

CITY OF SACRAMENTO
1231 I Street, Sacramento, CA 95814

Permit No: 0005571
Insp Area: 3

Site Address: 841 38TH ST SAC
Parcel No: 841-0032-037

Sub-Type: ASFR
Housing (Y/N): N

CONTRACTOR

OWNER
MIX MICHAEL
1851 HERITAGE LANE
SACTO. CA. 95815

ARCHITECT

Nature of Work: REMODEL, ADDITION. 900 SQ. FT. FIRST FLOOR. 2ND FLOOR 900 SQ. FT.

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class _____ License Number _____ Date _____ Contractor Signature _____

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

X I, as owner of the property, am exclusively contracting with licensed contractors to construct or improve (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

PAID
CITY OF SACRAMENTO
JUL 31 2000

I am exempt under Sec _____ B & PC for this reason: _____

X Date _____ X Owner Signature Michael Odean (NEIGHBORHOODS, PLANNING & DEVELOPMENT SERVICES AGENT)

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the above mentioned property for inspection purposes.

X Date 8/31/00 X Applicant/Agent Signature Michael Odean

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:
I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier _____ Policy Number _____ Exp Date _____

X This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, shall forthwith comply with those provisions.

X Date 8/31/00 X Applicant Signature Michael Odean

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

Date of Request: 5/23/00
By: _____

CITY OF SACRAMENTO DEVELOPMENT SERVICES DIVISION
PLANNING AND ZONING INFORMATION REQUEST

Project Address: 841 38TH STREET, SACRAMENTO CA 95819

Assessor's Parcel Number: 008-0032-037-0000

~~Previous Use:~~ EXISTING SINGLE FAMILY R1

Description of Request/Proposed Use: REMODEL/ADDITION

~~add on to~~ Make 2-story addition to existing 1-story single family dwelling

Is This a Change of Use? NO

Zoning Designation: R1

Prior Applications for Project Site(P#, Z#, DRPB#): none

Comments: None were ~~not~~ not exceeding 40% lot coverage. Okay to continue 4' side yard setback on north side of house (per Section Ch. 4, Sec. 1-C-2-a-1.)

Are There Any Planning Issues?: (circle one) ~~YES~~ NO See above

- * Staff Site Plan Check Required? (Circle one) YES NO
- * Field Inspection Required? (Circle one) YES NO
- * Design Review/Preservation Required?: (Circle one) YES NO

Planning Review by/Date: [Signature] 5/23/00

A list of items that must be reviewed by Planning is provided on the reverse side of this form.

MICROFILM AFTER FINAL

5/23/00

THE INTENT OF THIS LETTER IS TO INDICATE THAT MICHAEL ODEM HAS THE AUTHORITY TO SIGN FOR PERMITS AND RELATED DOCUMENTS AS HE IS THE PROPERTY MANAGER FOR 841 38TH STREEET.

THANK YOU,



Michael Mix
418-6225
996-3300 (cell)

REVISION M

KIM C. STALEY
CIVIL ENGINEER

0005571

I. OVERCUT IN WALL STUDS
FOR 3" WASTE VENT

Job 841 38TH STREET
SACRAMENTO
Date 11/2001 Sheet 1 Of 2

EVEN THOUGH STUDS WERE DRILLED BEYOND THE MAX.
PER CODE, THE WALL CARRIES MINIMUM LOAD PER
CALCS:

$$\text{LOAD FROM ROOF} = 2\frac{1}{2}(20 \times 10) = 300 \text{ LB/FT}$$

$$\text{LOAD FROM UPPER WALL} = 8 (15 \text{ LB/FT}^2) = 120 \text{ LB/FT}$$

$$\text{Total} = 420 \text{ LB/FT}$$

HOWEVER - MICRO RIM JOIST

CARRIES LOAD

CHECK L=9'

a. Shear $V = 9\frac{1}{2}(420) = 1890 \text{ LB}$

$$A = 1890 (3\frac{1}{2})(95) = 29,84 \text{ IN}^2$$

b. Bending $M = 9\frac{1}{2}(420) = 4253 \text{ LB}$

$$S = 4253 (12) / 1300 = 39.26 \text{ IN}^3$$

c. Deflection $\Delta = 9 \times 12 / 360 = .30$

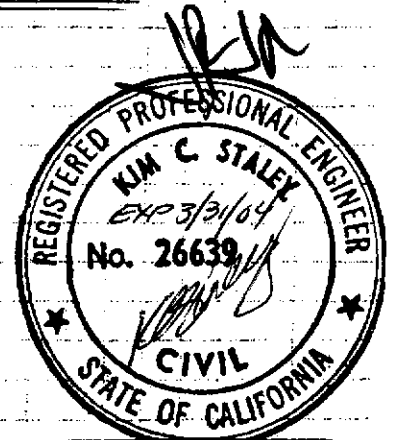
$$I = 5(420)(12)(9 \times 12)^4 / 384(1.9 \times 10^6)(.30) = 108.70 \text{ IN}^4$$

1 3/4 x 11 1/4 MICROLAM RIM JOIST WILL

CARRY ALL OF THE LOAD ABOVE.

SO THERE IS VIRTUALLY NO LOAD

ON CUT STUDS - RECOMMENDATION:

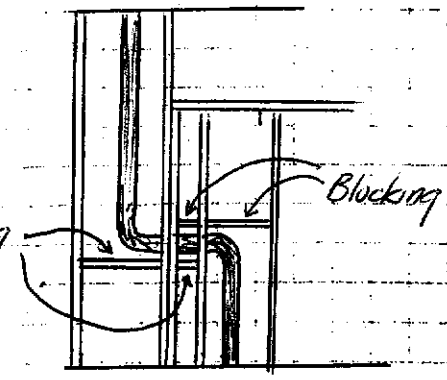


KIM C. STALEY
CIVIL ENGINEER

Job 841 38TH STREET
SACRAMENTO
Date 11/2001 Sheet 2 Of 2

SOLID BLOCKING AS SHOWN TO
STIFFEN UP STUDS IN SIDEWAY
DIRECTION.

EXTERIOR PLYWOOD/STUCCO
WILL STIFFEN IN FRONT
TO BACK DIRECTION AND
THIS WILL BE SATISFACTORY

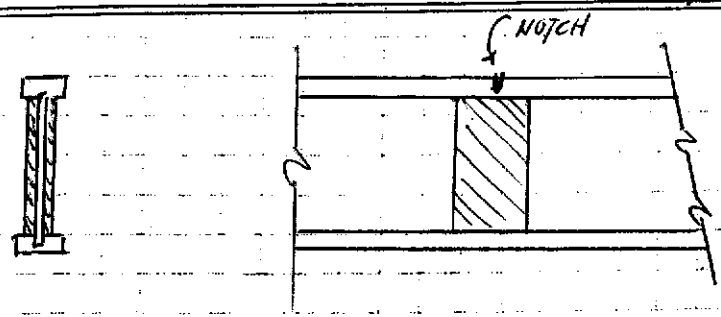


II. NOTCH IN TOP OF FLANGE

IN CUTTING A VENT HOLE THROUGH UPPER
FLOOR, A SLIGHT CUT WAS MADE INTO THE TOP
FLANGE OF A TIE @ THE CORNER OF THE BATHROOM

THE NOTCH, FORTUNATELY OCCURS DIRECTLY ABOVE THE
SUPPORTING WALL SO IT SHOULD NOT PRESENT A
STRUCTURAL PROBLEM

RECOMMENDATION: BLOCK THE WEBS ON BOTH SIDES
OF TIE AT NOTCH FOR ADDED PROTECTION, USING $\frac{3}{8}$ x 4" WIDE
PIECES MIN.



ISSUED
NOV 30 2001
Sacramento Building Division



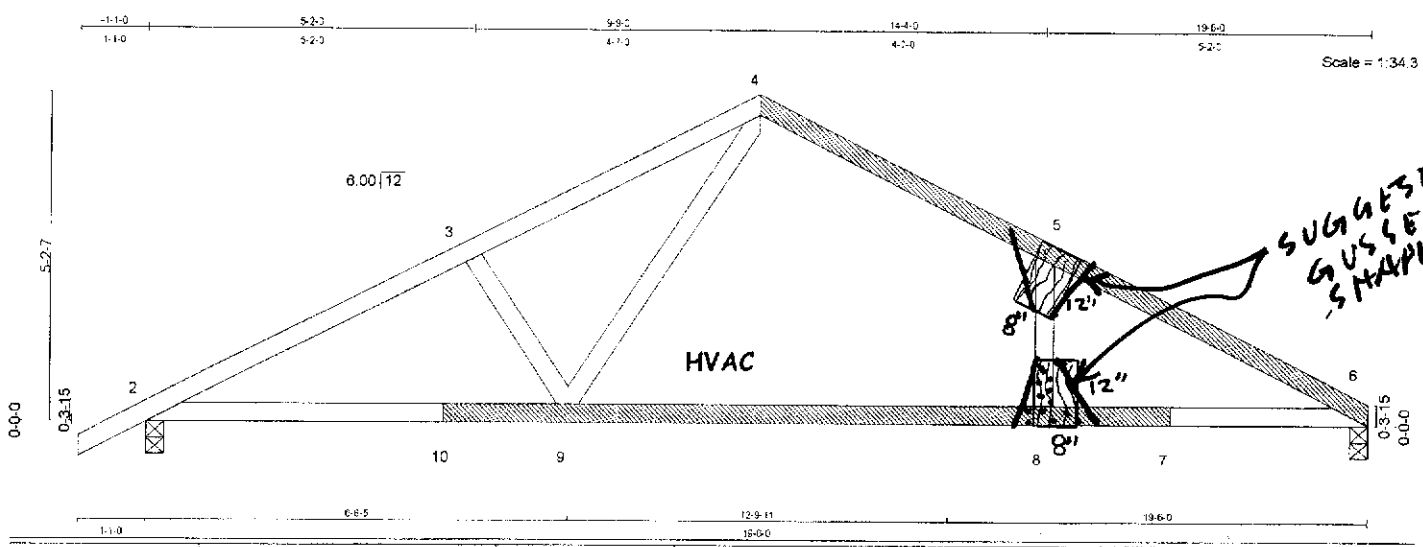
TRUSS REPAIR FOR HVAC

0005571 R

Job	Truss	Truss Type	Qty	Ply	841 38th Street (optional)
DYNAMIC	R1	COMMON	2	1	

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①



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 16.0	2-0-0	TC 0.90	in (loc) l/defl		
TCDL 19.8	Plates Increase 1.00	BC 0.58	Vert(LL) 0.25 8-9 >910		
BCLL 0.0	Lumber Increase 1.25	WB 0.34	Vert(TL) -0.55 8-9 >420		
BCDL 7.0	Rep Stress Incr NO	(Max)	Horz(TL) 0.03 6 n/a		
	Code UBC97/ANSI95		1st LC LL Min l/defl = 360	Weight: 103.1 lb	

LUMBER
 TOP CHORD 2 X 4 DF No.1&Btr
 BOT CHORD 2 X 4 DF No.1&Btr
 WEBS 2 X 4 DF Stud/Std
 LBR SCAB 7-10 2 X 4 DF X No.1&Btr one side
 4-6 2 X 4 DF X No.1&Btr one side

BRACING
 TOP CHORD Sheathed or 3-10-3 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1001/0-3-8, 6=922/0-3-8
 Max Horz 2=81(load case 3)
 Max Uplift 2=-217(load case 4), 6=-156(load case 4)

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=34.5, 2-3=-1699.2, 3-4=-1522.8, 4-5=-1110.5, 5-6=-1295.0
 BOT CHORD 2-10=1422.0, 9-10=1422.0, 8-9=1022.9, 7-8=1022.9, 6-7=1022.9
 WEBS 3-9=-158.0, 4-9=594.6, 5-8=44.0

- NOTES (5-9)**
- This truss has been checked for unbalanced loading conditions.
 - This truss has been designed for the wind loads generated by 80.0 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 216.9 lb uplift at joint 2 and 156.0 lb uplift at joint 6.
 - This truss has been designed with ANSI/TPI 1-1995 criteria.
 - REPAIR NOTES when truss webs removed;
 - Truss repair calculations based on information from fabricator/contractor whose responsibility it is to verify the adequacy of repair as to its field application. Return all joints to original position before applying repair. The end distance, edge distance, nail spacing, and size of plywood gussets and/or truss members shall be such as to avoid splitting of the wood.
 - Using 10d nails staggered 6" oc, attach to one side of the top chord an equal grade and size SCAB as shown.
 - Using 10d nails staggered 6" oc, attach to one side of the bottom chord an equal grade and size SCAB as shown.
 - Field-fit 2x4 #1&Btr DF web 5-8 using 3/4" CDX plywood one side of truss, 12-8d each member each end as shown. Obtain wood-to-wood tight-fitting joints.

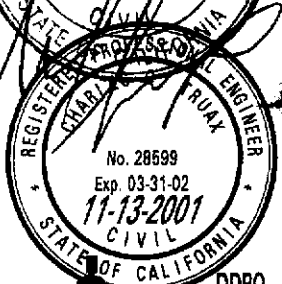
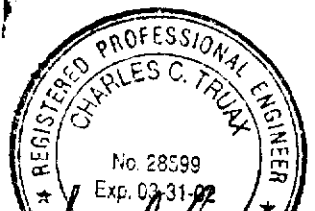
LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.00

Continued on page 2

Kept on file job at all times and available for review and copies of all drawings and specifications without written permission from the Building Inspection Division.

The approval of this plan and specification SHALL NOT be held to permit or approve the violation of any City Ordinance or State Law.

Charles C. Truax
 11/21/01



GENERAL NOTES: This individual building component is designed in accordance with UBC specifications and is to be used in a building system designed by others. The input loading criteria and dimensions were provided by others and must be verified and approved for the specific application by the project design professional. Design assumes adequate drainage and a Dry-Condition use in a Non-Corrosive environment without the use of Fire-Retardant or Preservative-Treated lumber. Shim or wedge if necessary to provide full bearing area required. Cut truss members to bear tightly against each other. Installation is entirely the responsibility of the contractor. All bracing, temporary and permanent, is the responsibility of others. For additional information contact International Conference of Building Officials or Truss Plate Institute.

TRU-TRUSS ENGINEERING
 El Dorado Hills, CA 95762

Job	Truss	Truss Type	Qty	Ply	
DYNAMIC	R1	COMMON	1	1	841 38th Street (optional)

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LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 2-9=-14.0, 8-9=-39.0, 6-8=-14.0, 1-4=-71.6, 4-6=-71.6

Perforated Shearwall: Front Elevation, Entry wall, left of door Project: Odem, 38th St
D. Rt/Lt Lateral Forces, Level 1, Forward Bldg Portion

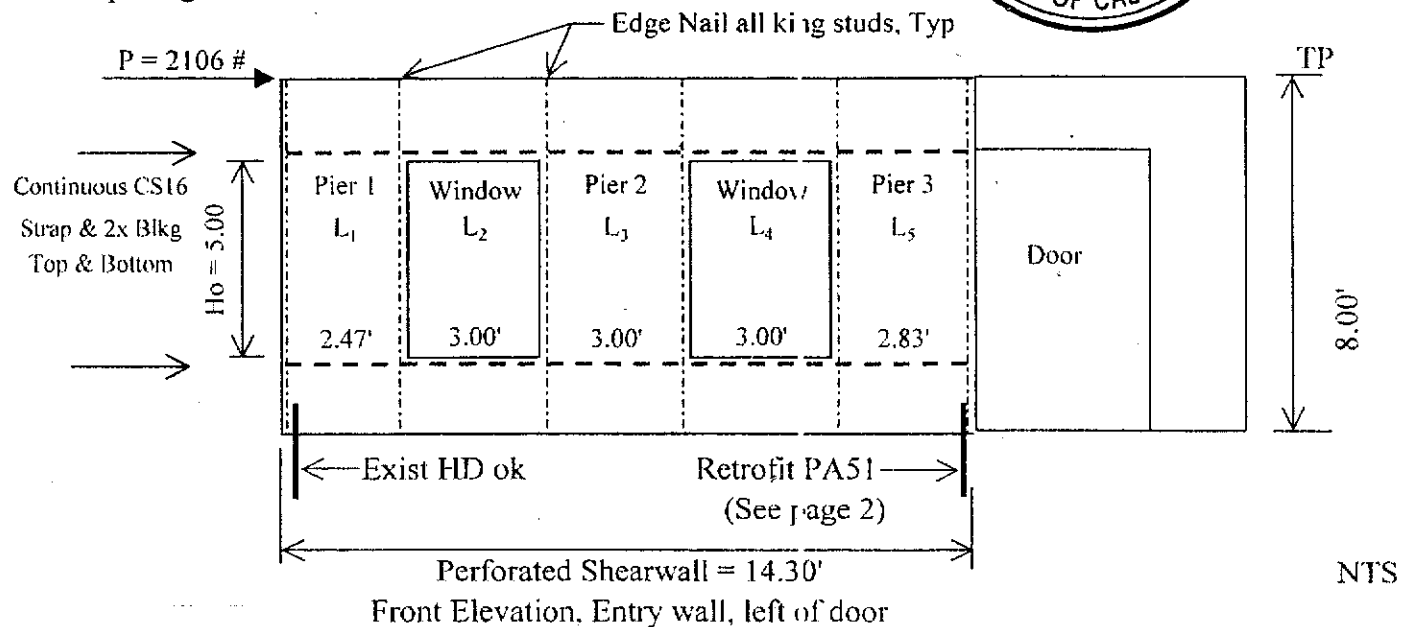
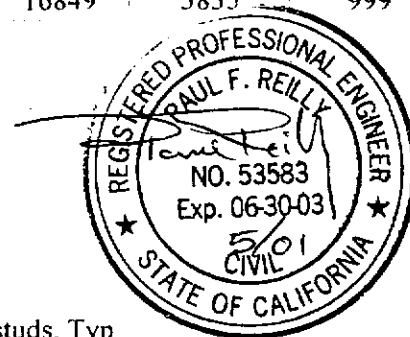
Shearwall Length = 14.30' Unit Shears
 Wall Model Height (H) = 8.00' $v_{top} = v_{bot} = P/L_T = 147 \text{ plf @ Plates}$
 $P = \text{Lateral Force} = 2106 \#$ $v_i = P/(L_1+L_2+L_3) = 254 \text{ plf @ Piers}$
 $W_{Roof DL} = R_D = 42 \text{ plf}$ Gross OTM = V (H)
 $W_{Wall DL} = W_D = 25 \text{ psf}$ Pier OTM = $V_i (H_0)$
 $RM = R_D L_i^2/2 + (R_D/2) L_{i+1}^2/2 + L_i H W_D/2 + L_{i+1}/2 (H-H_0) W_D/2$
 $RM_{DL} \text{ Factor} = 0.67$ $T = OTM - (\text{Factor}) (RM)$

Location	L _i	Height of Opening		Strap lbf at Blocking V _i = v (L _i)	OTM lb-ft	RM lb-ft	T lb-ft	
		L _i	H ₀					
Pier 1	L ₁ = 2.47'			627	3134	582	1111	
Opening	L ₂ = 3.00'	5.00'						
Pier 2	L ₃ = 3.00'			761	3806	696	1113	
Opening	L ₄ = 3.00'	5.00'						
Pier 3	L ₅ = 2.83'			718	3590	658	1113	
	L _T = 14.30'							
Full Wall					16849	3835	999	

Perforated Shearwall Detailing:

(see project plan for shear schedule)

- 1 Provide structural panel sheathing, Shearwall Type 1.
- 2 Set continuous Simpson CS16 strap to the interior face with 8d @ 4" OC, across continuous 2x blocking at the line of both the window sill and header, adjacent to openings.



Revised May 17, 2001

Project Odem, 38th St

REVISE LOCATION OF DOORS & WINDOWS AT FRONT ENTRY WALL
REVISED PERFORATED SHEARWALL DESIGNATION AT FRONT ENTRY WALL

(SEE SCHEMATIC)

Addendum to previous calculations for lateral force model.

Model notes.

1. Building footprint normalized (conservative) for lateral modeling.
2. Wind model: Roof/wall height adjusted for mean height.
3. R_{max} calculates value based on minimum pier width L_w .

D. Rt/Lt Lateral Forces, Level 1, Forward Bldg Portion

Seismic loads.	V factor = 0.117	~ 3.04 plf/ft	D.1 $w_i = W_{wall}/L = 208$ plf
	DL Total = 26.0 psf		D.2 $w_i = W_{wall}/L = 208$ plf
	Wind Load = 11.70 psf		Uplift = $((V*H*Li) - (w_i*0.9*Li^2)/2) / Li$
Ps data: Bldg Length (L) = 19.50'			CF = $(V_{wind}/b)L^2/8b = 167 \#$
Bldg Depth (b) = 31.58'			Est Lvl 1 $V_{story} = 7434 \#$
Pw data: Wall Height (H) = 8.00'			$r_{max} = (V_{wall}/V_{story})(10/L_w) = 0.33$
Roof Ht = (T) = 5.50'			Rho = $2 - 20 / (r_{max} * \text{sqrt}(\text{Area}))$
			Calc Rho = 1.30
			Use Rho = 1.50
			$V_{seismic} = 2810 \#$
$V_{seismic} = (b)(L)(\text{factor}) \times 26.0 \text{ psf} = 1873 \#$			
$V_{wind} = (b)(H/2+T) \times 11.7 \text{ psf} = 3510 \#$		Wind Governs	
Gross lateral load reported. Wall line unit shear = $(\text{Trib})V_{max}/L$			Boundary Shear = $V_{max}/2L = 90 \text{ plf}$

1. Front Elevation	% Trib	Wall	
Unit Base Shear: 147 plf	60%	* Uplift	Anchor Spec Shear Use Shear Wall
Shear wall = 14.30'	*Lt side front door	999 #	** PA51 2106 # Type 1
Total L = 14.30'	* see perforated shearwall calc at end of section		
	** see detail attached for retrofit adjacent to door.		
	Existing holdown at left corner ok.		

Wood to Concrete

[1997 NDS Table 8.2E] 1/2" AB Shear Allow with 2x sill plate: 825# ca AB: Exist AB @ 32" OC ok

Steel Plate to Concrete

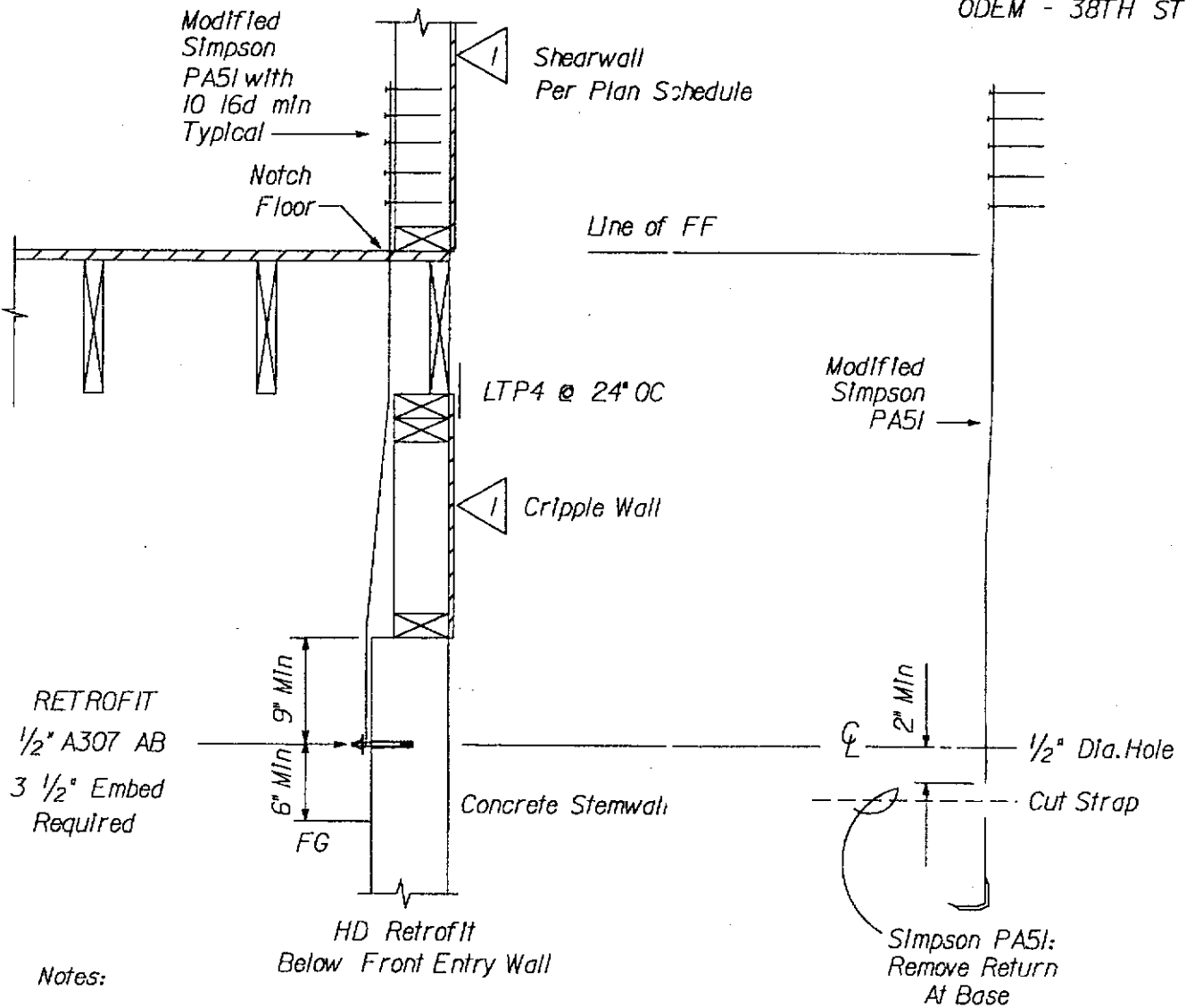
[UBC Table 19-D] $f_c = 2500 \text{ psi}$, 1/2" AB Shear Allow = 1600 lbf (1.33) = 2128 lbf.

One 1/2" AB required to retrofit modified PA51 to stemwall below.

Retrofit HD Set Simpson PA51 strap to interior or exterior face of stemwall with one 1/2" AB.
 (Max strap capacity 3685#)
 10 16d required in double 2x or 4x above for 1287.lbf tension load.

PA51 AB Options: 1/2" Simpson 'Wedge All' or 'Drop In', min embedment 3.5". Ref ICBO ER 3632
 1/2" Rawl 'Power Bolt'. Min embedment 3.5". Ref ICBO ER 5225
 Install retrofit bolt per manufacturers specifications. See 'Concrete Notes' 13 / S1.

ODEM - 38TH ST



Notes:

1. See Details 6 & 12, Sht SI, for nailing requirements to framing members.
2. Set retrofit AB per 'Concrete Notes', 13 SI.
3. PA51 may be set to either face of stemwall to facilitate placement.

RETROFIT HOLDOWN DETAILS



CAPITOL ENGINEERING LABORATORIES, INC.

631 Commerce Drive, Suite #200 • Roseville, California 95678 • (916) 786-2488

JOB REPORT

PAGE: 1 of 1

PROJECT NAME: 841 38th and 20th St

FILE NO. 5445

INSPECTOR: Barry Anderson

DATE: 6-4-01

PERSONS CONTACTED: Michael Ode

PERMIT #:

REFERENCE DOCUMENTS: 2001 05# 4415

WEATHER: cloudy

SERVICE PROVIDED: CONCRETE (INSP/SAMPLE ONLY/PU) MASONRY WELDING (SHOP/FIELD) SOILS

OTHER Survey for foundation

Remarks: Inspected 2 foundation pits at 38th and 20th St. Found 15" dia. pipe at bottom of pits. No soil samples taken.

COMPLIANCE OF WORK:

ATTACHMENTS:

EQUIPMENT/SUPPLIES USED:

NEXT VISIT:

REMARKS: Paid check # 3035 = \$135

REVIEWED BY: Barry Anderson DATE: