

**CITY OF SACRAMENTO**  
1231 I Street, Sacramento, CA 95814

**Permit No: 9712626**  
**Insp Area: 1**

**Site Address: 256 40TH ST SAC**  
**Parcel No: 0040112008**

**Sub-Type: ASFR**  
**Housing (Y/N): N**

**CONTRACTOR**

**OWNER**

**ARCHITECT**

LAINO NICHOLAS J/JAYNEE  
256 40TH ST  
SACRAMENTO CA 95819  
Phone: 916-455-7847

Phone:

Phone:

**Nature of Work: SECOND STORY ADDITION TO DETACHED GARAGE - STORAGE/EXERCISE**  
**- NON-HABITABL**

**CONSTRUCTION LENDING AGENCY :** I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name \_\_\_\_\_ Lender's Address \_\_\_\_\_

**LICENSED CONTRACTORS DECLARATION:** I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class \_\_\_\_\_ License Number \_\_\_\_\_ Date \_\_\_\_\_ Contractor Signature \_\_\_\_\_

**OWNER-BUILDER DECLARATION:** I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

\_\_\_\_ I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his or her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he or she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

\_\_\_\_ I am exempt under Sec. \_\_\_\_\_ B & PC for this reason: \_\_\_\_\_

Date 9-26-97 Owner Signature Nicholas J. Laino

**IN ISSUING THIS BUILDING PERMIT,** the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

Date 9-26-97 Applicant/Agent Signature Nicholas J. Laino

**WORKER'S COMPENSATION DECLARATION:** I hereby affirm under penalty of perjury one of the following declarations:

\_\_\_\_ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

\_\_\_\_ I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier \_\_\_\_\_ Policy Number \_\_\_\_\_

\_\_\_\_ (This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date  9-26-97 Applicant Signature Nicholas J. Laino

**WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.**

**THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.**

29877 X

CITY OF SACRAMENTO  
DEVELOPMENT SERVICES DIVISION

# EXPRESS PLAN REVIEW

DATES					
1ST REVIEW		RECHECK		2ND RECHECK	
IN	OUT	IN	OUT	IN	OUT
1/15/97	9/11/97	9/17/97	1/1	1/1	1/1

*am*

PLAN CHECK NO. <u>SP-2987</u>	COMM. <u>RES.</u>
-------------------------------	-------------------

CONTACT PERSON: NICK LAINO PHONE: 916-455-7847  
 PROJECT ADDRESS: 256 40<sup>TH</sup> STREET SAC. 95819 FAX: 916-455-7837  
 DESCRIPTION OF WORK: SECOND STORY ADDITION TO DETACHED GARAGE

DISCIPLINE	1ST REVIEW			RECHECK			2ND RECHECK		
	EPR	OC	APPR	EPR	OC	APPR	EPR	OC	APPR
LIFE SAFETY			JDC						
STRUCTURAL	<i>AT</i>		<del>NA</del>			<i>AT</i>			
MECHANICAL/PLUMBING			<del>NA</del>						
ELECTRICAL			<del>NA</del>						
FIRE									
PLANNING									

Legend  
 EPR = OK for Express Plan Review  
 OC = OK for Over the Counter Recheck  
 APPR = Approved as submitted

*9/12/97 forced comments - am*  
*AT*  
*OWNER COMMENT*

CITY OF SACRAMENTO

COMMERCIAL PLAN CHECKING/PERMIT SERVICES SECTION

PC 29977 ADDRESS 2510 HOWM ST

I am in receipt of the above plans and I will return the plans upon my review and completion with the appropriate corrections. I am aware of the fact that my delay or failure in returning the plans to the Building Department may delay the issuance of a permit and may constitute a complete recheck of the plans.

NOTE: RETURN CHECK SET OF PLANS WITH NEXT SUBMITTAL! Please cloud, delta, and date all revisions with next submittal of plans, indicate detail and sheet number in last column where correction was made on plans.

[Signature]  
REPRESENTATIVE

[Signature]  
COMPANY REPRESENTING

9/25/97  
DATE

CITY USE ONLY

NOTE ON COMPUTER: \_\_\_\_\_

DATE OUT: 9 25 97

DATE RETURNED: \_\_\_\_\_

# OF BLDG SETS: 2

# OF BLDG SETS RET: \_\_\_\_\_

# OF SITE SETS: \_\_\_\_\_

# OF SITE SETS RET: \_\_\_\_\_

# OF CYCLES: 2

# OF CYCLES RET: \_\_\_\_\_

CHECKED OUT BY: [Signature]

RETURNED TO: \_\_\_\_\_

DATA ENTRY (OUT) BY: \_\_\_\_\_

DATA ENTRY (RET) BY: \_\_\_\_\_



# CITY OF SACRAMENTO

DEPARTMENT OF PLANNING AND DEVELOPMENT  
Sacramento, Ca 95814

Administration  
Room 300 449-5577  
Building Inspections  
Room 200 449-5716  
Planning  
Room 200 449-5604

Dear Plan check applicant:

It is our aim to provide accurate and efficient plan checking service to you. In order to make this possible, we request that when submitting **revisions** you adhere to the following guidelines:

1. Submit all revisions with a transmittal letter that lists the page number of the sheets being revised. State the disciplines being affected, and if possible enclose a copy of the city plan checker's correction list.
2. Please cloud, delta and date all revisions; (yellow lined acceptable).
3. **It is essential that all plan check revisions be marked with the plan check number and complete address when submitted or messengered in. We will only accept plans which provide this information.**
4. Make sure that the re-submittal package is complete; that **all** disciplines requiring revisions are submitted at the same time.
5. You must submit the **same** number of plans for **revisions** that you did for **the original plan check**. (Generally, four sets for shells, buildings, additions and site work; two for all others).
6. Revised sheets with Title 24 energy information must be stamped by the original T-24 review consultant prior to permit issuance.
7. If possible, have the plan checker briefly review the revisions at the counter to ensure compliance with information requested.

It is our hope that the above suggestions will expedite the plan re-check process for everybody. Please let us know if you have any additional suggestions on how we may better accommodate your needs.

Date 9/17/97

# REVISIONS

THIS SHEET IS TO BE USED WHEN PLANS ARE SUBMITTED WITH PLAN CHECK CORRECTIONS OR REVISIONS ON A PLAN WHICH IS STILL IN THE PLAN CHECK PROCESS.

ORIGINAL ROUTE	B	L	P	M	E	F	S	D	R
Status (opt)									
Revision to be routed to (order)	B	L	P	M	E	F	S	D	R
	13 47 9-19-97								

# of sets submitted 5 BY (NAME) ~~Art~~ Nick LAINO  
 PHONE # 455-7847

Plan Address 2960 40<sup>th</sup> St W

Plan Check # 3057

Submitted to SC

Comments \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

KEEP TRACK OF HOURS? Yes No

CITY OF SACRAMENTO


COMMERCIAL PLAN CHECKING/PERMIT SERVICES SECTION

PC 2987

ADDRESS 256-40<sup>th</sup> ST

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REPRESENTATIVE

\_\_\_\_\_  
COMPANY REPRESENTING

9 15 97  
DATE

\_\_\_\_\_  
CITY USE ONLY

NOTE ON COMPUTER: \_\_\_\_\_

\_\_\_\_\_

DATE OUT: 9 15 97

DATE RETURNED: \_\_\_\_\_

# OF BLDG SETS: 1

# OF BLDG SETS RET: \_\_\_\_\_

# OF SITE SETS: \_\_\_\_\_

# OF SITE SETS RET: \_\_\_\_\_

# OF CYCLES: \_\_\_\_\_

# OF CYCLES RET: \_\_\_\_\_

CHECKED OUT BY: \_\_\_\_\_

RETURNED TO: \_\_\_\_\_

DATA ENTRY (OUT) BY: \_\_\_\_\_

DATA ENTRY (RET) BY: \_\_\_\_\_



# CITY OF SACRAMENTO

DEPARTMENT OF PLANNING AND DEVELOPMENT

Sacramento, Ca 95814

Administration  
Room 300 449-5577  
Building Inspections  
Room 200 449-5716  
Planning  
Room 200 449-5604

Dear Plan check applicant:

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# FLOOR FRAMING

## BCI Joists

PC 298 7 x

**NOTE**

The illustration below is showing several suggested applications for the Boise Cascade products. It is not intended to show an actual house under construction.

BCI joist blocking or 2x4 "squash" block on each side required when supporting a load-bearing wall above.

NOT TO BE USED FOR BRIDGING  
OR BEARING FOR BCI'S

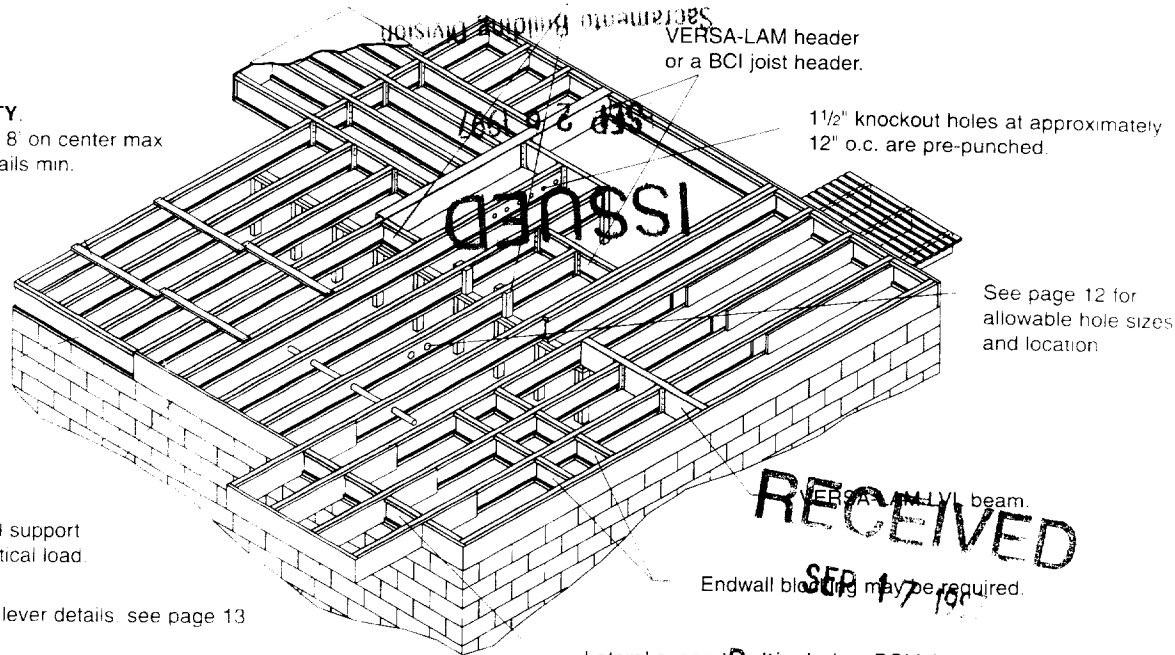
**FOR INSTALLATION STABILITY.**

Temporary struts (1x4 min.) 8' on center max. Fasten to each joist with 2-8d nails min.

BCI joists (where bearing length allows) will support 2000 lbs per lineal foot of vertical load.

VERSA-RIM 98 mm board will support 4000 lbs per lineal foot of vertical load.

For load bearing cantilever details see page 13



**RECEIVED**  
SEP 17 1988

Lateral support required when BCI joists are cantilevered. Use BCI joist blocking for at least every 25 feet of bearing wall length and at least 4 feet on each end of cantilevered area.

O.C. spacing	45 SERIES - 1 3/4" FLANGE WIDTH				60 SERIES - 2 1/16" FLANGE WIDTH							
	9 1/2"	11 7/8"	14"	16"	11 7/8"	14"	16"	18"	20"			
★ CODE APPROVED ★												
12	18'-1"	21'-7"	24'-6"	19'-2"	22'-10"	25'-11"	28'-9"	24'-8"	28'-0"	30'-0"	30'-0"	30'-0"
16	16'-6"	19'-6"	21'-8"	17'-5"	20'-9"	23'-7"	26'-2"	22'-5"	25'-5"	28'-2"	30'-0"	30'-0"
24	15'-5"	17'-10"	19'-9"	16'-5"	19'-7"	21'-10"	24'-5"	19'-10"	21'-10"	24'-11"	29'-0"	30'-0"
32	13'-9"	15'-10"	17'-5"	13'-9"	15'-10"	17'-5"	19'-11"	15'-10"	17'-5"	19'-11"	24'-8"	27'-10"
36	12'-4"	14'-10"	16'-1"	10'-4"	11'-10"	13'-1"	14'-11"	11'-10"	13'-1"	14'-11"	18'-5"	20'-10"
★★★ THREE STAR ★★★												
12	16'-4"	19'-5"	22'-1"	17'-3"	20'-7"	23'-5"	25'-11"	22'-3"	25'-3"	28'-0"	30'-0"	30'-0"
16	14'-10"	17'-8"	20'-1"	15'-8"	18'-8"	21'-3"	23'-6"	20'-2"	22'-11"	25'-4"	27'-9"	30'-0"
24	14'-0"	16'-8"	18'-11"	14'-9"	17'-7"	19'-11"	22'-1"	18'-11"	21'-6"	23'-10"	26'-1"	28'-3"
32	12'-11"	15'-5"	17'-5"	13'-8"	15'-10"	17'-5"	19'-11"	15'-10"	17'-5"	19'-11"	24'-1"	26'-1"
36	10'-4"	12'-10"	14'-1"	10'-4"	11'-10"	13'-1"	14'-11"	11'-10"	13'-1"	14'-11"	18'-5"	20'-10"
★★★★ FOUR STAR ★★★★★												
12	15'-6"	18'-0"	20'-1"	11'-6"	13'-11"	15'-11"	18'-1"	17'-2"	19'-6"	21'-7"	23'-8"	25'-8"
16	13'-5"	15'-7"	17'-6"	11'-6"	13'-4"	15'-4"	17'-1"	15'-5"	17'-7"	19'-6"	21'-4"	23'-1"
24	12'-0"	14'-9"	16'-6"	10'-0"	12'-5"	14'-3"	16'-11"	14'-5"	16'-5"	18'-2"	19'-11"	21'-7"
32	9'-10"	11'-9"	13'-4"	10'-0"	12'-4"	14'-1"	15'-7"	13'-3"	15'-1"	16'-9"	18'-4"	19'-11"
36	8'-7"	10'-4"	11'-10"	8'-7"	10'-4"	11'-10"	13'-2"	11'-2"	12'-9"	14'-2"	15'-7"	16'-11"

Span tables assume that sheathing is glued and nailed to joists.

Spans represent the most restrictive of simple or multiple span applications.

Span tables are based on a residential floor load of 40 PSF live load and 10 PSF dead load, and a clear distance between supports.

- Code allowed live load deflection at L/360 see page 3 "about floor performance guide" for additional data.

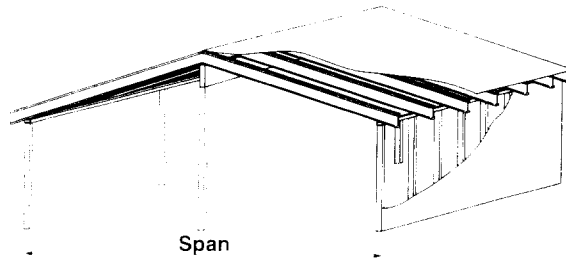
★★★ Live Load deflection at L/480.

★★★★ Live Load deflection at L/960 to give a floor that is much stiffer for the more discriminating purchaser.



# ROOF RIDGE BEAMS

## Versa-Lam Beams



Building Division

ROOF LOADING	COLUMN SPACING (FT.)	SPAN OF SUPPORTED ROOF FRAMING (SUM OF SPANS ON BOTH SIDES OF BEAM IN FT.)						
		20	24	26	30	32	36	40
Non-Snow (125%) 20 PSF Live 10 PSF Dead	14'	3 1/2" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8"	3 1/2" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"
	18'	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 16 5 1/4" x 14	3 1/2" x 16 5 1/4" x 14	3 1/2" x 16 5 1/4" x 14	3 1/2" x 16 5 1/4" x 14
	22'	3 1/2" x 16 5 1/4" x 14	3 1/2" x 16 5 1/4" x 14	3 1/2" x 18 5 1/4" x 16	3 1/2" x 18 5 1/4" x 16	3 1/2" x 18 5 1/4" x 16	3 1/2" x 18 5 1/4" x 16	5 1/4" x 16 5 1/4" x 16
Non-Snow (125%) 20 PSF Live 20 PSF Dead	14'	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	5 1/4" x 14 5 1/4" x 14
	18'	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 16 5 1/4" x 14	5 1/4" x 14	5 1/4" x 14	5 1/4" x 16	5 1/4" x 16	5 1/4" x 16
	22'	3 1/2" x 18 5 1/4" x 16	3 1/2" x 18 5 1/4" x 16	5 1/4" x 16	5 1/4" x 16	5 1/4" x 18	5 1/4" x 18	
Snow (115%) 25 PSF Live 10 PSF Dead	14'	3 1/2" x 9 1/2" 5 1/4" x 7 1/4"	3 1/2" x 9 1/2" 5 1/4" x 7 1/4"	3 1/2" x 9 1/2"	3 1/2" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"
	18'	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"
	22'	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 16 5 1/4" x 14	3 1/2" x 16 5 1/4" x 14	3 1/2" x 16 5 1/4" x 14	3 1/2" x 18 5 1/4" x 14	3 1/2" x 18 5 1/4" x 16
Snow (115%) 30 PSF Live 10 PSF Dead	14'	3 1/2" x 9 1/2" 5 1/4" x 7 1/4"	3 1/2" x 9 1/2" 5 1/4" x 7 1/4"	3 1/2" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"
	18'	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 16 5 1/4" x 11 7/8"	3 1/2" x 16 5 1/4" x 14
	22'	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 16 5 1/4" x 14	3 1/2" x 16 5 1/4" x 14	3 1/2" x 16 5 1/4" x 14	3 1/2" x 18 5 1/4" x 14	3 1/2" x 18 5 1/4" x 16	5 1/4" x 16
Snow (115%) 40 PSF Live 10 PSF Dead	14'	3 1/2" x 9 1/2" 5 1/4" x 7 1/4"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"
	18'	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 16 5 1/4" x 14	3 1/2" x 16 5 1/4" x 14	3 1/2" x 16 5 1/4" x 14	5 1/4" x 14 5 1/4" x 14
	22'	3 1/2" x 16 5 1/4" x 14	3 1/2" x 16 5 1/4" x 14	3 1/2" x 18 5 1/4" x 16	5 1/4" x 16	5 1/4" x 16	5 1/4" x 16	5 1/4" x 18
Snow (115%) 50 PSF Live 10 PSF Dead	14'	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8" 5 1/4" x 9 1/2"	3 1/2" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 14 5 1/4" x 11 7/8"	5 1/4" x 11 7/8"
	18'	3 1/2" x 14 5 1/4" x 11 7/8"	3 1/2" x 16 5 1/4" x 14	3 1/2" x 16 5 1/4" x 14	5 1/4" x 14	5 1/4" x 14	5 1/4" x 16	5 1/4" x 16
	22'	3 1/2" x 18 5 1/4" x 14	3 1/2" x 18 5 1/4" x 16	5 1/4" x 16	5 1/2" x 18	5 1/4" x 18	5 1/4" x 18	

**NOTES**

- Deflection is limited to L/240 at live load and to L/180 at total load.
- Assumes bearing length of 3" at each end and 4 1/2" in shaded area
- Also assumes a bearing length of 7 1/2" at intermediate support and 10 1/2" in shaded area
- Values based on most restrictive of simple or continuous beam span
- Sizes shown are for one-piece applications. For multiple members of 1 1/4" Versa-Lam please contact your distributor or dealer for additional information

Project: BOLDEN - Location: RIDGE BEAM  
 Summary

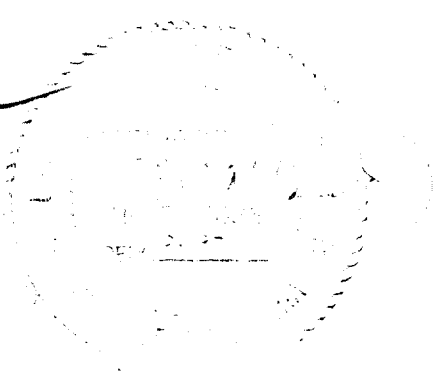
3.50 IN x 16.00 IN x 19.6 FT / 2.0E WS Parallam - TRUS JOIST-MACMILLAN  
 Section Adequate By: 18.9% Controlling Factor: Section Modulus / Depth Required 14.77 In

Deflections			
Dead Load:	DLD=	0.37	IN
Live Load:	LLD=	0.40	IN = L/583
Total Load:	TLD=	0.77	IN = L/305
End Reactions(Left Side):			
Live Load:	RL1=	2352	LB
Dead Load:	RD1=	2221	LB
Total Load:	RT1=	4572	LB
End Reactions(Right Side):			
Live Load:	RL2=	2384	LB
Dead Load:	RD2=	2245	LB
Total Load:	RT2=	4629	LB
Bearing Length Req.(Left)	BL1=	2.01	IN
Bearing Length Req.(Right)	BL2=	2.03	IN
Beam Data			
Span:	L=	19.6	FT
Maximum Unbraced Span:	Lu=	0.0	FT
Live Load Duration Factor:	Cd=	1.00	
Live Load Deflect. Criteria:	L/	360	
Total Load Deflect. Criteria:	L/	240	
Uniform Load:			
Live Load:	wL=	160	PLF
Dead Load:	wD=	153	PLF
Beam Self Weight:	BSW=	14	PLF
Total Load:	wT=	327	PLF
Concentrated Load P1:			
Live Load:	PL1=	1600	LB
Dead Load:	PD1=	1200	LB
Total Load:	PT1=	2800	LB
Location:	X1=	10.0	FT
Properties For: 2.0E WS Parallam- TRUS JOIST-MACMILLAN			
Bending Stress:	Fb=	2900	PSI
Shear Stress:	Fv=	290	PSI
Modulus of Elasticity:	E=	2000000	PSI
Stress Perpendicular to Grain:	Fc_perp=	650	PSI
Adjusted Properties:			
Fb (Tension):	Fb'=	2809	PSI
Adjustment Factors: Cd=1.00 Cf=0.97			
Fv	Fv'=	290	PSI
Adjustment Factors: Cd=1.00			
Design Requirements:			
Maximum Moment:	M=	29389	FT-LB
10.002 FT From Left Support			
Shear (@ d from beam end)	V=	4194	LB
Comparisons With Required Sections:			
Section Modulus:	Sreq=	125.6	IN3
	S=	149.3	IN3
Area:	Areq=	21.7	IN2
	A=	56.0	IN2
Moment of Inertia:	Ireq=	940.4	IN4
	I=	1194.6	IN4

(continued from previous page)

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*Howard G. Taylor*  
 1997



Project: BCLDEN - Location: lateral ridge beams

Summary

3 50 IN x 7 25 IN x 10.0 FT / Select Structural - DOUGLAS FIR-LARCH (NORTH) - Dry Use  
 Section Adequate By: 28.0% Controlling Factor: Section Modulus / Depth Required 6.41 In

Deflections

Dead Load: DLD= 0.16 IN  
 Live Load: LLD= 0.17 IN = L/704  
 Total Load: TLD= 0.33 IN = L/363

Reactions (Each End)

Live Load: RL= 800 LB  
 Dead Load: RD= 752 LB  
 Total Load: RT= 1552 LB  
 Bearing Length Req'd.: BL= 0.71 IN

Beam Data

Span: L= 10.0 FT  
 Maximum Unbraced Span: Lu= 0.0 FT  
 Pitch Of Roof: RP= 8.00 : 12  
 Live Load Deflect. Criteria: L/240  
 Total Load Deflect. Criteria: L/180

Beam Loading

Live Load: LL= 16 PSF  
 Roof Loaded Area: RLA= 100 SF  
 Roof Live Load Method: 1  
 Side One: Roof Dead Load: DL1= 12 PSF  
 Roof Rafter Tributary Width: TW1= 5.0 FT  
 Side Two: Roof Dead Load: DL2= 12 PSF  
 Roof Rafter Tributary Width: TW2= 5.0 FT  
 Roof Duration Factor: Cd= 1.15

Slope Adjusted Lengths and Loads:

Adjusted Beam Length: Ladj= 10.0 FT  
 Beam Live Load W/ Slope Red'n: wL= 160 PLF  
 Beam Self Weight: BSW= 6 PLF  
 Beam Total Dead Load: wD= 150 PLF  
 Total Maximum Load: wT= 310 PLF  
 Controlling Total Design Load: wTcont= 310 PLF

Properties For: Select Structural- DOUGLAS FIR-LARCH (NORTH)

Bending Stress: Fb= 1300 PSI  
 Shear Stress: Fv= 95 PSI  
 Modulus of Elasticity: E= 1900000 PSI  
 Stress Perpendicular to Grain: Fc\_perp= 625 PSI

Adjusted Properties

Fb' (Tension): Fb'= 1943 PSI  
 Adjustment Factors: Cd=1.15 Cf=1.30  
 Fv': Fv'= 109 PSI  
 Adjustment Factors: Cd=1.15

Design Requirements

Maximum Moment: M= 3880 FT-LB  
 Shear (@ d from beam end): V= 1364 LB

Comparisons With Required Sections:

Section Modulus: Sreq= 24.0 IN3  
 S= 30.6 IN3  
 Area: Area= 18.8 IN2  
 A= 25.3 IN2  
 Moment of Inertia: Ireq= 55.2 IN4  
 I= 111.1 IN4

ISSUED  
 SEP 20 1997

Howard G. Taylor

Professional Engineer Seal (left): Howard G. Taylor, License No. 10000, State of Oregon, expires 12/31/97.

Professional Engineer Seal (right): Howard G. Taylor, License No. 10000, State of Oregon, expires 12/31/97.

Project: BOLDEN - Location: valley  
 Summary:

1.75 IN x 11.875 IN x 12.0 FT (Actual 14.422 FT) / 1.8E WS MicroLam - TRUS JOIST-MACMILLAN  
 Section Adequate By: 53.4% Controlling Factor: Section Modulus / Depth Required 9.97 In

Deflections

Dead Load: DLD= 0.28 IN  
 Live Load: LLD= 0.29 IN = L/488  
 Total Load: TLD= 0.57 IN = L/253

Reactions (Each End):

Live Load: RL= 960 LB  
 Dead Load: RD= 896 LB  
 Total Load: RT= 1856 LB  
 Bearing Length Req'd: BL= 1.41 IN

Beam Data

Span: L= 12.0 FT  
 Maximum Unbraced Span: Lu= 0.0 FT  
 Beam End Elevation Diff: EL= 8.0 FT  
 Pitch Of Roof: RP= 8.00 : 12  
 Live Load Deflect. Criteria: U= 240  
 Total Load Deflect. Criteria: L= 180

Beam Loading

Live Load: LL= 16 PSF  
 Roof Loaded Area: RLA= 120 SF  
 Roof Live Load Method: 1  
 Side One: Roof Dead Load: DL1= 12 PSF  
 Roof Rafter Tributary Width: TW1= 5.0 FT  
 Side Two: Roof Dead Load: DL2= 12 PSF  
 Roof Rafter Tributary Width: TW2= 5.0 FT  
 Roof Duration Factor: Cd= 1.15

Slope Adjusted Lengths and Loads:

Adjusted Beam Length: Ladj= 14.422 FT  
 Beam Live Load W/ Slope Red'n: wL= 133 PLF  
 Beam Self Weight: BSW= 4 PLF  
 Beam Total Dead Load: wD= 124 PLF  
 Total Maximum Load: wT= 257 PLF  
 Controlling Total Design Load: wTcont= 257 PLF

Properties For: 1.8E WS MicroLam- TRUS JOIST-MACMILLAN

Bending Stress: Fb= 2600 PSI  
 Shear Stress: Fv= 285 PSI  
 Modulus of Elasticity: E= 1800000 PSI  
 Stress Perpendicular to Grain: Fc\_perp= 750 PSI

Adjusted Properties:

Fb (Tension): Fb'= 2994 PSI  
 Adjustment Factors: Cd=1.15 Cf=1.00  
 Fv: Fv'= 328 PSI  
 Adjustment Factors: Cd=1.15

Design Requirements:

Maximum Moment: M= 6691 FT-LB  
 Shear (@ d from beam end): V= 1601 LB

Comparisons With Required Sections:

Section Modulus: Sreq= 26.9 IN3  
 S= 41.1 IN3  
 Area: Areq= 7.4 IN2  
 A= 20.7 IN2  
 Moment of Inertia: Ireq= 144.8 IN4  
 I= 244.2 IN4

*Howard G. Taylor*  
 10-97

Project BOLDEN - Location header  
 Summary

3.50 IN x 11.25 IN x 5.0 FT / Select Structural - DOUGLAS FIR-LARCH (NORTH) - Dry Use  
 Section Adequate By: 3.0% Controlling Factor: Area / Depth Required 10.93 In

Deflections

Dead Load DLD= 0.01 IN  
 Live Load: LLD= 0.01 IN = L/4422  
 Total Load: TLD= 0.03 IN = L/2263

End Reactions(Left Side)

Live Load: RL1= 1144 LB  
 Dead Load: RD1= 1102 LB  
 Total Load: RT1= 2246 LB

End Reactions(Right Side)

Live Load: RL2= 1240 LB  
 Dead Load: RD2= 1191 LB  
 Total Load: RT2= 2431 LB  
 Bearing Length Req.(Left) BL1= 1.03 IN  
 Bearing Length Req.(Right) BL2= 1.11 IN

Beam Data

Span L= 5.0 FT  
 Maximum Unbraced Span: Lu= 0.0 FT  
 Live Load Duration Factor: 1.00  
 Live Load Deflect. Criteria:  $\leq$  360  
 Total Load Deflect. Criteria:  $\leq$  240

Uniform Load:

Live Load: wL= 0 PLF  
 Dead Load: wD= 0 PLF  
 Beam Self Weight: BSW= 10 PLF  
 Total Load: wT= 10 PLF

Concentrated Load P1

Live Load: PL1= 2384 LB  
 Dead Load: PD1= 2245 LB  
 Total Load: PT1= 4629 LB  
 Location: X1= 2.6 FT

Properties For: Select Structural- DOUGLAS FIR-LARCH (NORTH)

Bending Stress: Fb= 1300 PSI  
 Shear Stress: Fv= 95 PSI  
 Modulus of Elasticity: E= 1900000 PSI  
 Stress Perpendicular to Grain: Fc\_perp= 625 PSI

Adjusted Properties:

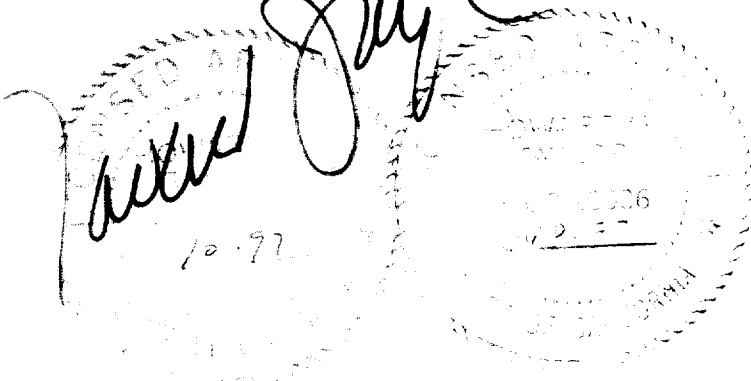
Fb'(Tension) Fb'= 1430 PSI  
 Adjustment Factors: Cd=1.00 Cf=1.10  
 Fv' Fv'= 95 PSI  
 Adjustment Factors: Cd=1.00

Design Requirements:

Maximum Moment: M= 5807 FT-LB  
 2.6 FT From Left Support  
 Shear (@ d from beam end): V= 2422 LB

Comparisons With Required Sections:

Section Modulus: Sreq= 48.8 IN3  
 S= 73.8 IN3  
 Area: Areq= 38.3 IN2  
 A= 39.3 IN2  
 Moment of Inertia: Ireq= 44.1 IN4  
 I= 415.2 IN4

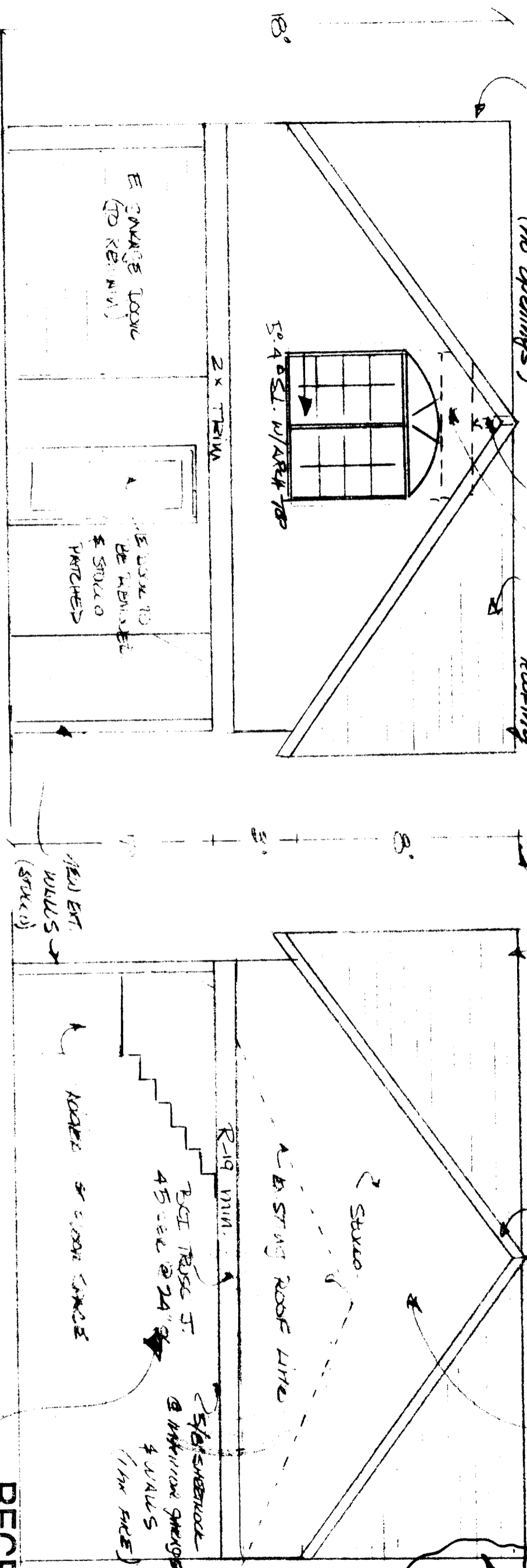
*Howard G. Taylor*  
 10-97  


LATINO GARAGE ALTERATION - (STORAGE ONLY - NOT AVAILABLE SPACE)

256 40th STREET

SACRAMENTO CA

Revised 9/15/97 S. Golden



3 1/2 x 16 LAM. RAFTERS  
4x12 HEADERS

18' MAX HEIGHT  
PENNSI

1 HR. FIRE WALL  
(7/8" STUCCO + 5/8" BRICK  
INTERIOR)

18' max. height  
per obtain a  
variance from  
Planning typ.

NO OVERHANG S.S. SIDE

DOOR TO  
BE REFINISHED  
& STUCCO  
MATCHED

E. GARAGE DOOR  
(TO REMAIN)

NEW EXT.  
WALLS  
(STUCCO)

ROOFLINE STUCCO GARAGE

R-19 MIN.

PCI ROOF ST.  
45' x 24'

5/8" SHEETROCK  
& MINIMUM SPACERS  
& WALLS  
(1/2" FIRE)

RECEIVED  
SEP 17 1997

RECEIVED  
SEP 17 1997

FRONT EAST ELEVATION

REAR WEST ELEVATION

Building Inspection Division

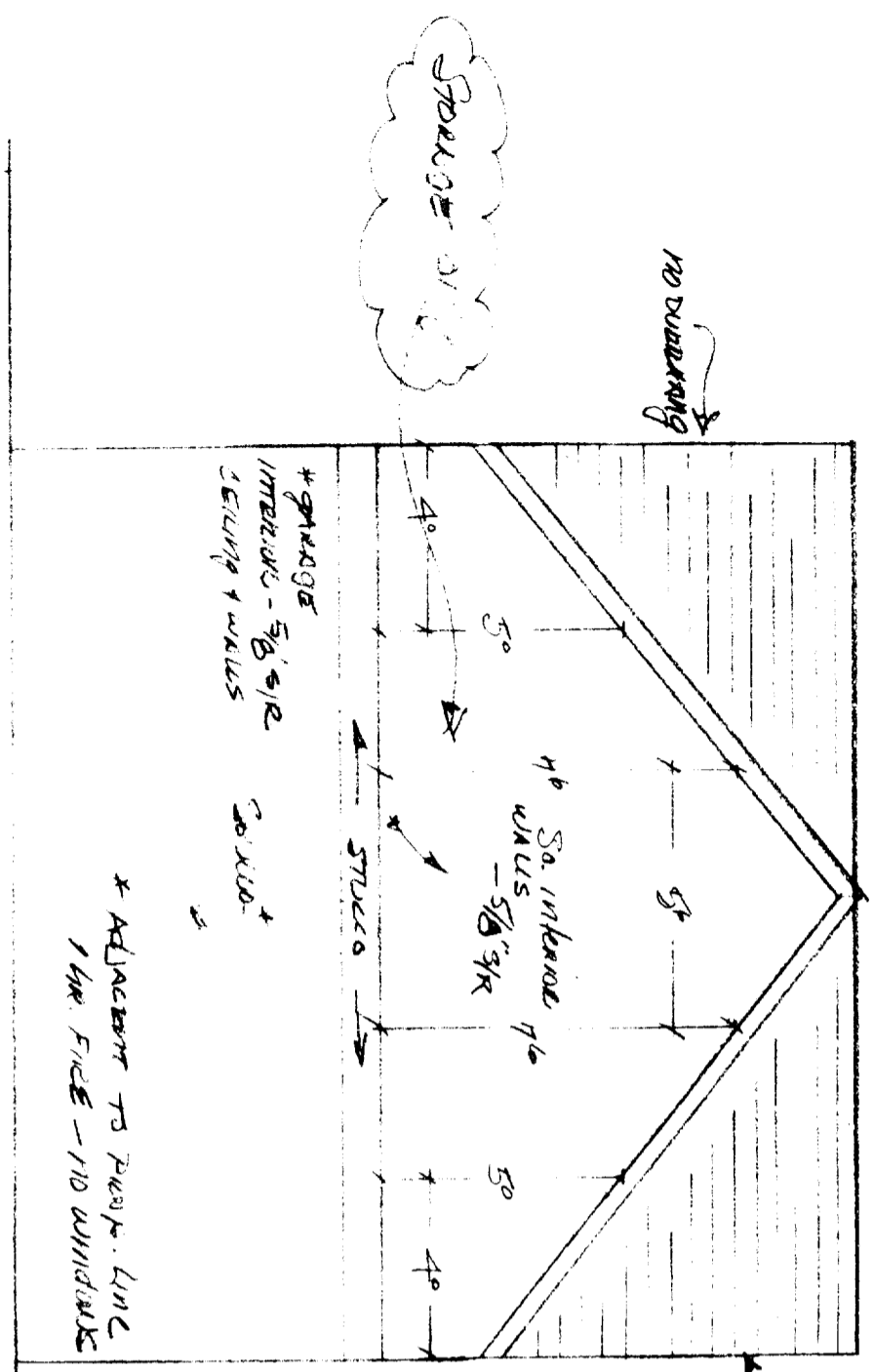
SEP 17 1997  
S. Golden  
4100 ARBOLING DEPARTMENT DIVISION  
2100 40th ST  
SACRAMENTO CA 95800

at 4/980

9/15/97

PC 2992 x 104

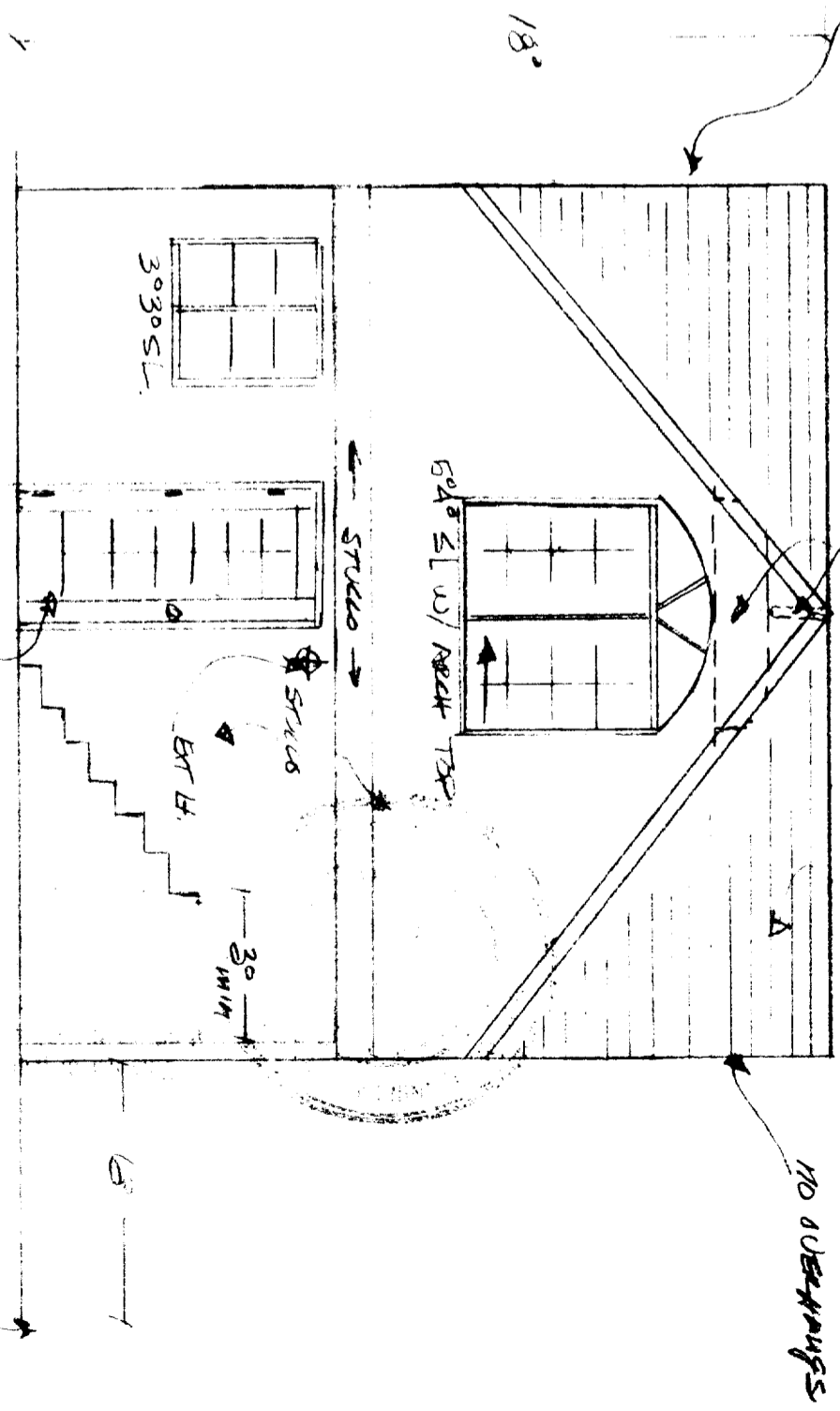
2nd FLOOR AREA = 400 SF  
AREA 76 + = 190 SF  
AREA 50 = 64 SF



SOUTH ELEVATION

19'

NO SUBPARTING



NORTH ELEVATION

RECEIVED  
HW 8058 PERMAN CODE (10/17)  
SEP 17 1997

Building Inspection Division

ISSUED

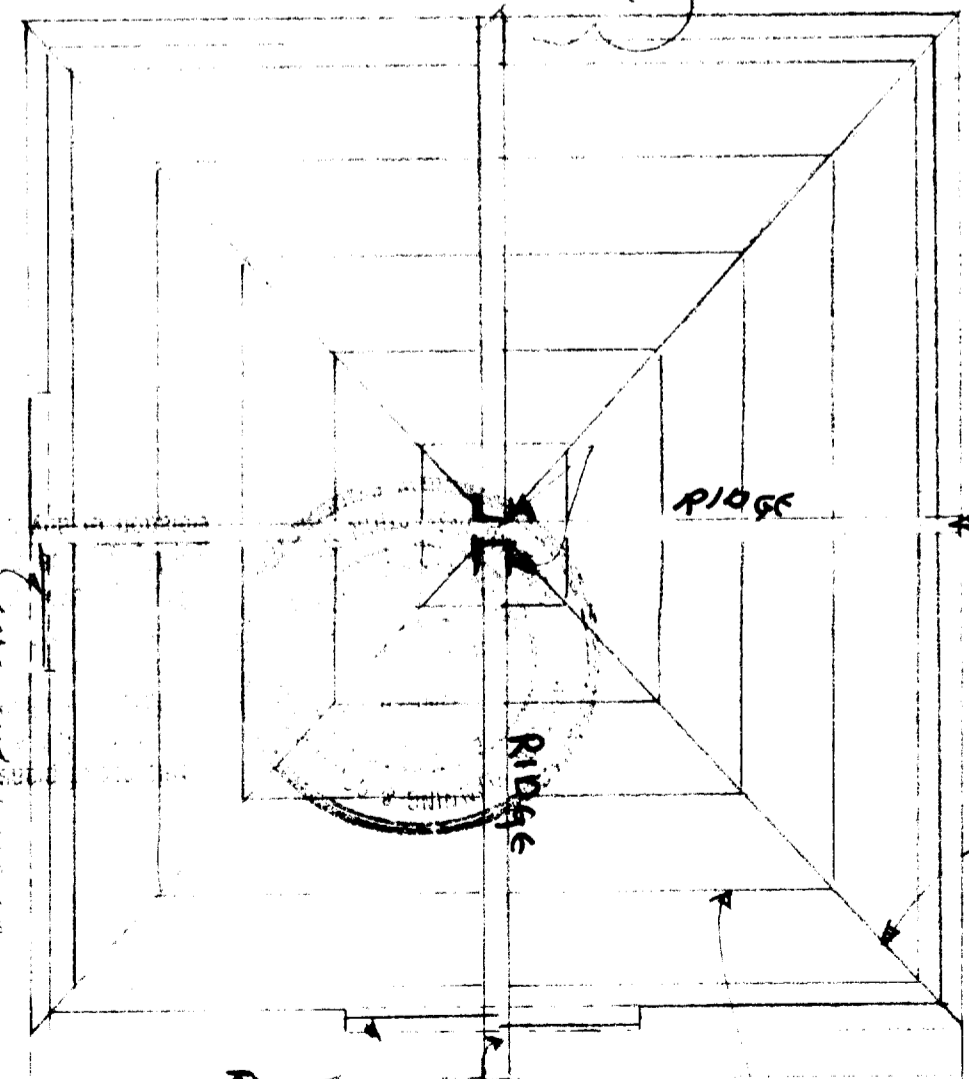
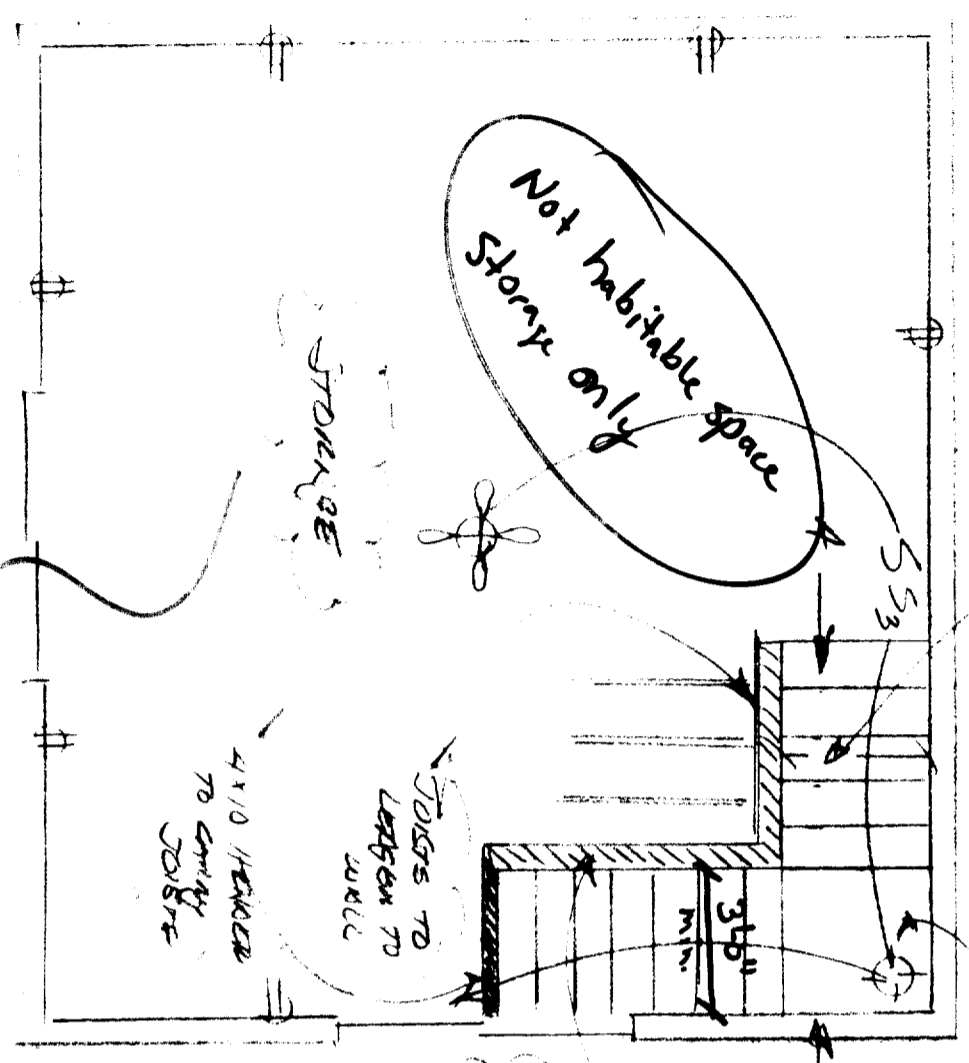
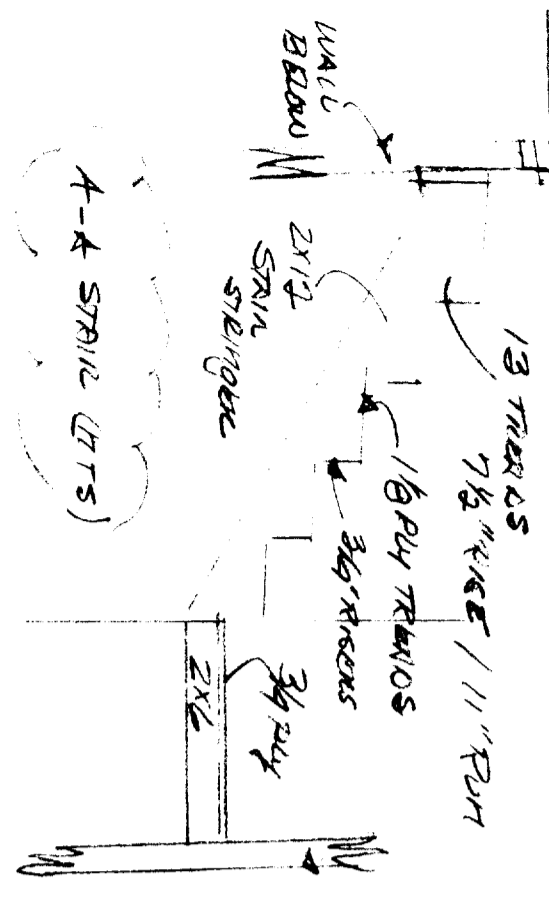
SEP 20 1997

Sacramento Building Division

A







ELECTRICAL PLAN 2nd Floor

POST SIGN STORAGE ONLY  
 MAXIMUM FLOOR LOADING  
 SHALL NOT EXCEED 40 PSF

30 MIN. IS. WIDTH

3030 MIN DISTANCE

SLT 3.611 HANGAR

3 1/2\"/>
 VERSO-LAM BEAM  
 (19\"/>

**ISSUED**  
 10/12/12  
 SEP 20 1997  
 Sacramento Building Division

**RECEIVED**

Building Inspection Division

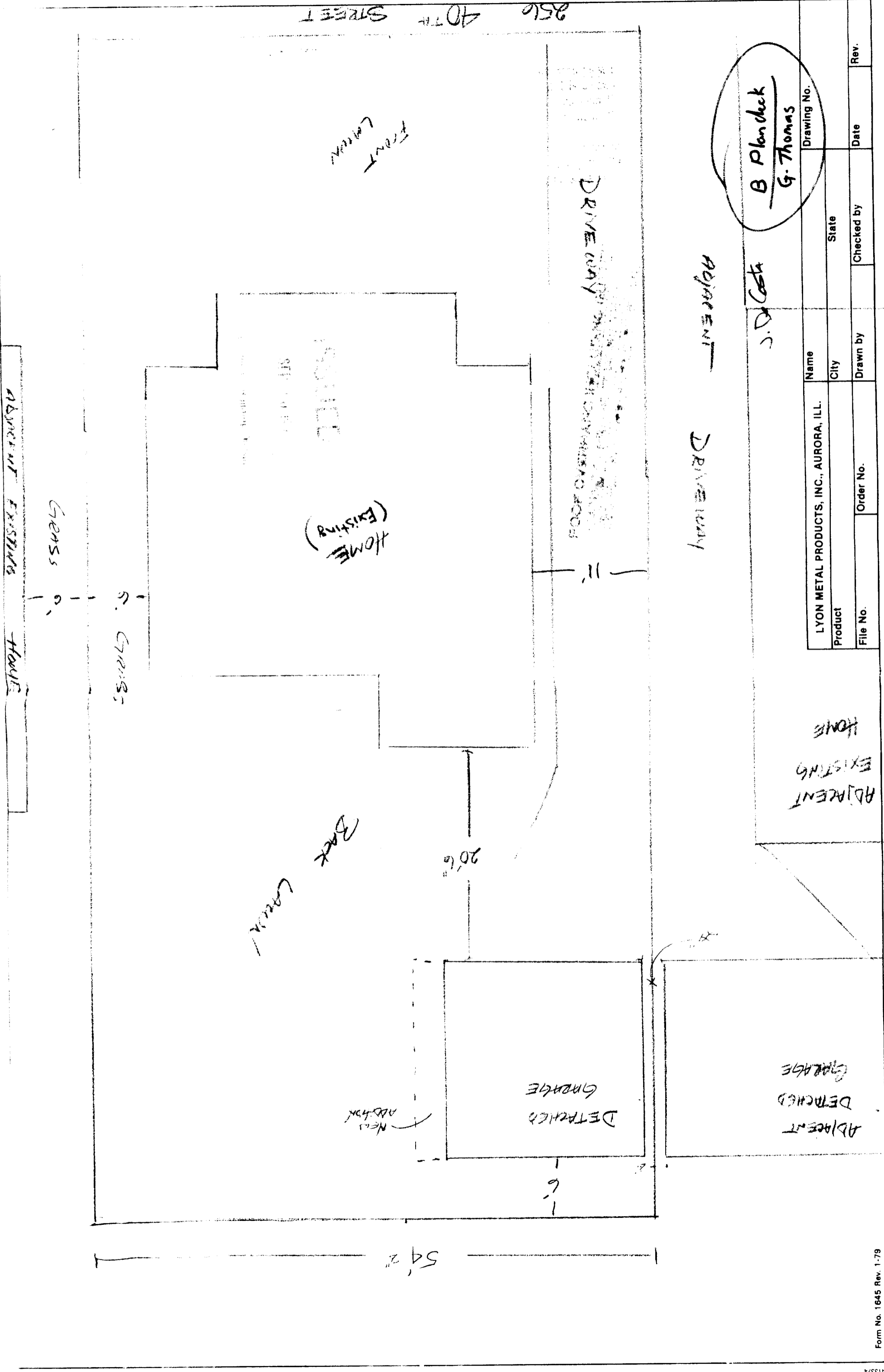
SEP 17

4\"/>
 REFER TO SHEET 1 OF 4

VERSO-LAM  
 3 1/2\"/>

2\"/>
 RAFTERS  
 24\"/>

A



LYON METAL PRODUCTS, INC., AURORA, ILL.		Name		Drawing No.	
Product		City		State	
File No.		Order No.		Checked by	
		Date		Rev.	

**B Plan check**  
**G. Thomas**

J. De Costa

ADJACENT DRIVEWAY

ADJACENT EXISTING HOME

ADJACENT DETACHED GARAGE

DETACHED GARAGE

NEAR ADDRESS

HOME (EXISTING)

BACK LAWN

FRONT LAWN

SOLID

DRIVEWAY

ADJACENT DRIVEWAY

20'6"

5' GRASS

GRASS

ADJACENT EXISTING HOME

54' 2"