

CITY OF SACRAMENTO
1231 I Street, Sacramento, CA 95814

Permit No: 0301523
Insp Area: 2
Thos Bros: 317 B7

Site Address: 6806 SOUTH LAND PARK DR SAC
Parcel No: 029-0051-011

Sub-Type: RES
Housing (Y/N): N

CONTRACTOR
CREATIVE ROOFING
1020 TABER ST
WEST SACRAMENTO, CA 95605

OWNER
HARING LANCE F
6806 SOUTH LAND PARK D
SACRAMENTO CA 95831

ARCHITECT

Nature of Work: TEAR OFF SHAKE/INSTALL LIGHTWEIGHT TILE MONIER TILE W/ENG REPORT

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class C39 License Number 438395 Date 2/4/03 Contractor Signature Patricia Riggs

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

____ I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

____ I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

____ I am exempt under Sec. _____ B & PC for this reason: _____
Date _____ Owner Signature _____

PAID
CITY OF SACRAMENTO

FEB 04 2003

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 2/4/03 Applicant/Agent Signature Patricia Riggs

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

____ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier STATE FUND Policy Number 1313938-00 Exp Date 07/01/2003

____ (This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 2/4/03 Applicant Signature Patricia Riggs

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION DAMAGES INCURRED BY ANYONE INJURED BY SUCH FAILURE.

CITY OF SACRAMENTO
NORTH PERMIT
CENTER

FEB 03 2003

RECEIVED

SCHOEN ENGINEERING

9524 BEDINGTON WAY
SACRAMENTO, CA 95827

(916) 369 6866

Licensed by the California State Board
for Engineers and Land Surveyors
LIC.# C042913

January 23, 2003



Frank Pisi
6806 Southland Park Drive
Sacramento, CA 95831

SUBJECT: Reroof at 6806 Southland Park Drive, Sacramento, CA 95831

Frank:

On January 17th 2003 I inspected your home at the above mentioned address. I found the roof to be conventionally framed with 2x6 D.F. No. 2 rafters @ 2' o.c. spanning a max. of 12' in the garage and 2x6 D.F. no. 2 rafters @ 2' o.c. spanning a max of 7' in the attic areas of the house.

The following modifications will need to be made prior to reroofing:

- * In the garage the rafter pair out 4' from the firewall should have a 2x6 tie installed at the plate level. The tie should be attached to the rafters with 5-16d nails. If it is spliced the splice should lap 4' and be attached with 5-16d nails(see attached sketch for details and plan for location).
- * In the bedroom wing install a vertical ridge brace off of the same wall that the diagonal ridge brace is installed off of. Also, the diagonal ridge brace should be doubled(see attached sketch for detail and plan for location).
- * In the front slope of the main wing of the house some of the purling braces are framed so as to run by the purlin and then rely on a block nailed to the brace to support the purlin. These are not adequate to support the roof loads and should be redone for direct bearing(see sketch for details and plan for location).
- * In the back slope of the main wing over the living room the lower purlin is not currently braced. A new 1-3/4"x 9-1/2" Microlam purlin should be installed just upslope from the existing purlin. Also the ridge over the living room should be supported by a 1-3/4"/9-1/2" Microlam. Both of these Microlams should be braced off of the side walls of the living room(see sketch for location and sketch for details).

It is my finding that this structure is adequate for the following : 1/2" plywood or 7/16 OSB installed over the existing skip sheathing; Tarred felt or similar underlayment installed on top of the plywood or OSB sheathing; 1x2 battens; Lightweight concrete tile weighing 6 lbs./sq.ft. Or less.

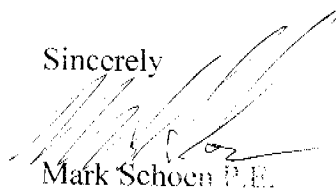
NOTE: It is possible when reroofing that the increased loads to structural elements also some sag and ceiling and floor finishes could cause some minor cosmetic cracking of these finishes. This is typical of wood framed structures and does not of itself indicate structural inadequacy of these members.

This report deals with the structural adequacy of roof supporting members that were readily observable. It does not address any structure that was covered by wall finishes, buried in the ground or otherwise not observable. Any such structures were assumed to conform to standard construction requirements to the Uniform Building Code at the time of construction or that they were designed in accordance to engineering criteria in that code. Also, it does not address any existing deflection or warping of roof surfaces or other finish surfaces. There is also no guarantee that any structural modification listed in this report will remove such deflections or warping. The repair of such deflections or warping is at the option of the building owner and the roofing contractor.

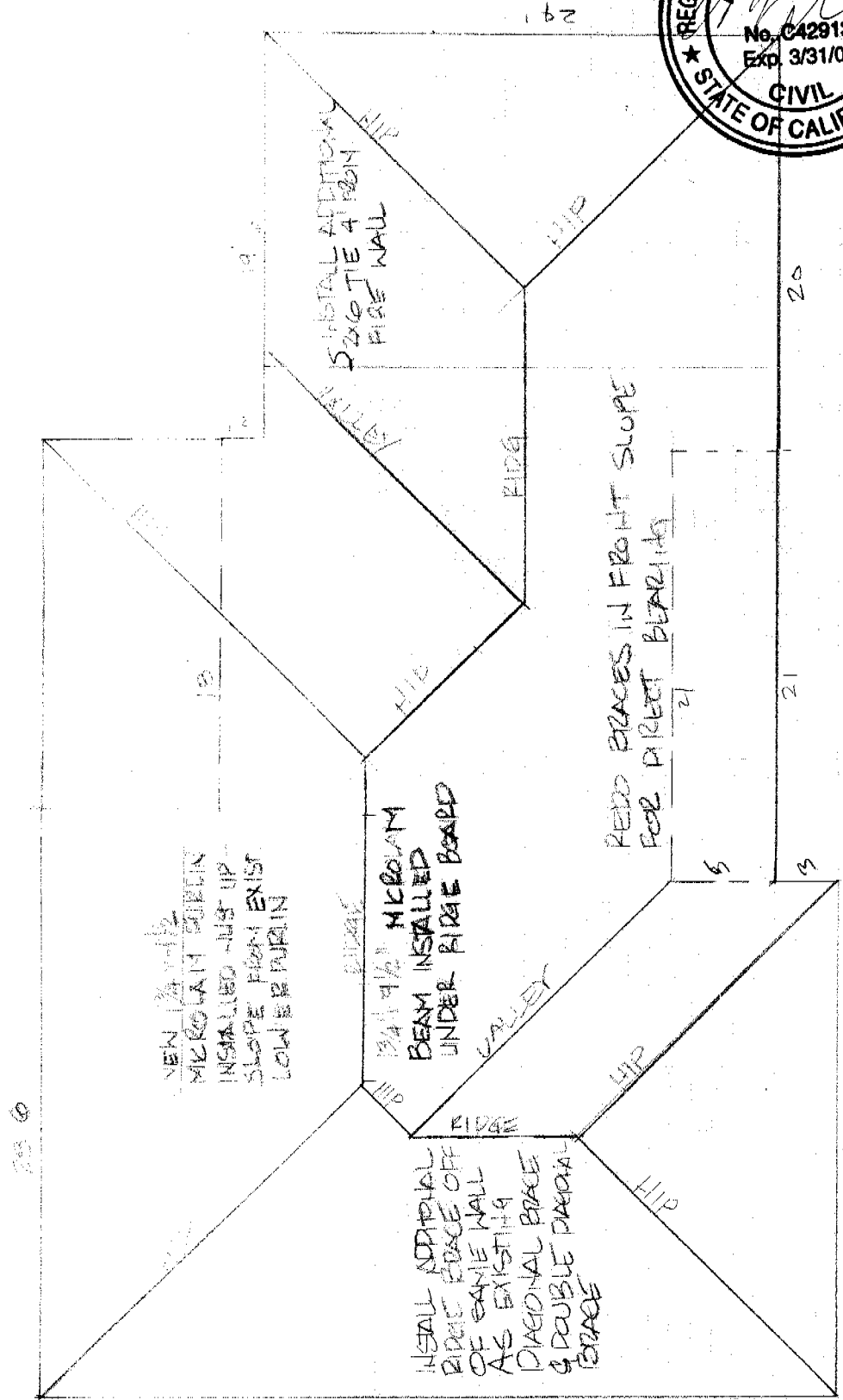
This report has been prepared for the sole benefit of the individual to whom it is addressed. No reliance on this report by any other individuals or entities without the expressed written consent of the above addressee and Schoen Engineering is forbidden. This does not preclude a licensed contractor acting as an agent for the addressee from using this report to obtain a building permit.

I would like to thank you for allowing me to be of service in this matter. Please let me know if I can be of further assistance.

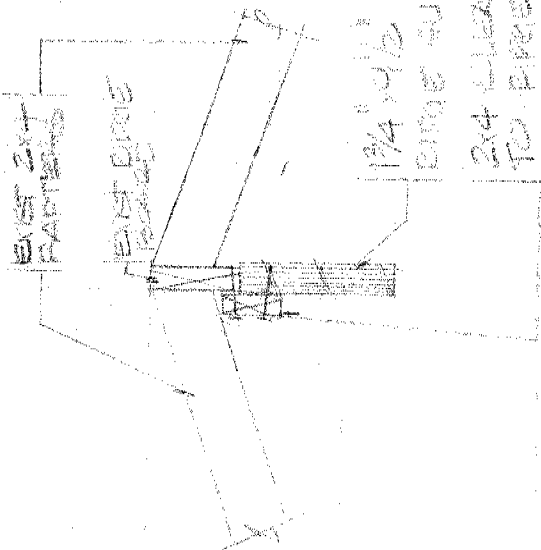
Sincerely



Mark Schoen P.E.



ROOF PLAN FOR
 6806 SOUTH LAND PARK DR
 SACRAMENTO, CA 95831



NOTE: BRACKET WIDTH IS THE SAME AS THE RAFTER EDGE BRACE (SEE EDGE DETAIL DRAWING)

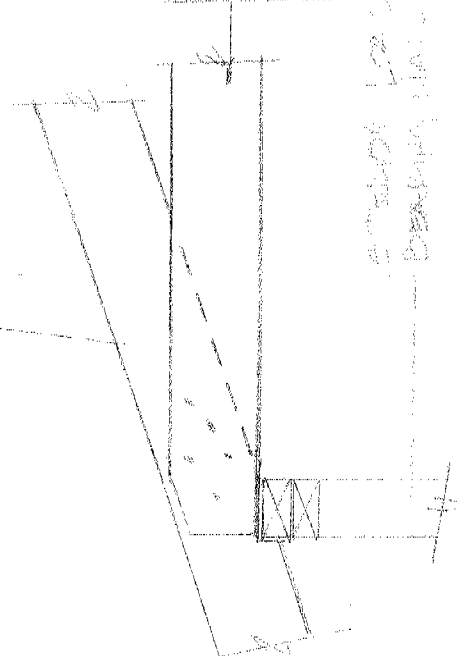
1/4" x 1/2" MESSINA
 BRACE SURFACES FROM
 2x4 GIRDERS ATTACHED
 TO PIPE SURFACE
 MESSINA W/13L
 ATTACHED TO 2x4.

MESSINA CIVIL ENGINEERING





PARTIAL (EXIST.)



NEW 24" THICK
W/ 5-BARS (4M)
TO PARTIAL
(ANY STUDS TO
LAP A MIN. OF 4'
BEARING WALL HAVE 5-BARS (4M)
(A.I.S.)

2x6 TIE C PLATE

Calculation for the required section modulus and moment of inertia for simple span wood beams. Dead load(dl) and Live load(ll) are in pounds per square ft. Spans(l) and Tributary load length or spacing(sp) are in ft., Section moduli are in inches cubed and Moments of inertia are in inches to the 4th power. Allowable stress (Fy) is in lbs./sq.in. per 1997 U.B.C. Joist, modulus shape factor reduction and load modification are per U.B.C. Sec. 2504

MICROLAM PURLIN AND MICROLAM RIDGE SUPPORT BEAM OVER LIVING AREA

Loads:

Frame(2x6 rafters): fr := 1 skip sht: purl: 1 Plywood: ply := 1.5

Roofing: rl := 6 misc := .0 Ceiling: clg := 0

Total roof dead load: rdl := fr + pur + ply + rl + misc + clg

Beam weight: Wdl := 1.75 * $\frac{9.5}{144}$ * 35 Wdl := 4 Beam length: l := 12

Roof trib area per ft.: rta := 8

Total area for live load determination: rta * l = 112 Roof live load: ll := 20

Beam load for stress: wt := rta * (rdl + ll) + Wdl

Beam load for deflection wtδ := rta * (rdl * .5 + ll) + Wdl * .5

Live load duration factor Cd := 1.25

E := 1900000 Fb := 2600 * 1.25

S min. required = $\frac{(wt) \cdot l^2 \cdot 1.5}{Fb} = 18.8$

I min. required = $\frac{5 \cdot wt\delta \cdot (1-12)^4}{12 \cdot 384 \cdot E \cdot 1.25} = 109.2$

End reactions: R := wt * $\frac{l}{2}$ R := 116.3

Check 1-3/4"x9.5" Microlam purlin:

w := 1.75 d := 9.5 C := 1.5

S := C * w * $\frac{d^2}{6}$ I := w * $\frac{d^3}{12}$

S = 27 > 18.8 I = 125 > 109.2 therefore O.K.



CHECK OF WOOD HEADERS AND SUPPORT BEAMS.

Calculation for the required area, section modulus and moment of inertia for simple span wood beams. Dead load(dl) and Live load(ll) are in pounds per square ft., Spans(l) and Tributary load length or spacing(sp) are in ft., Areas are in sq.in., Section modulus are in inches cubed and Moments of inertia are in inches to the 4th power. Allowable stresses (fy),(Fb),(Fv) are lbs./sq.in. per 1997 U.B.C. (NOTE: When structures were built prior to 1994 code year use at the time or 1991 edition of the UBC will be used in lieu of no higher quality materials available at the time.)

4X12 DOUGLAS FIR NO. 2 GARAGE DOOR HEADER

Framing dead load: fdr := 1 Exist. shtg. dead load: skip := 1 New shtg. dead load: shtg := 0

Roofing dead load: rfdl := 6 Ceiling dead load: clg := 0 Misc. dead load: msl := 0

$rdl := fdr + skip + shtg + rfdl + clg + msl$

$rdl = 9.5$

$rlf := 14$

$ra := 13.75$

$l := 15.25$

$fdl := 9$

$flf := 40$

$fla := \frac{0}{2}$

$ra-f = 209.7$

$wt := (ra \cdot (rdl + rlf) + flf \cdot (fdl + flf)) + 10$

$Cd := 1.25$

$wt = 333.1$

$Fb := 1250$

$Fbp := Fb \cdot Cd$

$Fbp = 1562.5$ $Pwt = 1700000$

$l \cdot wt = 5080.5$

A min. required =

$\frac{Pwt}{Fbp} = \frac{1700000}{1562.5} = 32.1$

S min. required =

$\frac{1.5 \cdot wt \cdot l}{Fbp} = \frac{1.5 \cdot 333.1 \cdot 15.25}{1562.5} = 74.4$

I min. required =

$\frac{5 \cdot wt \cdot l^3}{12 \cdot Fbp} = \frac{5 \cdot 333.1 \cdot 15.25^3}{12 \cdot 1562.5} = 312.7$

Check Beam properties:

$CF := \frac{12}{d} \cdot \frac{1}{9}$

$A := w \cdot d$

$w := 3.5$

$d := 11.25$

$S := w \cdot CF \cdot \frac{d^2}{6}$

$I := w \cdot \frac{d^3}{12}$

$Stiffw := I \cdot Ew$

$A = 39.4$

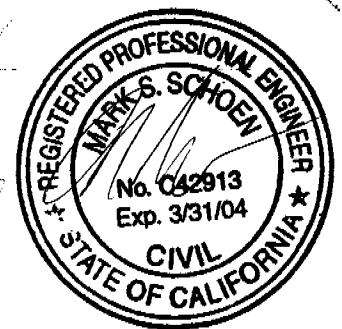
$S = 32.1$

$S = 74.4$

$S = 74.4$

$I = 415.3$

therefore ok



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February 10, 2003

Frank Pisi
6806 Southland Park Drive
Sacramento, CA 95831

SUBJECT: Reroof at 6806 Southland Park Drive, Sacramento, CA 95831

Frank:

This letter is an addendum to my report of January 23rd 2003. In regards the following modification specification:

* In the garage the rafter pair out 4' from the firewall ~~should have a~~ 2x6 tie installed at the plate level. The tie should be attached to the rafters with 5-16d nails. If it is spliced the splice should lap 4' and be attached with 5-16d nails(see attached sketch for details and plan for location).

It has been brought to my attention that this modification cannot be made in the exact manner specified because a longitudinal strong back in the garage would prevent placement of the tie directly on the top plates. In this case the tie may be installed so that it passes directly above the strong back as the original ties do.

I would like to thank you for allowing me to be of service in this matter. Please let me know if I can be of further assistance.

Sincerely

A handwritten signature in black ink, appearing to read "Mark S. Schoen".

Mark Schoen P.E.