CITY OF SACRAMENTO

1231 I Street, Sacramento, CA 95814

Permit No: 0004215

Insp Area: 3

Site Address: 5031 60TH ST SAC
Parcel No: 023-0122-006

Sub-Type: ASFR
Housing (Y/N): N

CONTRACTOR

OWNER
SERGEY GORBENKO
5031 60TH ST
SAC CA. 95820

ARCHITECT

Nature of Work: ADD GGARAGE/PORCH/LIVING AREA

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender'sAddress _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class _____ License Number _____ Date _____ Contractor Signature _____

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars ($500.00):

____ I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

____ I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

____ I am exempt under Sec. _____ B & PC for this reason: _____

Date 6-15-2000 Owner Signature _____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and herby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 6-15-2000 Applicant/Agent Signature _____

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:
____ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, forthe performance of work for which the permit is issued.

____ I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier Policy Number Exp Date

____ (This section need not be completed if the permit is for $100 or less) I certify that in the performance of the work for which this permit is issued,I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 6-15-2000 Applicant Signature _____

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS ($100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

Date of Request: _____
By: _____

CITY OF SACRAMENTO DEVELOPMENT SERVICES DIVISION
PLANNING AND ZONING INFORMATION REQUEST

Project Address: 5031 60th Street Sacramento, CA, 95820

Assessor's Parcel Number: 023-0122-006

Previous Use: SFR

Description of Request/Proposed Use:

Addition

Is This a Change of Use? _____

Zoning Designation: R-1

Prior Applications for Project Site(P#, Z#, DRPB#): _____

Comments: Need to verify setbacks of two adjacent structures on same side of block

Are There Any Planning Issues?: (circle one) YES (NO)

Per Plans neighbors average 21' setback OK

*	Staff Site Plan Check Required? (Circle one)	YES NO
*	Field Inspection Required? (Circle one)	YES NO
*	Design Review/Preservation Required?: (Circle one)	YES (NO)

Planning Review by/Date: W. Toure 4/19/00 P Smith 4-20-00

A list of items that must be reviewed by Planning is provided on the reverse side of this form.

MICROFILM AFTER FINAL

Gorbenco

5031 60th St.

023 0122 006

0004215R

Add 956 ☐ House

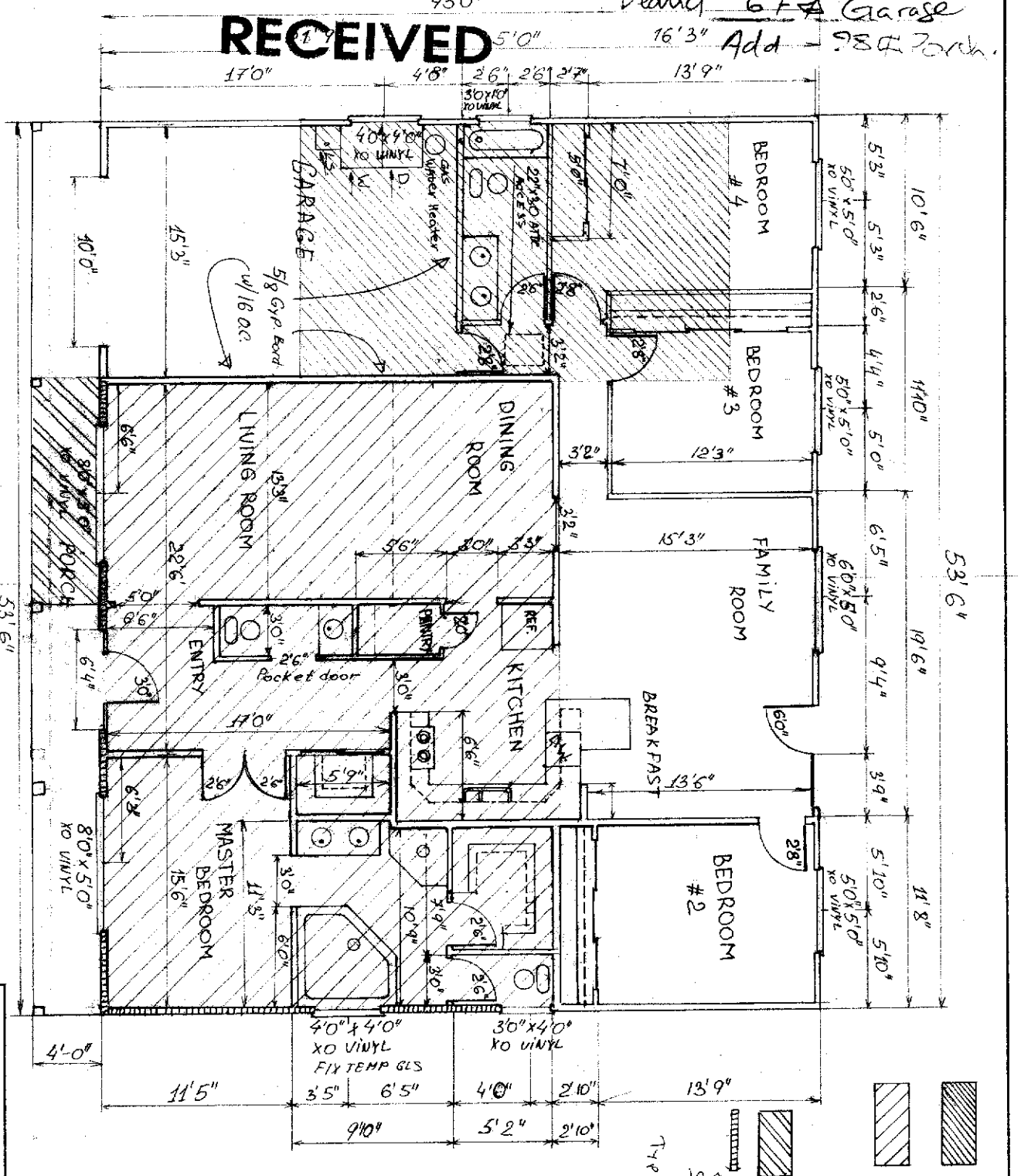
Deduct 67 ☐ Garage

16'3" Add 98 ☐ Porch.

CITY OF SACRAMENTO
PERMIT ASSISTANCE

MAY 2 3 2000

RECEIVED



Existing garage

Existing house

Existing porch

Existing footing for new wall See pg 8

Typical wall framing

CITY OF SACRAMENTO
PERMIT ASSISTANCE
APR 2 0 2000
RECEIVED

5031 60th Street	
SCALE: 1/4"=1'	APPROVED BY
DATE	DRAWN BY
RESIDENCE FOR THE FAMILY GORBENCO	REVISED
Sacramento CA 95820	DRAWING NUMBER 2

Certification of Compliance
School District Development Fees

(Print or Type) If Printing, press hard for four copies

PART I To be completed by the APPLICANT

OWNER'S NAME Sergey Ivan Shorbenko

OWNER'S ADDRESS 5031 60th st.

PROJECT ADDRESS 5031 60th st.

PARCEL NUMBER [illegible] LOT NO. _____

SUBDIVISION NAME _____

NUMBER OF UNITS 1

Upon payment of the fees listed below, a 90-day approval period commences upon which the applicant paying the fees may protest such fees. Any failure to file such protest within the 90-day period shall result in forfeiture of any rights to challenge such fees, through litigation or otherwise.

APPLICANT'S SIGNATURE [signature]

TITLE OF APPLICANT _____

DATE 5/16/00 PHONE NUMBER (10) 455-1804

PART II To be completed by BUILDING DEPARTMENT

PLAN IDENTIFICATION NUMBER [illegible]

BUILDING TYPE

RESIDENTIAL (✓) APARTMENT/CONDOMINIUM ( ) COMMERCIAL/INDUSTRIAL ( )

SQUARE FEET OF CHARGEABLE BUILDING AREA [illegible]

SIGNATURE _____

TITLE _____ DATE _____

PART III To be completed by SCHOOL DISTRICT

SCHOOL DISTRICT WSD

DISTRICT CERTIFICATION NO. 782

EXEMPT _____ COMMENTS _____

RESIDENTIAL/APT/CONDO [illegible] SQ FT X $ [illegible] = $ 1718.28

COMMERCIAL/INDUSTRIAL _____ SQ FT X $ _____ = $ _____

OTHER FEE _____ TYPE _____ SQ FT X $ _____ = $ _____

05-16-00P02:54 PAID

TOTAL FEES COLLECTED = $ 1718.28

This Certification covers only the amount of square footage indicated above. Any additions or corrections to the square footage for this project will require an amendment to the Certificate of Compliance.

As the authorized school district official, I hereby certify that the requirements of Government Code Section 65995 and any other authorized requirements have been complied with by the above signed applicant.

AUTHORIZED SCHOOL DISTRICT OFFICIAL

SIGNATURE [signature]

TITLE [illegible] DATE 5/16/00

Original: School District 1st copy: School District 2nd copy: Building Department 3rd copy: Applicant

Revised 12/12/96

WALKER LUMBER CO., Inc

TRUSSES
ROOFS
& FLOORS

Commercial, Residential, or Custom

Truss Maintenance and Guarantee Instruction

Project: Leonid Molnychuk Date: 4-7-02

Contractor: _____

Trusses are guaranteed to meet specifications on approved drawings and can not be guaranteed unless all items listed are complied with, whether written or implied.

1. Provide proper ventilation in all areas at all times.
2. Restraint of the structures lateral bracing is to be designed, installed and verified by job engineer, contractor & owner.
3. Provide a proper and adequate vapor barrier in all appropriate locations.
4. Do not expose truss to the weather. If any part of the truss is exposed to the weather it must be properly protected and maintained.
5. Owner and contractor must protect trusses from loads above the designed loads shown on the drawings.
6. Do not cut or otherwise alter the truss, its members, or support points.
7. Provide approved walk ways to mechanical units or unit that require service.
8. Owner and contractor to verify loads on the drawings are adequate for the building department load requirements and the requirements of the application.
9. All lateral bracing indicated on the approved truss drawings must be installed prior to loading of any trusses.
10. Trusses must be properly installed right side up at the proper bearing locations with the approved bracing and fasteners.
11. Truss bottom chords should not be restrained or permanently fastened down in any manner to prevent movement different from adjacent trusses other than support points.
12. Do not over load Trusses with any materials (units of sheeting, roof materials, sheet rock etc.)
13. Failure to comply with these requirements, all local building codes, specifications on plans, construction documents and approved truss drawings will void all guarantee's.

Thank You

Mike Walker
Mike Walker Lumber Company, Inc.

PO BOX 958 6915 30th STREET NORTH HIGHLANDS, CA. 95660
916 338-2121 FAX 916 338-5353 mikew@walker-lumber.com

ICBO Evaluation Service, Inc.

5360 WORKMAN MILL ROAD • WHITTIER, CALIFORNIA 90601-2299

A subsidiary corporation of the International Conference of Building Officials

EVALUATION REPORT

ER-4994

Reissued November 1, 1997

Filing Category: FASTENERS—Steel Gusset Plates (066)

ROBBINS LOCK (RL) AND ROBBINS HIGH STRENGTH (RHS) METAL CONNECTOR PLATES FOR WOOD TRUSSES

ROBBINS MANUFACTURING CO.
POST OFFICE BOX 17939
TAMPA, FLORIDA 33682-7939

1.0 SUBJECT

Robbins Lock (RL) and Robbins High Strength (RHS) Metal Connector Plates for Wood Trusses.

2.0 DESCRIPTION

2.1 General:

Robbins Lock (RL) and Robbins High Strength (RHS) plates are metal connector plates for wood trusses. The plates are manufactured from galvanized steel in various lengths and widths, and have integral teeth that are designed to laterally transmit loads between truss wood members. Plans and calculations must be submitted to the building official for the trusses using metal connector plates described in this report.

2.2 Materials:

Robbins Lock (RL) plates are manufactured from No. 20 gage [0.035 inch (0.89 mm)], ASTM A 653, Grade 40, structural-quality steel with a hot-dipped galvanized coating designated G60.

Robbins High Strength (RHS) plates are manufactured from No. 20 gage [0.035 inch (0.89 mm)], ASTM A 653 HSLA, Type I, Grade 60, high-strength, structural-quality steel with a hot-dipped galvanized coating designated G60.

Both the Robbins Lock (RL) and Robbins High Strength (RHS) plates have slots approximately $^1/_2$ inch (12.7 mm) long by $^1/_8$ inch (3.2 mm) wide that have been punched along the longitudinal axis of the plate. Each punched slot forms two opposite-facing, sharply pointed teeth protruding at right angles from the parent metal. The punched slots are spaced approximately $^1/_4$ inch (6.4 mm) on center across the width of the plate and approximately 1 inch (25.4 mm) on center along the length of the plate, with adjacent longitudinal rows staggered $^1/_2$ inch (12.7 mm). Connector plates are available in 1-inch (25.4 mm) increments of width and length. Minimum plate width and length are 1 inch (25.4 mm) and 3 inches (76 mm), respectively. See Figure 1 for details of plate dimensions.

There are eight teeth per square inch (1.24 teeth per square centimeter) of plate surface. The length of each tooth, including the thickness of the parent metal, is approximately $^3/_8$ inch (9.5 mm), and the width of each tooth is approximately $^1/_8$ inch (3.2 mm). The shank of each tooth is concave and the tip of each tooth is twisted approximately 40 degrees with respect to the plate width. Adjacent longitudinal rows of teeth are twisted in opposite directions.

2.3 Allowable Loads:

Tables 1, 2 and 3 provide allowable lateral loads, tension loads and shear loads for the RL and RHS plate connectors. These values are based on the National Design Standard for Metal Plate Connected Wood Truss Construction, ANSI/TPI 1-1995. A copy of the ANSI/TPI 1-1995 standard must be supplied to the building department when this is requested by the building official.

2.3.1 Lateral Resistance: Each metal connector plate must be designed to transfer the required load without exceeding the allowable load per square inch of plate contact area, which is based on species, the orientation of the teeth relative to the load, and the direction of load relative to grain. Design for lateral resistance must be in accordance with Section 11.2.1 of ANSI/TPI 1-1995. Table 1 provides allowable lateral loads for the metal connector plates.

2.3.2 Tension Resistance: Each metal connector plate must be designed for tension capacity that is based on the orientation of the metal connector plate relative to the direction of load. Design for tension must be in accordance with Section 11.2.2 of ANSI/TPI 1-1995. Table 2 provides allowable tension loads for the metal connector plates. Additionally, the net section of the metal connector plates for tension joints must be designed using the allowable tensile stress values of the metal that are adjusted by the metal connector plate tensile effectiveness ratios shown in Table 2.

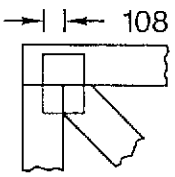
2.3.3 Shear Resistance: Each metal connector plate must be designed for shear capacity that is based on the orientation of the plate relative to all possible lines of shear. Design for shear must be in accordance with Section 11.2.3 of ANSI/TPI 1-1995. Table 3 provides allowable shear loads for the metal connector plates. Additionally, the net section of the metal connector plates for heel joints, and other joints involving shear, must be designed using the allowable shear values for the metal connector plates that are adjusted by the shear resistance effectiveness ratios shown in Table 3.

2.3.4 Metal Plate Reductions: Several allowable-load reduction factors for the metal plates, when they are applicable, must be considered cumulatively in the design of metal connector plates used in fabricated wood trusses:

1. Allowable lateral resistance values for the RL and RHS metal connector plates must be reduced by 15 percent when the plates are installed on the narrow face of truss lumber members.
2. Allowable lateral resistance values must be reduced by 20 percent when the RL and RHS metal connector plates are installed in lumber having a moisture content greater than 19 percent at the time of truss fabrication.

Symbols

PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or in IN-16ths (i.e. 108).

PLATE ORIENTATION

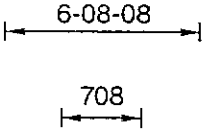
When shown, indicates direction of slots in connector plate.

PLATE SIZE

6.3 x 8.8

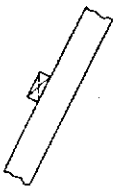
The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.

DIMENSIONS



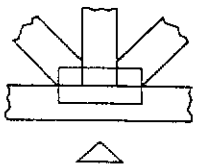
All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).

LATERAL BRACING



Indicates approximate location of continuous lateral bracing required for stability of individual member.

BEARING



Indicates support (bearing) at joint.

NOTICE:

This truss designed at request and specification of customer as an individual building component, in a vertical plane, to be incorporated into building design at the specification of the building designer. Bracing shown is for lateral support of individual members only. Temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. Design and materials are in accordance with latest editions of **NDS, HUD Design Criteria for Trussed Rafters,** and **TPI** specifications. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult **Quality Control Manual, Bracing of Wood Trusses** (HIB-91) and **Recommended Code of Standard Practice**, available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

General Notes:

1. Provide copies of this truss design to the building designer and erection supervisor.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and seat securely. Avoid knots and wane at joint locations.
4. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
5. Unless specifically noted, this design is not applicable for use with fire retardant or preservative treated lumber.
6. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively.
7. Anchorage and/or tie-in components are the responsibility of others, unless shown.
8. Do not stack construction materials on floor or roof that induces loading on truss greater than design loads.
9. Do not cut or alter truss without prior approval of a professional engineer.
10. Building designer is responsible to insure that loading shown hereon is applicable to building site.
11. Care should be exercised in handling, erection, and installation of trusses.
12. Camber is a non-structural consideration and is the responsibility of the truss fabricator. General practice is to camber for dead load deflection.
13. Refer to On-Line Data's Ch. VI-B for joint details.

MIKE WALKER LUMBER
P.O. Box 958
North Highlands, CA 95660

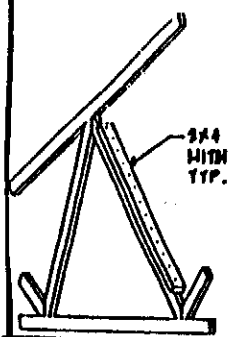
ROBBINS ENGINEERING, INC.
Bringing you the best of both worlds

NATIONAL HEADQUARTERS:
P.O. Box 280055
Tampa, FL 33682-0055
(800)282-1299/ FAX (813)971-6117

REGIONAL OFFICES:
1402 20th Street NW Suite 4
Auburn, WA 98001
(800)532-9404/ FAX (206)735-2238

1177 Rockingham
Richardson, TX 75080
(214)238-9609/ FAX (214)437-0467

THIS DETAIL IS TO BE USED AS ALT. FOR ONE IN 4 CONTINUOUS LATERAL BRACE.

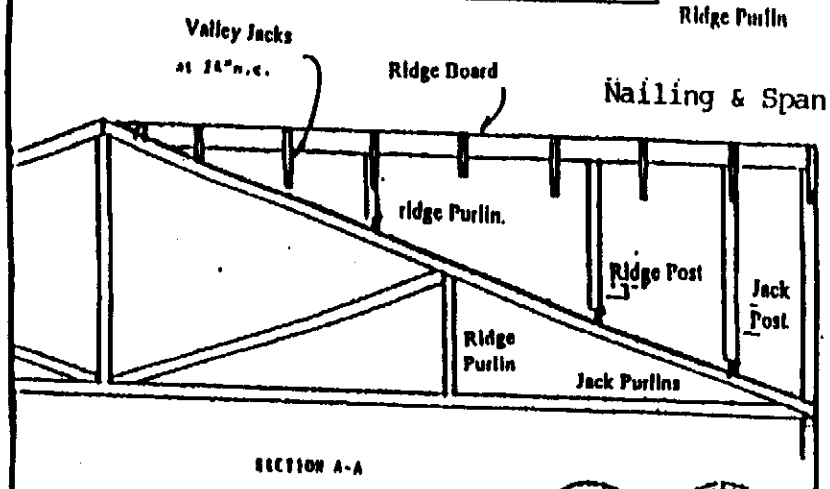


NOTE:
BRACE MUST BE 40 % THE LENGTH OF THE MEMB.

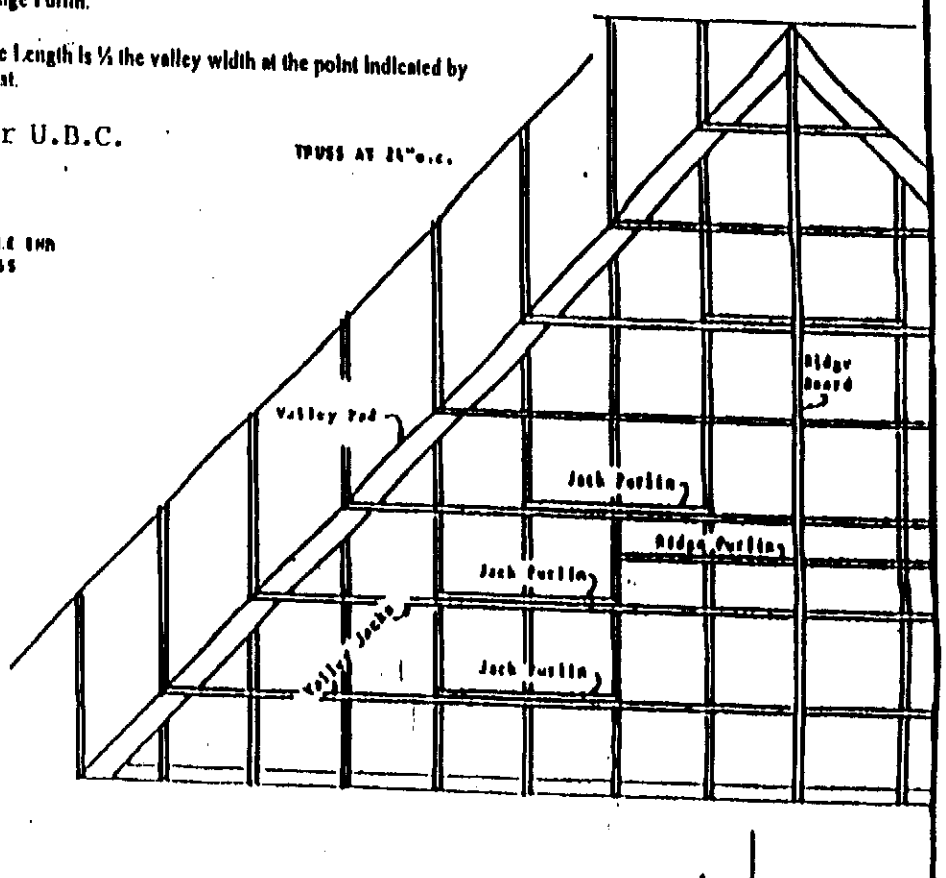
Detail for conventionally framed Valley Fill

Valley Jacks	Spaced 24" o.c. clear span not to exceed 8' - 0".
Valley Pads	See Plans
Jack Post	2 x 4's to be placed under valley jacks at no more than 8' - 0" o.c. and nailed To Jack Purlins.
Jack Purlins	2 x 4 x 4' - 0" spanning over three Trusses underneath and parallel to valley jacks To which the jack post supports are nailed.
Ridge Board	Minimum 2 x 6 No. 2 Hem-Fir.
Ridge Post	2 x 4's to be placed under ridge board at no more than 5' – 0" o.c. and Nailed to ridge Purlin.
Ridge Purlin	2 x 4 Whose Length is ½ the valley width at the point indicated by A Ridge Post.

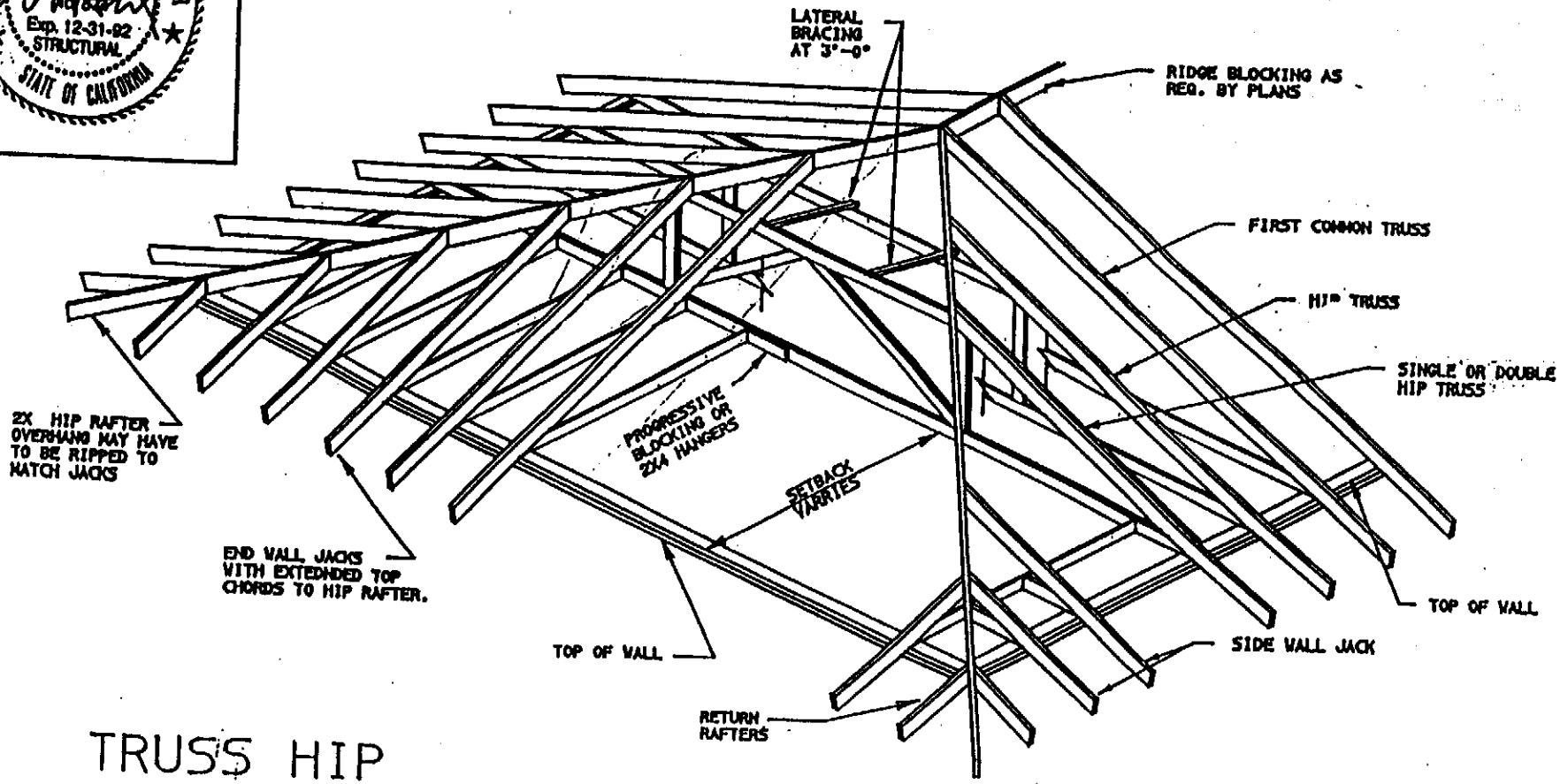
Nailing & Span Table per U.B.C.



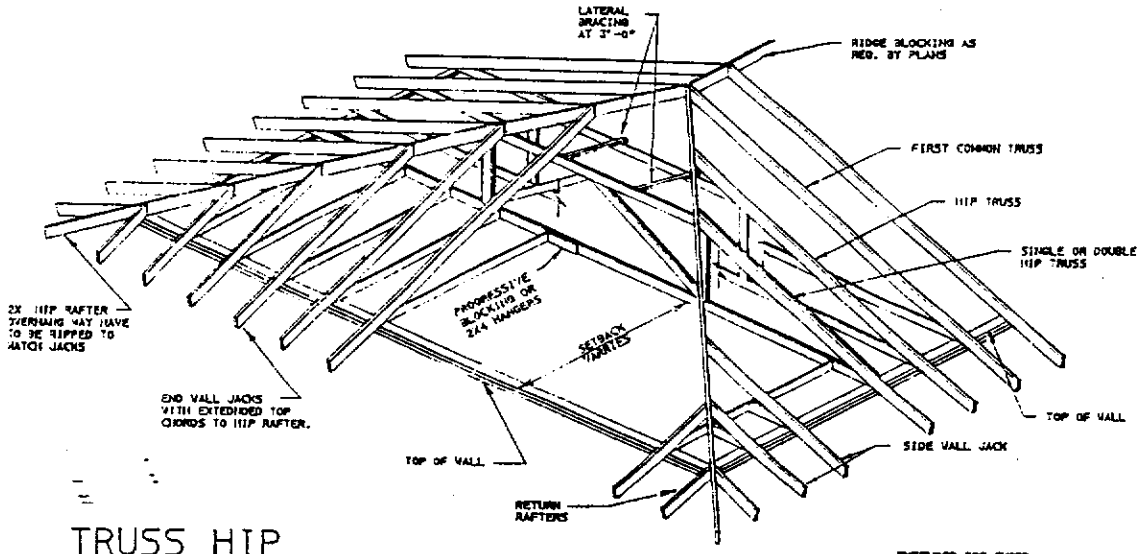
SECTION A-A



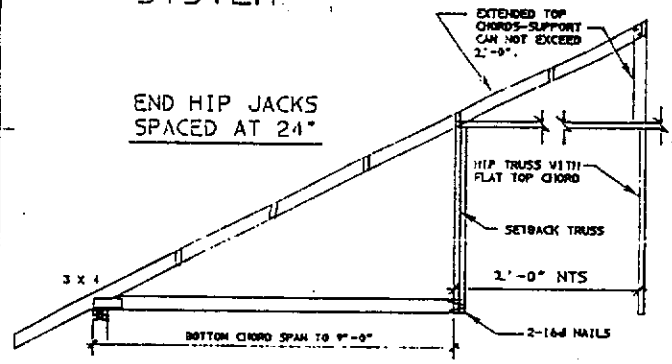
REGISTERED PROFESSIONAL ENGINEER
GORDON M. HART
Exp. 12-31-82
STRUCTURAL
STATE OF CALIFORNIA

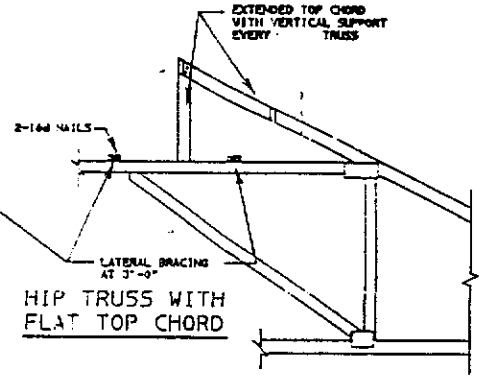


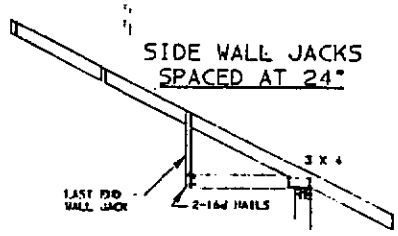
TRUSS HIP SYSTEM

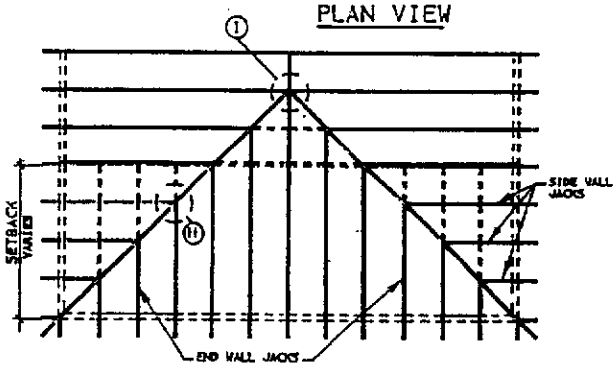


TRUSS HIP SYSTEM



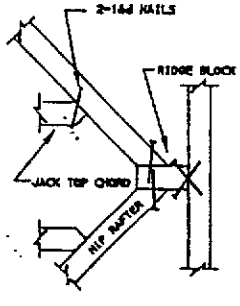






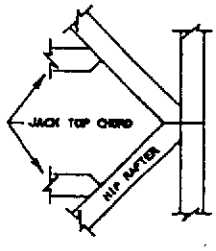
JACK TO HIP CONNECTION

DETAIL (H)



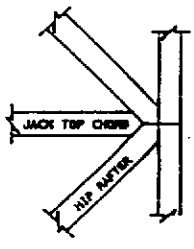
HIP TO TRUSS CONNECTION

DETAIL (1)



HIP TO TRUSS CONNECTION

DETAIL (1)



HIP TO TRUSS CONNECTION

DETAIL (1)

Timber Products Certificate of Quality

TIMBER PRODUCTS INSPECTION, INC.
dba
GENERAL TESTING AND INSPECTION AGENCY

PO Box 55878 - 97238
6600 NE 78th Ct., Suite A-5
Portland, OR 97218

We are an inspection agency recognized by the International Conference of Building Officials. Council of American Building Officials NER - QA275.

This is to verify that:

WALKER LUMBER, INC.
P.O.BOX 958
NORTH HIGHLANDS, CA 95660

is under our Audited Quality Control Program and has been since:

AUGUST 1, 1987

We audit the production under the Uniform Building Code Section 2304.4.4. Inspections are made at least quarterly. We find the quality observance of this plant to be in Conformance to the Standards.

Tony Lewin

TONY LEWIN
MANAGER OF WESTERN TRUSS DIVISION

WARNING

DO NOT INSTALL THESE TRUSSES BEFORE READING INSTRUCTIONS.

ERECTOR

DO NOT INSTALL THESE TRUSSES BEFORE READING AND COMPLYING WITH THESE INSTRUCTIONS.

GENERAL CONTRACTOR

DO NOT LOAD THESE TRUSSES WITH PERMANENT OR TEMPORARY LOADS BEFORE READING THESE INSTRUCTIONS.

1. BEFORE ERECTING THESE TRUSSES ERECTOR SHOULD:

   A. OBTAIN, STUDY AND COMPLY WITH "BRACING WOOD TRUSSES" (BWT 76) AS PUBLISHED BY TPI. OBTAIN THIS GUIDE FROM: THE FABRICATOR OR DEALER WHO PROVIDED THE TRUSSES OR TPI, 7411 RIGGS ROAD, HYATTSVILLE, MARYLAND 20783

ALTERNATE

   B. OBTAIN COMPLETE ERECTION BRACING PLAN FROM BUILDING ARCHITECT, ENGINEER OR STRUCTURALLY COMPETENT DESIGNER.

CAUTION

ERECTOR IS RESPONSIBLE FOR ALL DAMAGES OR INJURY AS A RESULT OF INADEQUATE BRACING FAILURES OCCURRING DURING ERECTION AND PRIOR TO INSTALLATION OF PERMANENT BRACING. COMPLIANCE WITH THIS INDUSTRY GUIDE IS IN YOUR BEST INTEREST.

(OVER)

2. BEFORE LOADING THESE TRUSSES WITH TEMPORARY OR PERMANENT LOADS GENERAL CONTRACTOR OR FRAMING CONTRACTOR SHOULD OBTAIN, STUDY AND COMPLY WITH BUILDING ARCHITECT OR ENGINEERS PERMANENT BRACING PLAN WHICH MUST INCLUDE SPECIFIC DETAILS RELATIVE TO ATTACHMENT, SIZE AND SECURING OF ALL BRACING SHOWN ON YOUR TRUSS SHOP DRAWING.

SECOND WARNING

COMPLIANCE WITH THESE INSTRUCTIONS WILL REDUCE OR ELIMINATE ERECTION AND OR BRACING FAILURES. YOUR COOPERATION IS REQUESTED BY THE TRUSS INDUSTRY TO BRING ABOUT SAFER TRUSS INSTALLATIONS.

THANK YOU

MIKE WALKER LUMBER CO. INC.

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