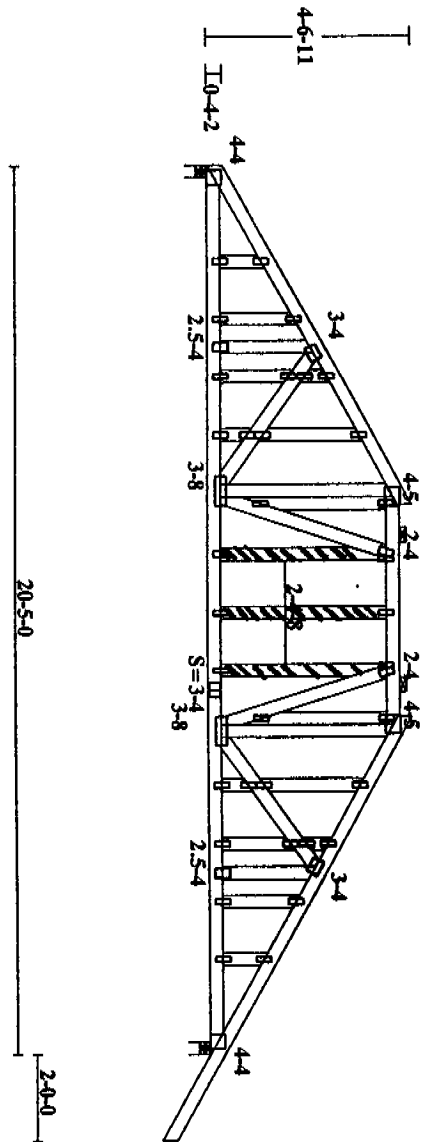
MAX DEFLECTION (span) :
1/999 IN WEM 15-16 (LIVE)
L_e = .08" D_e = .11" F_e = .19"

| MEM | FORCE | CSI | MEM | FORCE | CSI |
|------|-------|-----|------|-------|-----|
| 2-11 | 192 | .07 | 6-15 | -1245 | .42 |
| 3-12 | -812 | .22 | 7-15 | 1069 | .41 |
| 3-12 | 1069 | .41 | 8-15 | -812 | .22 |
| 4-12 | -1245 | .42 | 8-16 | 192 | .07 |
| 5-13 | 67 | .03 | | | |

BC FORCE 1560
10-11 1060
11-12 926
12-13 926
13-14 926
14-15 1060
15-16 1660
16-17 1660

AML END CSI
23 22 45
10 40 50
12 40 52
12 40 52
12 40 52
12 40 52
23 22 45

Joint Locations
1 0-0-0 10 0-0-0
2 4-2-6 11 4-2-6
3 7-7-4 12 7-7-4
4 8-10-0 13 10-2-8
5 16-2-8 14 12-2-2
6 11-7-0 15 12-9-12
7 12-9-12 16 2-10-8
8 16-2-10 17 20-5-0
9 20-5-0



All plates are 20 gauge Trussal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.
TYPICAL PLATE : 1.3-4

Truss ID: CIDG

3700 Riego Road, Elverta, CA 95626

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: TRUSSING MANUAL, by Trussal, QUALITY CONTROL STANDARD FORMERIAL PLATE CONNECTED WOOD TRUSSES - IHB-91 and IHB-91 SUMMARY (GSR-89), HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES - IHB-91 and IHB-91 SUMMARY SHEET by TPI, The Truss Plate Institute (TPI) is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, NW, Ste 200, Washington, DC 20036.

| Eng. Job: .BJ. | Job#: J0605 |
|------------------|-----------------------|
| Chk: CK | Date: 12/14/2000 |
| Dsgnr: DN | DurFacs L=1.25 P=1.25 |
| TC Live 16.0 psf | Reg Mbr Band 1.15 |
| TC Dead 14.0 psf | O.C.Spacing 2-0-0 |
| BC Live .0 psf | Design Spec UBC |
| BC Dead 7.0 psf | Segm T6.1.2 - |
| TOTAL 37.0 psf | 0 |

Job Name: 640 34 STREET

Truss ID: CIDG2

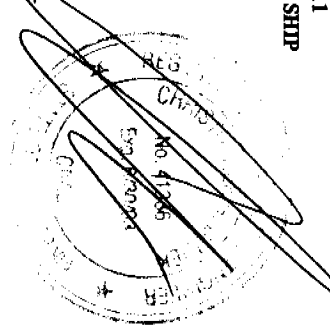
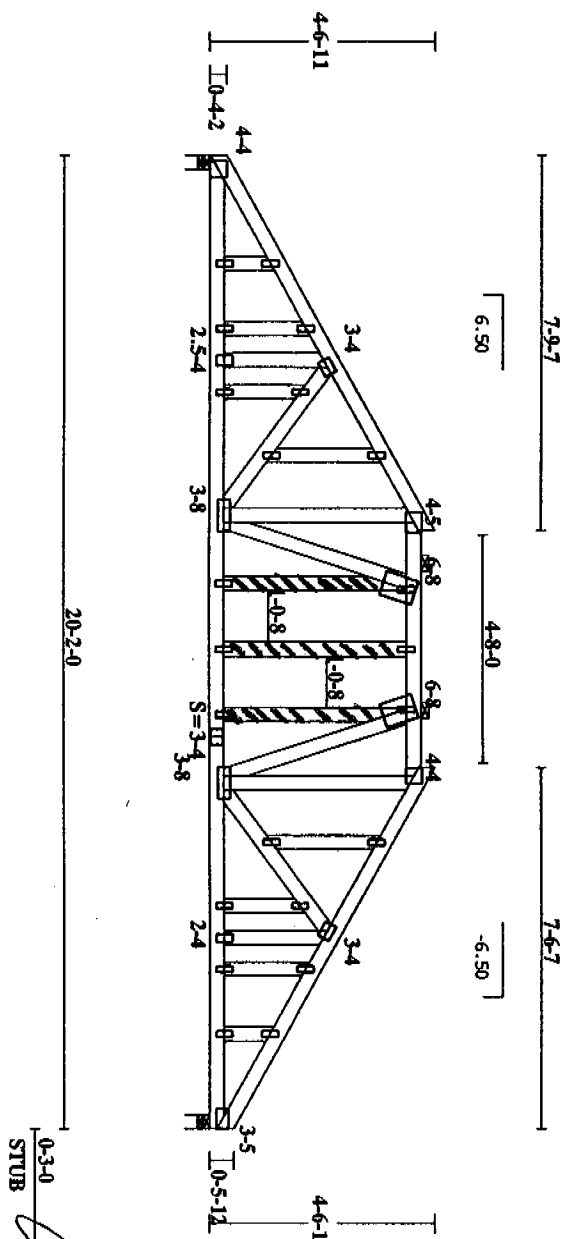
Drwg: CIDG2

| MEM | FORCE | CSI | MEM | FORCE | CSI | MEM | FORCE | CSI |
|-------|--------|------|-------|-------|-----|-----|-------|-----|
| 1 | 0-1-12 | 1089 | 3.50" | 1.50" | | | | |
| 2 | 20-0-4 | 1089 | 3.50" | 1.50" | | | | |
| TC | FORCE | AXL | BND | CSI | TC | DEL | #1 | |
| 1-2 | -1330 | .03 | .10 | .12 | 2x4 | DEL | #1 | |
| 2-3 | -859 | .01 | .98 | .99 | 2x4 | DEL | #1 | |
| 3-4 | -903 | .01 | .98 | .99 | 2x4 | DEL | #1 | |
| 4-5 | -850 | .01 | .99 | 1.00 | 2x4 | DEL | #1 | |
| 5-6 | -850 | .01 | .99 | 1.00 | 2x4 | DEL | #1 | |
| 6-7 | -1280 | .02 | .12 | .14 | 2x4 | DEL | #1 | |
| 7-8 | -1776 | .03 | .39 | .42 | 2x4 | DEL | #1 | |
| 8-9 | | | | | 2x4 | DEL | #1 | |
| BC | FORCE | AXL | BND | CSI | BC | DEL | #1 | |
| 10-11 | 1664 | .24 | .22 | .45 | 2x4 | DEL | #1 | |
| 11-12 | 1045 | .10 | .40 | .50 | 2x4 | DEL | #1 | |
| 12-13 | 903 | .12 | .40 | .52 | 2x4 | DEL | #1 | |
| 13-14 | 903 | .12 | .25 | .36 | 2x4 | DEL | #1 | |
| 14-15 | 903 | .12 | .42 | .54 | 2x4 | DEL | #1 | |
| 15-16 | 980 | .08 | .42 | .51 | 2x4 | DEL | #1 | |
| 16-17 | 1508 | .21 | .45 | .67 | 2x4 | DEL | #1 | |
| MEB | FORCE | CSI | MEM | FORCE | CSI | MEM | FORCE | CSI |
| 2-11 | 194 | .07 | 6-15 | -1256 | .42 | | | |
| 2-12 | -814 | .22 | 7-15 | 1040 | .40 | | | |
| 3-12 | 1072 | .41 | 8-15 | -840 | .25 | | | |
| 4-12 | -1249 | .42 | 8-16 | 217 | .08 | | | |
| 5-13 | 65 | .03 | | | | | | |

THE THREE VERTICALS THAT ARE SHADED ARE NON STRUCTURAL AND CAN BE ALTERED OR ELIMINATED

MAX DEFLECTION (SPEC):
L/999 IN MEM 11-12 (LIVE)
L = .08" D = .11" P = -.19"

Joint Locations:
1 0-0-0 10 0-0-0
2 4-2-6 11 4-2-6
3 7-7-4 12 7-7-4
4 8-10-0 13 10-2-8
5 10-2-8 14 12-0-0
6 11-7-0 15 12-9-12
7 12-9-12 16 16-2-10
8 16-2-10 17 20-2-0
9 20-2-0



All plates are 20 gauge Trusswall Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.
TYPICAL PLATE: 15-4

TRUSS ID: CIDG2

Western Wood Fabricators
3700 Riego Road, Elverta, CA 95626

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| | |
|----------------|-----------------------|
| Eng. Job: .57. | Job#: J0605 |
| CDK: CM | Date: 12/14/2000 |
| Design: DN | DurFacs L=1.25 P=1.25 |
| TC Live | 16.0 psf |
| TC Dead | 14.0 psf |
| BC Live | .0 psf |
| BC Dead | 7.0 psf |
| Design Spec | UBC |
| TOTAL | 37.0 psf |
| | Segn T6.1.2 - 0 |

| REQ | X-LOC | REQT | SIZE | REQ'D |
|-----|--------|------|--------|-------|
| 1 | 0-1-12 | 1094 | 3.50" | 1.50" |
| 2 | 16-5-4 | 1099 | 97.50" | 1.50" |

TC 2x4 DEL #1
 BC 2x4 DEL #1
 WEB 2x4 DEL #1
 GAB. BLK 2x4 DEL #1
 STRAND 2x4 DEL #1
 PLATE VALUES PER IBCO RESEARCH REPORT #1607.
 Drainage must be provided to avoid purling.
 Deck continuous lateral bracing attached to Flat TC where indicated w/ 2-10d nails each. Lumber must be structural grade.
 Brace @ 24" o.c. unless noted otherwise.
 PLATING BASED ON GREEN LUMBER VALUES.
 Designed for 250 P.F. dead load applied evenly along the top chord to the bottom chord continuously, concurrently with dead loads only. Durations: 1.33 Continuous
 Bearing reaction = 0 P.F. (+) (+)
 Connection (by others) must transfer equal load to each ply (or add-on) strand.

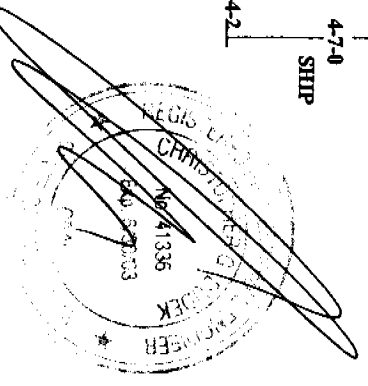
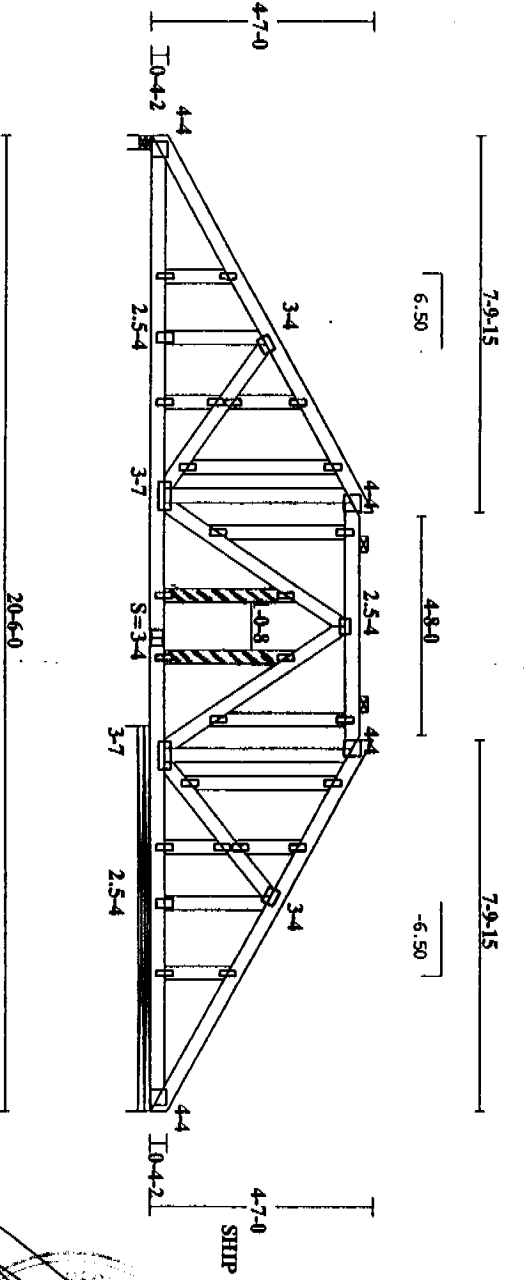
Plating spec: ANSI/APA - 1995
 THIS DESIGN IS THE CONCRETE RESULT OF
 MULTIPLE LOAD CASES.
 BEARING REQUIREMENTS shown are based ONLY
 on the truss material at each bearing.
 Loaded for 10 HR non-concurrent BTL.
 Gable blocks are 2x 4 @ 16.0 in. o.c. MAX.
 <It is assumed that one face of this truss
 < is sheathed with plywood, OSB, wood board
 < siding or hardboard siding. If not,
 < additional loads must be considered on
 < non-continuous bearing gables.
 Gable blocks may require lateral bracing
 due to wind applied to the face. See
 Trusswals suggested gable bracing detail(s).
 Lateral loads in line with the chords have
 not been considered unless noted otherwise.
 These loads and their connections are the
 responsibility of the building designer.

This design based on chord bracing applied
 per the following schedule:
 max o.c. From To
 TC 48.0" 7-9-15 12-7-0
 UPLIFT REACTION(S) :
 support 1 -237#
 support 2 -232#

MAX DEFLECTION (span) :
 L/999 IN W/ 10-11 (L/175)
 Lf = .03" Df = .04" Tf = -.07"

| Joint Locations | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 |
|-----------------|---|----|----|----|----|----|----|---|----|----|----|----|----|----|----|----|----|----|----|----|
| 0-0-0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0-2-10 | 4 | 2 | 10 | 9 | 4 | 2 | 10 | 4 | 2 | 10 | 4 | 2 | 10 | 4 | 2 | 10 | 4 | 2 | 10 | 4 |
| 7-7-12 | 7 | 7 | 12 | 10 | 7 | 7 | 12 | 7 | 7 | 12 | 10 | 7 | 7 | 12 | 10 | 7 | 7 | 12 | 10 | 7 |
| 10-3-0 | 4 | 10 | 3 | 0 | 11 | 10 | 6 | 4 | 4 | 10 | 3 | 0 | 11 | 10 | 6 | 4 | 4 | 10 | 3 | 0 |
| 12-10-4 | 5 | 12 | 10 | 4 | 12 | 12 | 10 | 4 | 12 | 12 | 10 | 4 | 12 | 12 | 10 | 4 | 12 | 12 | 10 | 4 |
| 16-1-1 | 6 | 16 | 1 | 1 | 13 | 16 | 1 | 1 | 13 | 16 | 1 | 1 | 13 | 16 | 1 | 1 | 13 | 16 | 1 | 1 |
| 20-6-0 | 7 | 20 | 6 | 0 | 14 | 20 | 6 | 0 | 14 | 20 | 6 | 0 | 14 | 20 | 6 | 0 | 14 | 20 | 6 | 0 |

*THE TWO VERTICALS THAT ARE
 SHADED ARE NON STRUCTURAL
 AND CAN BE ALTERED OR ELIMINATED*



All plates are 20 gauge Trusswals unless preceded by "18" for 18 gauge or "H" for 16 gauge.
 TYPICAL PLATE : I5-4

Truss ID: D2DG

Western Wood Fabricators
 3700 Riego Road, Elverta, CA 95626

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: TRUSSOM MANUAL, by Trusswals, QUALITY CONTROL STANDARD CONNECTED WOOD TRUSSELS (QST-88), HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSELS (HIB-91) and HIB-91 SUMMARY SHEET by TPI. The Truss Plate Institute (TPI) is located at 583 D'Oro Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave., NW, Ste 200, Washington, DC 20036.

| | |
|------------------|-----------------------|
| Eng. Job: .E2. | Job#: 30605 |
| Chk: CM | Date: 12/14/2000 |
| Design: DN | |
| TC Live 16.0 psf | DurFacs I=1.25 P=1.25 |
| TC Dead 14.0 psf | Rep Mbr Bnd 1.15 |
| BC Live .0 psf | O.C. Spacing 2-0-0 |
| BC Dead 7.0 psf | Design Spec UBC |
| TOTAL 37.0 psf | Segm T6.1.2 - 0 |

Job Name: 640 34 STREET

TRUSS ID: E2DG

DRWG: E2DG

| BRG | X-LOC | REACT | SIZE | REQ'D |
|-----|--------|-------|-------|-------|
| 1 | 0-1-12 | 671 | 3.50" | 1.50" |
| 2 | 12-7-0 | 671 | 3.50" | 1.50" |

TC 2x4 DEL #1 2100F-1.9E
 BE 2x4 DEL STANDARD
 WCB 2x4 DEL STANDARD
 GBL BRK 2x4 DEL STANDARD
 PLATE VALUES PER ICD RESEARCH REPORT #1607.
 Gable blocks are 2x 4 @ 16.0 in. o.c. MAX.
 <It is assumed that one face of this truss
 <is sheathed with plywood. OSB, wood board
 <sliding or hardwood siding. If not,
 <additional loads must be considered on
 <non-continuous bearing gables.
 <Key use adequate staples for gable blocks.
 <Gable blocks may require lateral bracing
 <due to wind applied to the face. See
 <Truss's suggested gable bracing detail (s).
 <Lateral loads in line with the chords have
 <not been considered unless noted otherwise.
 <These loads and their connections are the
 <responsibility of the building designer.

Plating spec: ANSI/AITC - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 BEARING REQUIREMENTS shown are based ONLY
 on the truss material at each bearing.
 Drainage must be provided to avoid ponding.
 1/4" min. continuous lateral bracing attached
 to flat TC where indicated w/ 2-10d nails
 each. Lumber must be structural grade.
 BRACE @ 24" o.c. unless noted otherwise.
 FINISH: PRESSED ON GREEN LUMBER VALUES.

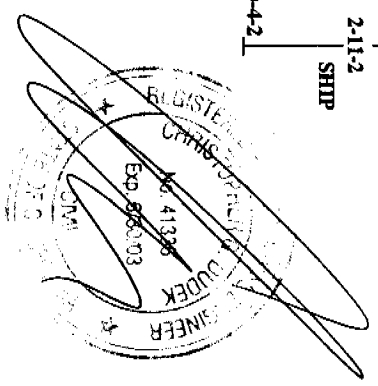
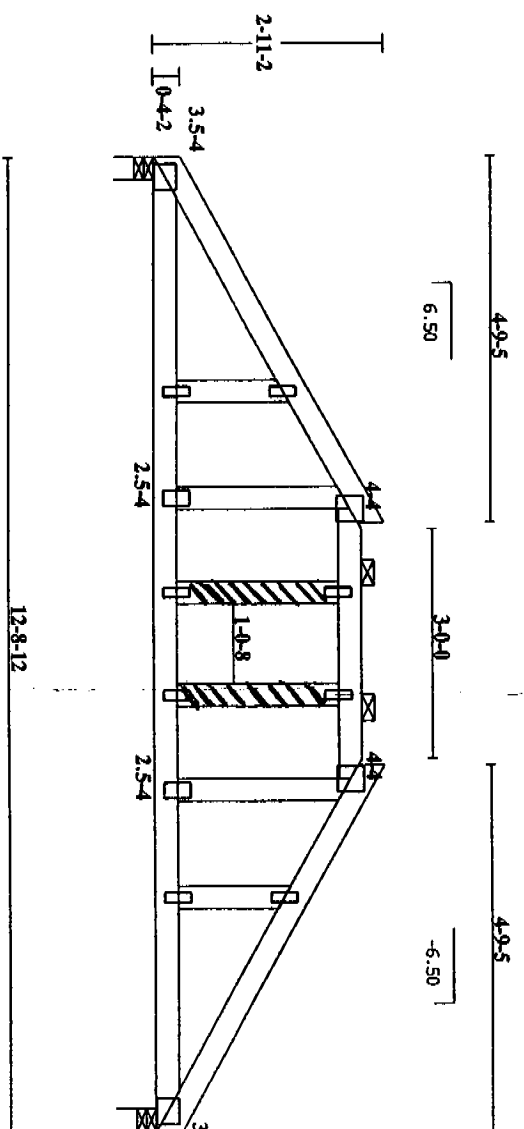
 Designed for 250 lbf dead load applied
 evenly along the top chord to the bottom
 chord continuously, concurrently with dead
 loads only. Duration=1.33 (Continuous
 bearing reaction = 0 lbf (**)
 Connection (by others) must transfer equal
 load to each ply (or add-on) ston.

This design based on chord bracing applied
 per the following schedule:
 TC max o.c. from TO
 support 1 48.0" 4-9-5 7-10-6
 support 2 -136#
 UPLIFT REACTION(S):
 -136#

*THE TWO VERTICALS SHADED BELOW
 ARE NON STRUCTURAL AND
 CAN BE OMITTED OR ELIMINATED*

MAX DEPLETION (Spec):
 L/567 IN (MIN) 7-8 (LIVE)
 L/1500 IN (MAX) 1-20 (DEAD)

| Joint Locations | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |
|-----------------|---|---|---|---|----|------|---|---------|
| 0-0-0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 4-7-2 | 2 | 4 | 7 | 8 | 12 | 8-12 | 8 | 12-8-12 |



All plates are 20 gauge Trussal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.
 TYPICAL PLATE : 1.5-4

Truss ID: E2DG

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor support of components members only to reduce building length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the moisture standards. TRUSSCOM MANUAL, by Trussal, QUALITY CONTROL STANDARD FORMETAL PLATE CONNECTED WOOD TRUSSES - (H1B-91) and H1B-91 SUMMARY (G8T-88), HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES - (H1B-91) and H1B-91 SUMMARY SHEET by TPI. The Truss Plate Institute (TPI) is located at 583 P'Onorio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave. NW, Ste 200, Washington, DC 20006.

| | |
|------------------|-----------------------|
| Eng. Job: .EJ. | Job#: J0605 |
| Chk: CM | Date: 12/14/2000 |
| Design: DW | DurPacs L=1.25 P=1.25 |
| TC Live 16.0 psf | Rep Mbr Bnd 1.15 |
| TC Dead 14.0 psf | O.C.Spacing 2-0-0 |
| BC Live .0 psf | Design Spec UBC |
| BC Dead 7.0 psf | |
| TOTAL 37.0 psf | Segn T6.1.2 0 |

Western Wood
Fabricators
 3700 Riego Road, Elverta, CA 95626



CAPITOL ENGINEERING LABORATORIES, INC.

631 Commerce Drive, Suite #200 • Roseville, California 95678 • (916) 786-2488

JOB REPORT

PROJECT NAME: 640 34th STREET PAGE: _____
 INSPECTOR: Kenneth L. Ph... FILE NO. 5349
 PERSONS CONTACTED: JIM DATE: 11-7-00
 REFERENCE DOCUMENTS: DWG PERMIT #: _____
 WEATHER: CLEAR

SERVICE PROVIDED: CONCRETE (INSP/SAMPLE ONLY/PU) MASONRY WELDING (SHOP/FIELD) SOILS
 OTHER TORQUE TEST BOLTS

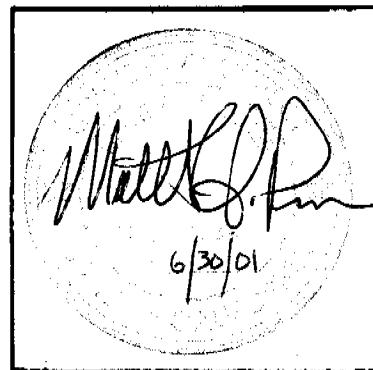
CHECKED TORQUE ON 1/2" & 5/8" STEEL BOLTS, CHECKED
 + 50% AND ALL 1/2" AT 35lbs OR ABOVE AND 5/8" AT 80lbs

COMPLIANCE OF WORK: _____
 ATTACHMENTS: _____
 EQUIPMENT/SUPPLIES USED: _____
 NEXT VISIT: _____

REMARKS: _____
 REVIEWED BY: Kenneth L. Ph... DATE: _____

ADDENDUM TO CONSTRUCTION DOCUMENTS:

DAWSON/MATTINGLY RESIDENCE
640 34TH ST.
SACRAMENTO, CA.
PERMIT # 0011600R



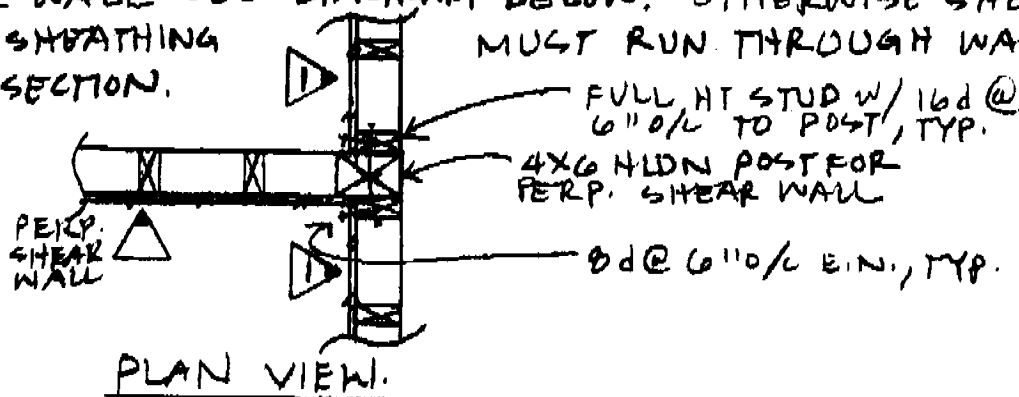
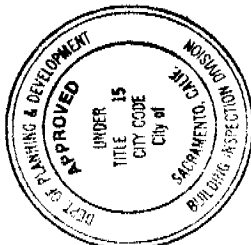
ITEM #1

CLARIFICATION TO FIELD INSPECTOR -
WHERE ROOF TRUSSES ARE CONNECTED
TO GIRDER ROOF TRUSS WITH METAL
HANGER AT BOTTOM CHORD, BLOCKING
IS NOT REQUIRED BETWEEN HANGERS

ITEM #2

ALTERNATE TO EXTERIOR APPLICATION OF SHEAR WALL
FRAMING TYPE Δ (6d @ 6" O/C E.N. & 12" O/C F.N.)
SHEAR WALL SHEATHING MAY BE APPLIED AT INTERIOR
FACE OF SHEAR WALL LOCATIONS. ADD SIMPSON ASS
FRAMING ANCHORS @ 32" O/C FROM 2ND FLR FRAMING RIM
DIST TO TOP PLATE. WHERE INTERIOR SHEAR WALL
SHEATHING MEETS POST AT END OF PERPENDICULAR
SHEAR WALL SEE DIAGRAM BELOW. OTHERWISE SHEAR
WALL SHEATHING MUST RUN THROUGH WALL
INTERSECTION.

This set of plans and specifications must be kept on the job at all times and it is unlawful to make any changes or alterations from the same without written permission from the Building Inspection Division. The approval of this plan and specification SHALL NOT be held to permit or approve a violation of any City Ordinance.



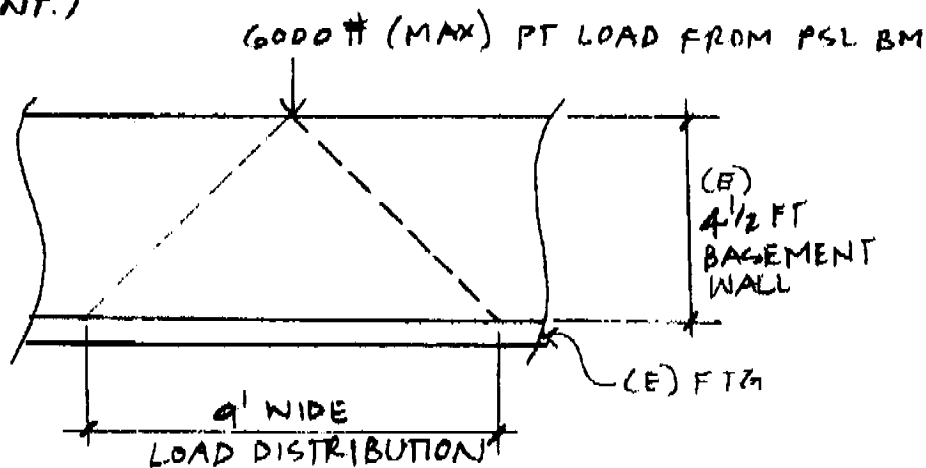
ITEM #3

BEARING FOR ANGLED PSL BEAM OVER BSMT MECH. ROOM
BEAM MAY BE SUPPORTED OVER INTERIOR WALL
LEDGE WITHIN 8" OF INTERIOR FACE. END REACTION
OF BEAM < 6000# & LOADS ARE DISTRIBUTED TO FTG
OF WALL 4 1/2 FT HIGH & 8" WIDE W/ NO OTHER VERT. LOADS.

MGP 2/6/01

(NEXT PAGE)

ITEM #2 (CONT.)

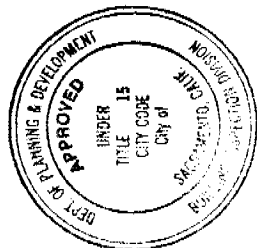


CHECK LOADING - ASSUME WORST CASE FTG = 8" WIDE
 ALLOW (1000 ASBP) = (67)(9)(1000) = 6000#

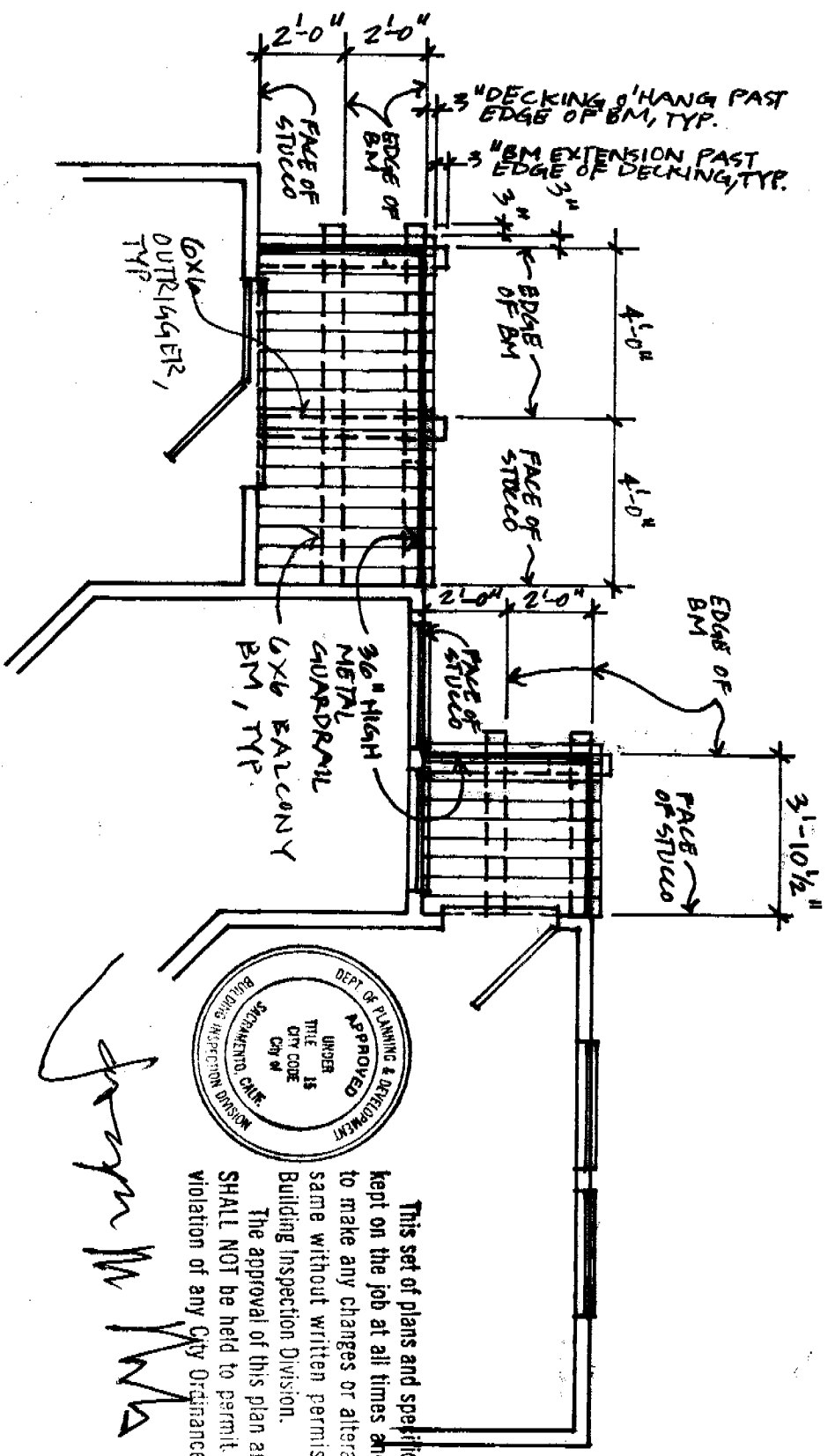
OK

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The approval of this plan and specification SHALL NOT be held to permit or approve the violation of any City Ordinance or State Law.



REVISION TO ACTIVE PERMIT # 0011600 R.
 640 34TH ST.



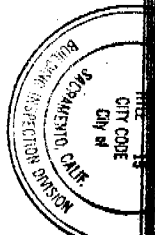
(N) PROPOSED 2ND FLOOR BALCONY PLAN

SC. 1/4" = 1'-0"



REVISED REAR ELEVATION

- DELETE POST & BEAM SUPPORTED 2ND FLOOR DECK FROM PLANS
- SUBSTITUTE (N) 2ND STORY BALCONIES AS SHOWN



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The approval of this plan and specification SHALL NOT be held to permit or approve the violation of any City Ordinance or State Law.

3/3