

CITY OF SACRAMENTO

1231 I Street, Sacramento, CA 95814

Permit No: **0106291**

Insp Area: 2
Thos Bros: 297D6

Site Address: **1400 X ST SAC**
Parcel No: 009-0254-007

Sub-Type: NCOM
Housing (Y/N): N

CONTRACTOR

OWNER
O K INVESTMENT CO
8615 ELDER CREEK RD
SACRAMENTO CA 95828

ARCHITECT
BUZZ OATES ENTERPRISES
8615 ELDER CREEK RD
SACRAMENTO, CA 95828

Nature of Work: NEW SHELL BUILDING:OFFICE/RETAIL (9770 SQ FT)

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class _____ License Number 381 3600 Date _____ Contractor Signature _____

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. _____ B & PC for this reason: _____

Date 14 Dec 01 Owner Signature [Signature] **NEIGHBORHOODS PLANNING AND DEVELOPMENT SERVICES**

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and herby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 14 Dec 01 Applicant/Agent Signature [Signature]

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier _____ Policy Number _____ Exp Date _____

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 14 Dec 01 Applicant Signature [Signature]

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

Date of Request: 16 May 01
By: Mike Peters

CITY OF SACRAMENTO DEVELOPMENT SERVICES DIVISION
PLANNING AND ZONING INFORMATION REQUEST

Project Address: 14 1/2 St Spec Shell: (1400 x 50)

Assessor's Parcel Number: 009-0254-007

Previous Use: Empty Lot 009-0254-007

Description of Request/Proposed Use: C-2 Commercial

→ Plan Review - DR00-211

CONSTRUCT 9770 SQFT BLDG RETAIL/OFFICE

Is This a Change of Use? ~~NO~~ YES

Prior Applications for Project Site(P#, Z#, DRPB#): DR00-211 Zoning Designation: C-2

Comments: ~~MINOR~~ 24 PARKING REQUIRED. 21 PROPOSED
MEETS REQUIRED SETBACKS

Are There Any Planning Issues?: (circle one) YES NO

* Staff Site Plan Check Required? (Circle one) YES NO COMMERCIAL

* Field Inspection Required? (Circle one) YES NO

* Design Review/Preservation Required?: (Circle one) YES NO DR00-211 (ATTACHED)

Planning Review by/Date: [Signature] 5-17-2001

A list of items that must be reviewed by Planning is provided on the reverse side of this form.

MICROFILM AFTER FINAL

OWNER-BUILDER VERIFICATION

ATTENTION PROPERTY OWNERS

An owner-builder building permit has been applied for in your name and bearing your signature.

Please complete and return this information in the envelope provided at your earliest opportunity to avoid unnecessary delay in processing and issuing your building permit. No building permit will be issued until this verification is received.

1. I personally plan to provide the major labor and materials for construction of the proposed Improvement (yes or no) _____
2. I (have/have not) _____ signed an application for A building permit for the proposed work.

3. I have contracted with the following person (firm) to provide the proposed construction:

Name TBA Address _____

City _____ Telephone _____

Contractors License No. _____

4. I plan to provide portions of the work, but I have hired the following person to coordinate, Supervise, and provide the major work.

Name _____ Address _____

City _____ Telephone _____

Contractors License No. _____

5. I will provide some of the work but I have contracted (hired) the following to provide the Work indicated:

Name	Address	Phone	Type of work

Signed [Signature]

Job Address 1400 X ST

Permit No: 0106291

Elm Group Unified

0106291

Certification of Compliance School District Development Fees

(Print or Type) If Printing, press hard for four copies

PART I To be completed by APPLICANT (MUST BE COMPLETED)

OWNER'S NAME Randy Calver, Inc.
 OWNER'S ADDRESS 4615 S. 1st. Creek Rd. State, 95826
 PROJECT ADDRESS 6002 99th St
 PARCEL NUMBER 062-0000-098 LOT NO. —
 SUBDIVISION NAME —
 NUMBER OF UNITS 1

Upon payment of the fees listed below, a 90-day approval period commences upon which the applicant paying the fees may protest such fees. Any failure to file such protest within the 90-day period shall result in forfeiture of any rights to challenge such fees, through litigation or otherwise.

APPLICANT'S SIGNATURE [Signature]
 TITLE OF APPLICANT [Title]
 DATE 2 PHONE NUMBER 319-307

PART II To be completed by BUILDING DEPARTMENT

PLAN IDENTIFICATION NUMBER 0104811
 BUILDING TYPE
 RESIDENTIAL () APARTMENT/CONDOMINIUM () COMMERCIAL/INDUSTRIAL (✓)
 SQUARE FEET OF CHARGEABLE BUILDING AREA 129,000 #
 SIGNATURE Dick Maynard
 TITLE Building Tech DATE 7-20-01

PART III To be completed by SCHOOL DISTRICT

SCHOOL DISTRICT 1-019
 DISTRICT CERTIFICATION NO. 2778
 EXEMPT _____ COMMENTS _____

RESIDENTIAL/APT/CONDO	SQ FT X \$	<u>22</u>	= \$
COMMERCIAL/INDUSTRIAL	SQ FT X \$	<u>129,000</u>	= \$ <u>42,570.00</u>
OTHER FEE _____ TYPE _____	SQ FT X \$		= \$
TOTAL FEES COLLECTED			= \$

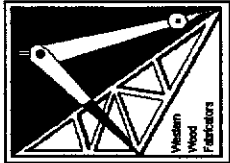
This Certification covers only the amount of square footage indicated above. Any additions or corrections to the square footage for this project will require an amendment to the Certificate of Compliance.

As the authorized school district official, I hereby certify that the requirements of Government Code Section 65995 and any other authorized requirements have been complied with by the above signed applicant.

AUTHORIZED SCHOOL DISTRICT OFFICIAL

SIGNATURE [Signature]
 TITLE _____ DATE 7/20/01

Original: School District 1st copy: School District 2nd copy: Building Department 3rd copy: Applicant



WESTERN WOOD FABRICATORS
 3700 Riego Rd., Elverta, CA 95626
 PH (916) 991-4400
 FAX (916) 991-4461

TRUSS ENGINEERING DRAWING PACKAGE
 PERMIT ASSISTANCE

ISSUED
 APR 09 2002

MAR 23 2002

Plan **RECEIVED**

Sacramento Building Division
 1400 X St

Customer: **Sequoia Pacific Builders** Project: **Fedor V** Submittal No. **1** Date: **2/5/2002** Job No. **2-058**

NOTES:

- ALL LATERAL BRACING SPECIFIED IS FOR BRACING COMPRESSION WEB MEMBERS, AND MUST BE INSTALLED. TOP CHORDS ARE ASSUMED TO BE LATERALLY RESTRAINED BY PLYWOOD, OR SPACED SHEATHING. WHERE NO RIGID CEILING IS APPLIED DIRECTLY TO THE BOTTOM CHORDS, IT SHALL BE BRACED AT INTERVAL OF NOT EXCEEDING 10'-0".
- VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.
- ALL FLOOR TRUSSES RECOMMEND 2X6 STRONGBACKS AT 10'-0" O.C.
- DUE TO THE VARIATIONS IN WEATHER, LUMBER DIMENSIONS AND MOISTURE CONTENT, AT THE TIME OF FABRICATION, WESTERN WOOD FABRICATORS CANNOT BE RESPONSIBLE FOR ANY TRUSS DIMENSION VARIANCE OF + - 1/4" OR LESS

DO NOT CUT OR ALTER TRUSSES



John Tange
WESTERN WOOD
 CERTIFIED INSPECTION
 IN STRICT ACCORDANCE
 WITH U.B.C. 2311.6
 PREFABRICATED

Reviewed
 3/15/02

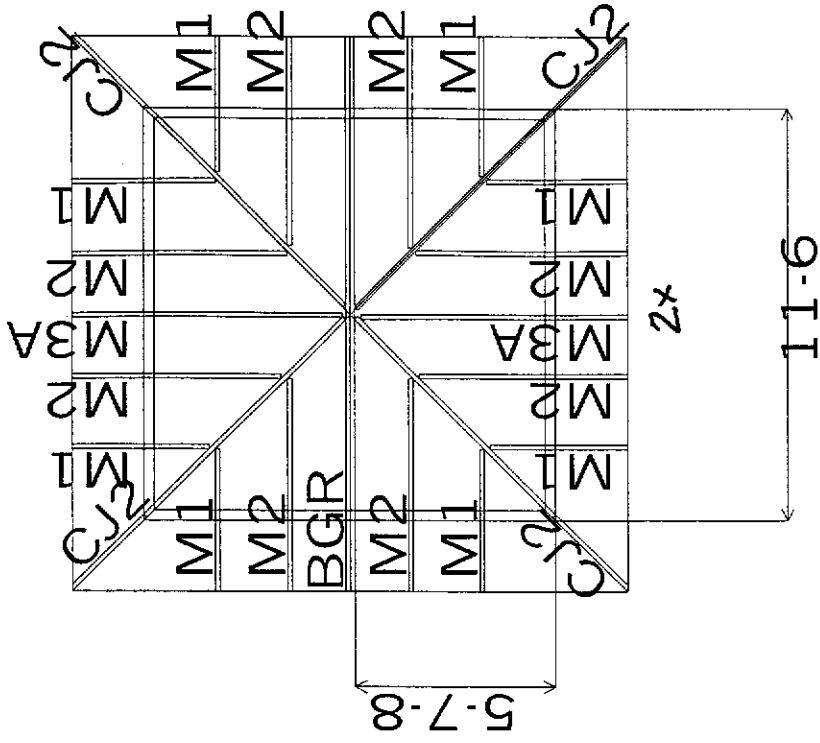
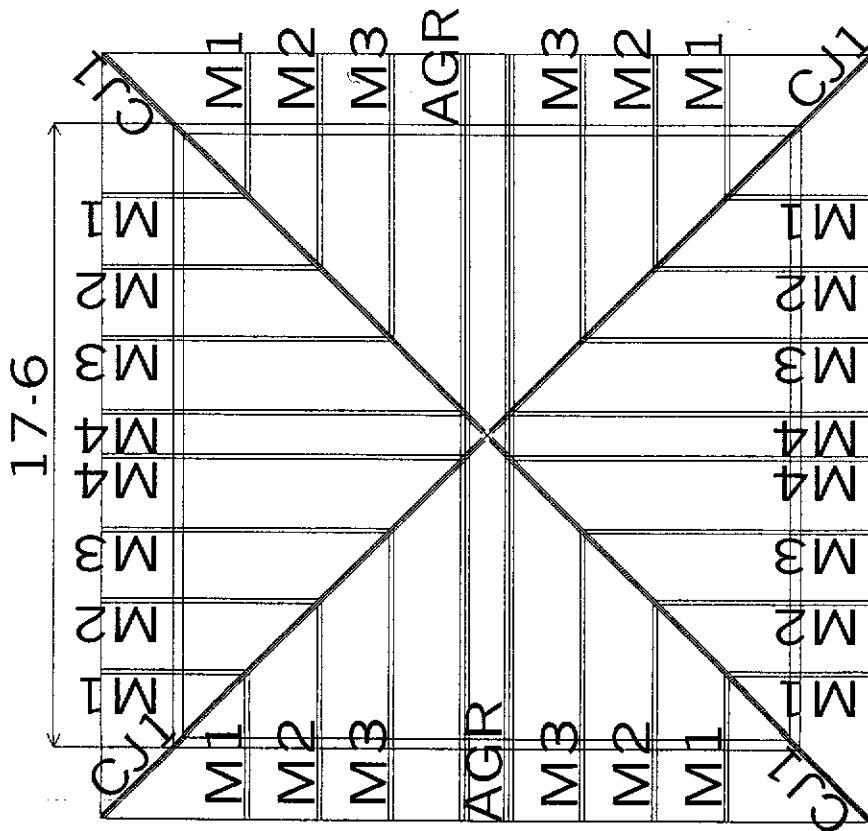
HANGER SCHEDULE

Project:

J#2058 1400X ST. BLDG.

TRUSS ID	Ply	Lbs LOAD	BC Size	Carrier TRUSS ID	HANGER ID	QTY
M1,2,3	1	644	TO 6	CJ1,CJ2	SUR/L26	
M3A,CJ2	1	3,015	TO 8	BGR	MTHM	
M4,CJ1	2	2,460	TO 8	AGR	MTHM-2	
			TO 10			
			TO 10			
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Note:
 - The contractors are responsible for all hanger installation, which is specified in Simpson hanger catalog.
 - All hangers above must be manufactured by SIMPSON.



J#2058
1400 X ST BLDG-SACRAMENTO

Job Name: 1400 X ST BLDG-SACRAMENTO

Truss ID: AGR

Dwg: AGR

BRG	X-LOC	REACT	SIZE	REQ'D
1	0-1-12	1963	3.50"	1.50"
2	17-4-4	1951	3.50"	1.50"

TC	FORCE	AXL	BND	CSI
1-2	-5658	0.08	0.16	0.24
2-3	-5895	0.24	0.00	0.24
3-4	-5901	0.27	0.00	0.27
4-5	-5786	0.07	0.06	0.13
5-6	-5674	0.08	0.16	0.24

BC	FORCE	AXL	BND	CSI
7-8	5457	0.29	0.25	0.54
8-9	5473	0.29	0.24	0.54
9-10	5902	0.31	0.33	0.64
10-11	5476	0.29	0.25	0.54
11-12	5473	0.29	0.25	0.54

WEB	FORCE	CSI	WEB	FORCE	CSI
2-8	-485	0.03	4-10	1211	0.25
2-9	461	0.09	5-10	364	0.07
3-9	1296	0.26	5-11	-394	0.02
3-10	331	0.07			

TC 2x4 DFL #1
 BC 2x8 DFL #2
 WEB 2x4 DFL STANDARD
 Nail pattern shown is for uniform loads only. Concentrated loads > 350# must be distributed (by others) equally to each ply unless nail clusters are shown ().
PLATING BASED ON GREEN LUMBER VALUES.
 ** This truss is designed for 400 lbs. of moving load at any point along the BOTTOM Chord.

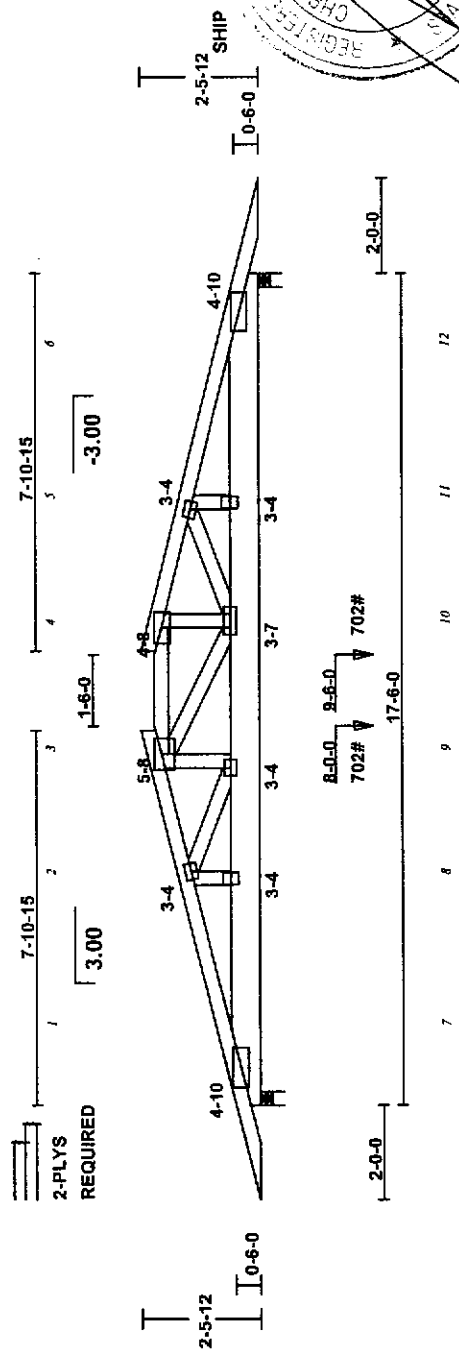
Plating spec: ANSITPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 PLATE VALUES PER ICBO RESEARCH REPORT #1607.
 Drainage must be provided to avoid ponding. Permanent bracing is required (by others) to prevent rotation/toppling. See HB-91 and ANSITPI 1-1995; 10.3.4.5 and 10.3.4.6.
 2-PLY Nail w/10d BOX, staggered (NDS-97 Sect. 12) in: TC- 2/ft BC- 4/ft WEBS- 2/ft

LOAD CASE #1 DESIGN LOADS

Dir	L.PIF	L.Loc	R.PIF	R.Loc	LLTL
TC Vert	68.00	0-0-0	68.00	17-6-0	0.99
BC Vert	20.00	0-0-0	20.00	8-0-0	0.00
BC Vert	152.00	8-0-0	152.00	9-6-0	0.39
BC Vert	20.00	9-6-0	20.00	17-6-0	0.00
Type...	lbs	X.Loc	LLTL		
BC Vert	319.0	8-0-0	1.00		
BC Vert	382.8	8-0-0	0.00		
BC Vert	319.0	9-6-0	1.00		
BC Vert	382.8	9-6-0	0.00		

MAX DEFLECTION (span):
 L/999 IN MEM 9-10 (LIVE)
 L = -0.09" D = -0.11" I = -0.21"

==== Joint Locations =====
 1 0-0-0 7 0-0-0
 2 4-9-11 8 4-9-11
 3 7-5-0 9 7-5-0
 4 10-1-0 10 10-1-0
 5 12-8-5 11 12-8-5
 6 17-6-0 12 17-6-0



All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge, positioned per Joint Report, unless noted.

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
 This design is for an individual building component not truss system. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer must ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code and the particular application. The design assumes that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood to exceed 19% and/or causes connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: JOINT DETAILS by Truswal, ANSITPI 1, WTCA 1 - Wood Truss Council of America Standard Design Responsibilities, HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES - (HIB-91) and HIB-91 SUMMARY SHEET by TPI. The Truss Plate Institute (TPI) is located at D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1111 19th Street, NW, Ste. 800, Washington, DC 20036.

WESTERN WOOD FABRICATORS
 3700 RIEGO ROAD, ELVERTA, CA 95626

Eng. Job: E.J.	JOB#: J2058
Chk: CM	TRUSS ID: AGR
Design: FV	DurFacs L=1.25 P=1.25
TC Live 20.00 psf	Rep Mbr Bnd 1.00
TC Dead 14.00 psf	O.C.Spacing 2-0-0
BC Live 0.00 psf	Design Spec UBC-97
BC Dead 10.00 psf	
TOTAL 44.00 psf	Seqn T6.2.6 - 50431

2/5/2002

Scale: 1/4" = 1'

Job Name: 1400 X ST BLDG-SACRAMENTO

Truss ID: BGR

Dwg: BGR

BRG	I-LOC	REACT	SIZE	REQ'D	
1	0-1-12	1210	3.50"	1.50"	
2	11-4-4	1192	3.50"	1.50"	
TC	FORCE	AXL	BND	CSI	
1-2	-2823	0.02	0.17	0.18	
2-3	-2824	0.02	0.17	0.19	
BC	FORCE	AXL	BND	CSI	
4-5	2709	0.14	0.21	0.35	
5-6	2709	0.13	0.21	0.34	
WEB	FORCE	CSI	WEB	FORCE	CSI
2-5	1043	0.21			

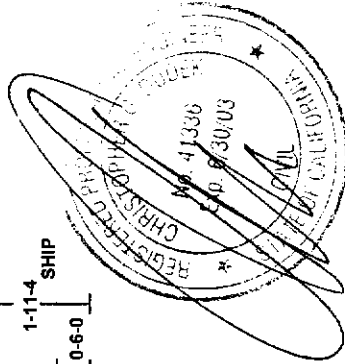
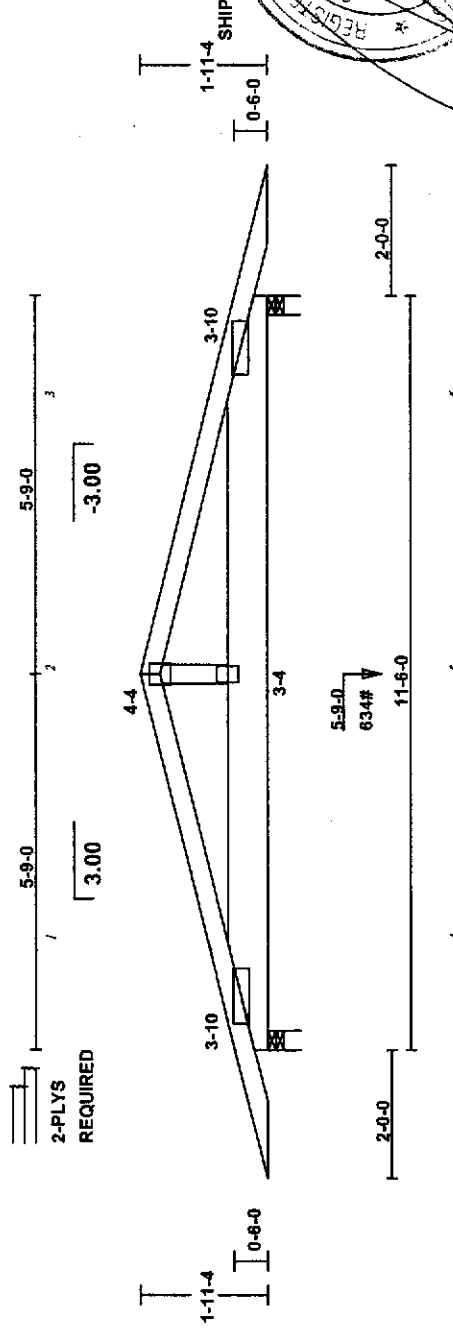
Plating spec: ANSIT/PI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 PLATE VALUES PER ICBO RESEARCH REPORT #1807.
 Permanent bracing is required (by others) to prevent rotation/toppling. See HIB-91 and ANSIT/PI 1-1995; 10.3.4.5 and 10.3.4.6.
 PLATING BASED ON GREEN LUMBER VALUES.
 ** This truss is designed for 400 lbs. of moving load at any point along the BOTTOM Chord.

LOAD CASE #1 DESIGN LOADS

	Dir	L/Plf	L/Loc	R/Plf	R/Loc	L/TL	L/TL
TC Vert	68.00	0-0-0	68.00	11-6-0	0.59		
BC Vert	20.00	0-0-0	20.00	5-9-0	0.00		
BC Vert	20.00	0-0-0	20.00	11-6-0	0.00		
..Type...	lbs	X/Loc	L/TL				
BC Vert	144.0	5-9-0	1.00				
BC Vert	172.8	5-9-0	0.00				
BC Vert	144.0	5-9-0	1.00				
BC Vert	172.8	5-9-0	0.00				

MAX DEFLECTION (span):
 U999 IN MEM 5-6 (LIVE)
 L = -0.03" D = -0.03" T = -0.06"

Joint Locations: =====
 1 0-0-0 4 0-0-0
 2 5-9-0 5 5-9-0
 3 11-6-0 6 11-6-0



2/5/2002

Scale: 11/32" = 1'

All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge, positioned per Joint Report, unless noted.

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
 This design is for an individual building component not truss system. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer must ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code and the particular application. The design assumes that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood to exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: JOINT DETAILS by Truswal, ANSIT/PI 1, WTC-A 1 - Wood Truss Council of America Standard Design Responsibilities, HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES - (HIB-91) and HIB-91 SUMMARY SHEET by TPI. The Truss Plate Institute (TPI) is located at D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1111 19th Street, NW, Ste 800, Washington, DC 20036.

Eng. Job: E.J.	JOB#: J2058
Chk: CM	Truss ID: BGR
Dsgnr: FV	Durfacs L=1.25 P=1.25
TC Live 20.00 psf	Rep Mbr Bnd 1.00
TC Dead 14.00 psf	O.C.Spacing 2-0-0
BC Live 0.00 psf	Design Spec UBC-97
BC Dead 10.00 psf	
TOTAL 44.00 psf	Seqn T6.2.5 - 50432

WESTERN WOOD FABRICATORS
 3700 RIEGO ROAD, ELVERTA, CA 95828

Job Name: 1400 X ST BLDG-SACRAMENTO

Truss ID: CJ1

Dwg: CJ1

BRG	X-LOC	REACT SIZE	REQ'D
1	0-2-7	1429 4.86"	1.50"
2	11-1-5	1289 3.50"	1.50"

TC	FORCE	AXL	BND	CSI
1-2	-3801	0.03	0.15	0.18
2-3	-3579	0.03	0.08	0.11
3-4	-26	0.00	0.08	0.08
4-0	-3	0.00	0.00	0.00

BC	FORCE	AXL	BND	CSI
5-6	3723	0.23	0.25	0.49
6-7	3713	0.23	0.19	0.42
7-8	2446	0.13	0.17	0.30
8-9	2491	0.16	0.07	0.23
9-0	0	0.00	0.00	0.00

WEB	FORCE	CSI	WEB	FORCE	CSI
2-6	385	0.08	3-8	708	0.14
2-7	-637	0.04	3-9	-2717	0.17
3-7	1798	0.37	4-9	-1119	0.01

TC 2x4 DFL #1
 BC 2x6 DFL #2
 WEB 2x4 DFL STANDARD
 Nail pattern shown is for uniform loads only. Concentrated loads > 350# must be distributed (by clusters) equally to each ply, unless nail clusters are shown ().
 2-PLY Nail w/10d BOX, staggered (NDS-97 Sect. 12) in. TC-2/ft BC-3/ft WEBS-2/ft

Plating spec: ANS/ITPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES
 PLATE VALUES PER TCBO RESEARCH REPORT #1807.
 Permanent bracing is required (by others) to prevent rotation/flopping. See HIB-91 and ANS/ITPI 1-1995; 10.3.4.5 and 10.3.4.6.
 PLATING BASED ON GREEN LUMBER VALUES.
 ** This truss is designed for 400 lbs. of moving load at any point along the BOTTOM Chord.

---LOAD CASE # DESIGN LOADS---

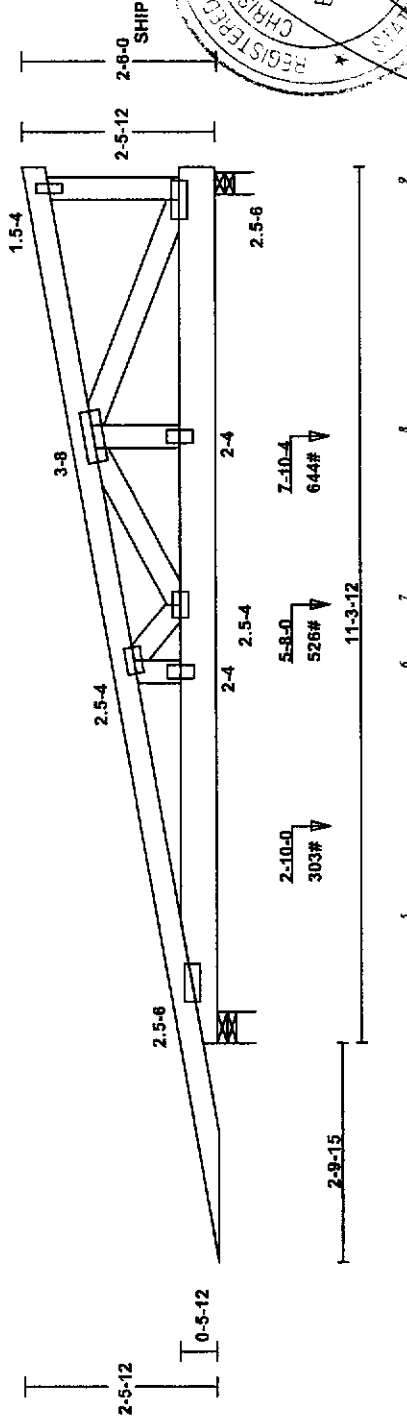
Dir	LPH	LLOC	RPH	RLOC	LL/TL
TC Vert	68.00	0-0-0	68.00	11-3-12	0.59
BC Vert	20.00	0-0-0	20.00	11-3-12	0.00
..Type..	lbs	X.Loc	LL/TL		
BC Vert	303.0	2-10-0	0.45		
BC Vert	526.0	5-8-0	0.45		
BC Vert	644.0	7-10-4	0.45		

MAX DEFLECTION (span):
 L/999 IN MEM 5-6 (LIVE)
 L = 0.04" D = 0.05" T = -0.09"
 MAX DEFLECTION (cant):
 L/999 IN MEM 9-0 (LIVE)
 L = 0.00" D = 0.00" T = 0.00"

==== Joint Locations =====

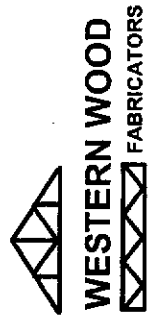
1	0-0-0	6	4-9-12
2	4-9-12	7	5-8-0
3	7-10-4	8	7-10-4
4	11-3-12	9	11-3-12
5	0-0-0		

2-PLYS
 REQUIRED



All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge, positioned per Joint Report, unless noted.

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3700 RIEGO ROAD, ELVERTA, CA 95626

Eng. Job: E.J.
 Chk: CM
 Dsgnr: FV

	TC Live	20.00 psf
TC Dead	14.00 psf	
BC Live	0.00 psf	
BC Dead	10.00 psf	
TOTAL	44.00 psf	

JOB#: J2058
 Truss ID: CJ1

DurFacs	L=1.25	P=1.25
Rep Mbr Bnd	1.00	
O.C.Spacing	2-0-0	
Design Spec	UBC-97	
Seqn	T6.2.6	50421

2/ 5/2002

Scale: 1/32" = 1'

Job Name: 1400 X ST BLDG-SACRAMENTO

Truss ID: CJ2

Dwg: CJ2

BRG	X-LOC	REACT	SIZE	REQ'D	
1	0-2-7	1038	4.86*	1.50*	
2	7-11-2	1197	3.50*	1.50*	
TC	FORCE	AXL	END	CSI	
1-2	-2372	0.05	0.28	0.32	
2-3	-1800	0.03	0.15	0.18	
3-0	-3	0.00	0.00	0.01	
BC	FORCE	AXL	END	CSI	
4-5	2218	0.27	0.47	0.74	
5-6	2172	0.27	0.23	0.50	
6-7	42	0.00	0.28	0.28	
7-0	0	0.00	0.00	0.00	
WEB	FORCE	CSI	WEB	FORCE	CSI
2-5	561	0.23	3-6	2072	0.84
2-6	-887	0.10	3-7	-972	0.12

TC 2x4 DFL #1
 BC 2x6 DFL #2
 WEB 2x4 DFL STANDARD
 Permanent bracing is required (by others) to prevent rotation/toppling. See HB-91 and ANSI/TPI 1-1995; 10.3.4.5 and 10.3.4.8.
 ** This truss is designed for 400 lbs. of moving load at any point along the BOTTOM Chord.
 ++++++

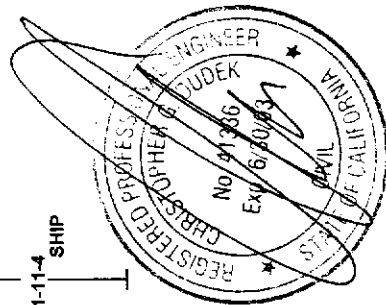
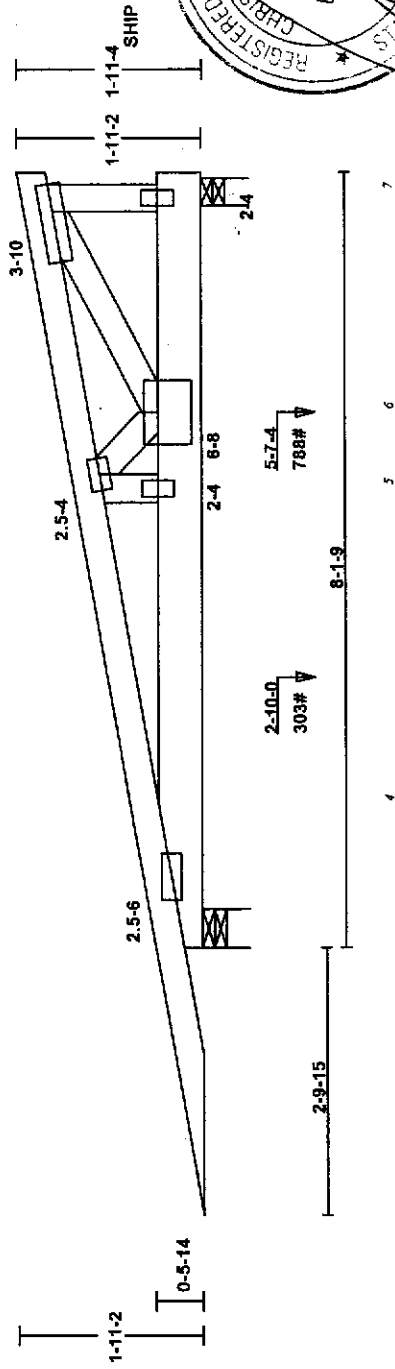
Plating spec: ANSITPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 PLATING VALUES PER ICBO RESEARCH REPORT #1607.
 NOTE: LEFT OVERHANG DOES NOT WORK WITHOUT ADDITIONAL SUPPORT UNLESS OTHER PROVISIONS ARE MADE TO REDUCE O.H. STRESS/DEFLECTION.
 Maximum Unsupported Overhang
 TC Left is 2'-4"

LOAD CASE #1 DESIGN LOADS

Dir	L.Pf	L.Loc	R.Pf	R.Loc	LL/TL
TC Vert	68.00	0-0-0	68.00	8-1-9	0.09
BC Vert	20.00	0-0-0	20.00	8-1-9	0.00
..Type...	lbs	X.Loc	LL/TL		
BC Vert	303.0	2-10-0	0.45		
BC Vert	526.0	5-7-4	0.45		
BC Vert	262.0	5-7-4	0.45		

MAX DEFLECTION (span):
 L/999 IN MEM 4.5 (LIVE)
 L = -0.04" D = -0.04" T = -0.08"
 MAX DEFLECTION (cant):
 L/999 IN MEM 7.0 (LIVE)
 L = 0.00" D = 0.00" T = 0.01"
 =====
 Joint Locations =====
 1 0-0-0 5 4-9-13
 2 4-9-13 6 5-7-4
 3 8-1-9 7 8-1-9
 4 0-0-0

2.12



2/5/2002

Scale: 1/2" = 1'

All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge, positioned per Joint Report, unless noted.

WESTERN WOOD FABRICATORS
 3700 RIEGO ROAD, ELVERTA, CA 95626

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Eng. Job: E.J.	TC Live	20.00 psf
Chk: CM	TC Dead	14.00 psf
Dsgnr: FV	BC Live	0.00 psf
	BC Dead	10.00 psf
	TOTAL	44.00 psf

JOB#: J2058	DurFacs	L=1.25 P=1.25
Truss ID: CJ2	Rep Mbr Bnd	1.00
	O.C.Spacing	2-0-0
	Design Spec	UBC-97
	Seqn	T6.2.6 - 50422

Job Name: 1400 X ST BLDG-SACRAMENTO

Truss ID: M1

Drwg: M1

BRG	X-LOC	REACT	SIZE	REQ'D
1	0-1-12	413	3-50*	1-50*
2	1-11-4	84	1-50*	1-50*
3	1-11-4	303	1-50*	1-50*

TC	FORCE	AXL	BND	CSI
1-2	19	0.00	0.09	0.09
2-0	-1	0.00	0.00	0.00

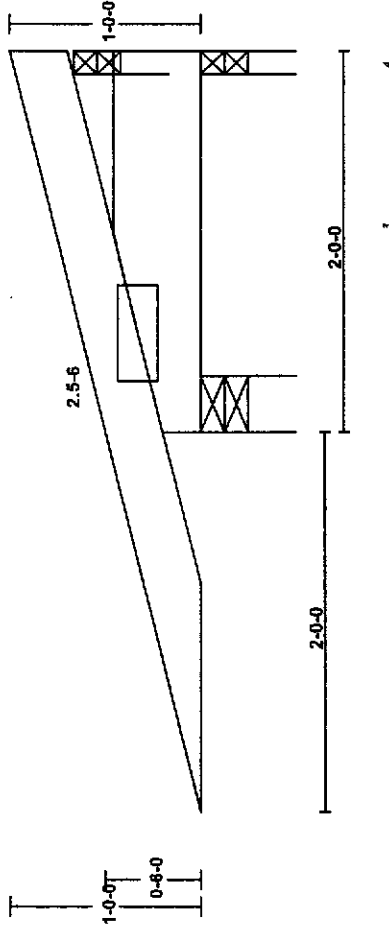
BC	FORCE	AXL	BND	CSI
3-4	0	0.00	0.10	0.10
4-0	0	0.00	0.00	0.00

TC 2x4 DFL #1
 BC 2x6 DFL #2
 PLATE VALUES PER ICBO RESEARCH REPORT #1607.
 Interior support or temporary shoring must be in place before erecting this truss.
 PLATING BASED ON GREEN LUMBER VALUES.

Plating spec.: ANSIT/PI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Location of interior bearings should be clearly marked on each truss.
 Shim bearings (if needed) for req. support.

 ** This truss is designed for 400 lbs. of moving load at any point along the BOTTOM Chord.

MAX DEFLECTION (span) :
 L/999 IN MEM 3-4 (LIVE)
 L = 0.00" D = 0.00" T = 0.00"
 ***** Joint Locations *****
 1 0-0-0 3 0-0-0
 2 2-0-0 4 2-0-0



2/5/2002

Scale: 0/32" = 1'

OVER 3 SUPPORTS

WESTERN WOOD FABRICATORS
 3700 RIEGO ROAD, ELVERTA, CA 95628

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Eng. Job: E.J.
 Chk: CM
 Dsgnr: FV

TC Live	20.00 psf
TC Dead	14.00 psf
BC Live	0.00 psf
BC Dead	10.00 psf
TOTAL	44.00 psf

JOB#: J2058
Truss ID: M1
 Durfacs L=1.25 P=1.25
 Rep Mbr Bnd 1.00
 O.C.Spacing 2-0-0
 Design Spec UBC-97
 Segn T6.2.6 - 50423

Job Name: 1400 X ST BLDG-SACRAMENTO

Truss ID: M2

Drwg: M2

BRG	X-LOC	REACT	SIZE	REQ'D
1	0-1-12	543	3-50"	1-50"
2	3-11-4	526	1-50"	1-50"

TC	FORCE	AXL	BND	CSI
1-2	31	0.00	0.25	0.25
2-3	-21	0.00	0.01	0.01
3-0	-5	0.00	0.01	0.01

BC	FORCE	AXL	BND	CSI
4-5	0	0.00	0.41	0.41
5-6	0	0.00	0.12	0.12
6-0	0	0.00	0.00	0.00

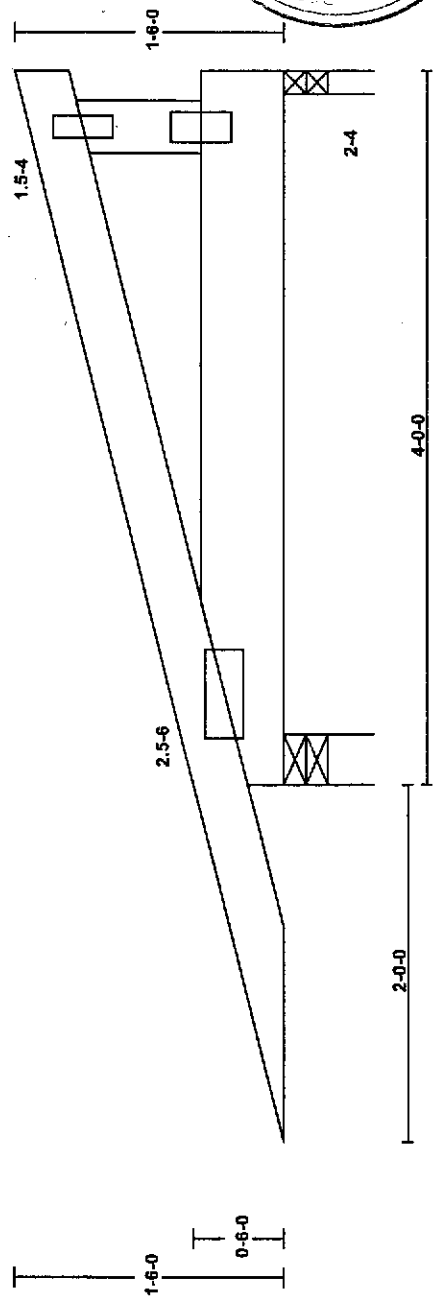
WEB	FORCE	CSI	WEB	FORCE	CSI
2-5	-149	0.02			

TC 2x4 DFL #1
BC 2x6 DFL #2
WEB 2x4 DFL STANDARD
PLATING BASED ON GREEN LUMBER VALUES.

Plating spec: ANS/TPI - 1995
THIS DESIGN IS THE COMPOSITE RESULT OF
MULTIPLE LOAD CASES.
PLATE VALUES PER ICBO RESEARCH REPORT #1607.

** This truss is designed for 400 lbs. of
moving load at any point along the
BOTTOM Chord.

MAX DEFLECTION (span):
L/999 IN MEMB 4-5 (LIVE)
L = -0.01" D= -0.02" T= -0.03"
MAX DEFLECTION (cant):
L/999 IN MEMB 6-0 (LIVE)
L = 0.00" D= 0.00" T= 0.00"
***** Joint Locations *****
1 0-0-0 4 0-0-0
2 3-8-4 5 3-8-4
3 4-0-0 6 4-0-0



2/5/2002

Scale: 15/16" = 1'

Eng. Job: E.U.	JOB#: J2058
Chk: CM	Truss ID: M2
Dsgnr: FV	DurFacs L=1.25 P=1.25
TC Live 20.00 psf	Rep Mbr Bnd 1.00
TC Dead 14.00 psf	O.C.Spacing 2-0-0
BC Live 0.00 psf	Design Spec UBC-97
BC Dead 10.00 psf	Seqn T6.2.6 - 50424
TOTAL 44.00 psf	

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WESTERN WOOD FABRICATORS
3700 RIEGO ROAD, ELVERTA, CA 95828

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Job Name: 1400 X ST BLDG-SACRAMENTO

Truss ID: M3

Drwg: M3

BRG	X-LOC	REACT	SIZE	REQ'D
1	0-1-12	645	3.50"	1.50"
2	5-11-11	644	1.50"	1.50"

TC	FORCE	AXL	RND	CSI
1-2	43	0.00	0.47	0.47
2-3	-21	0.00	0.01	0.01
3-0	-5	0.00	0.01	0.01

BC	FORCE	AXL	RND	CSI
4-5	0	0.00	0.75	0.75
5-6	0	0.00	0.16	0.16
6-0	0	0.00	0.00	0.00

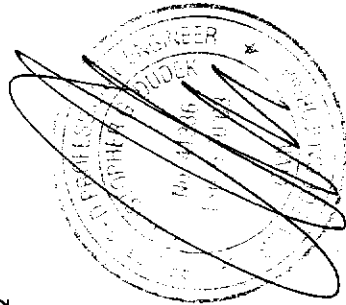
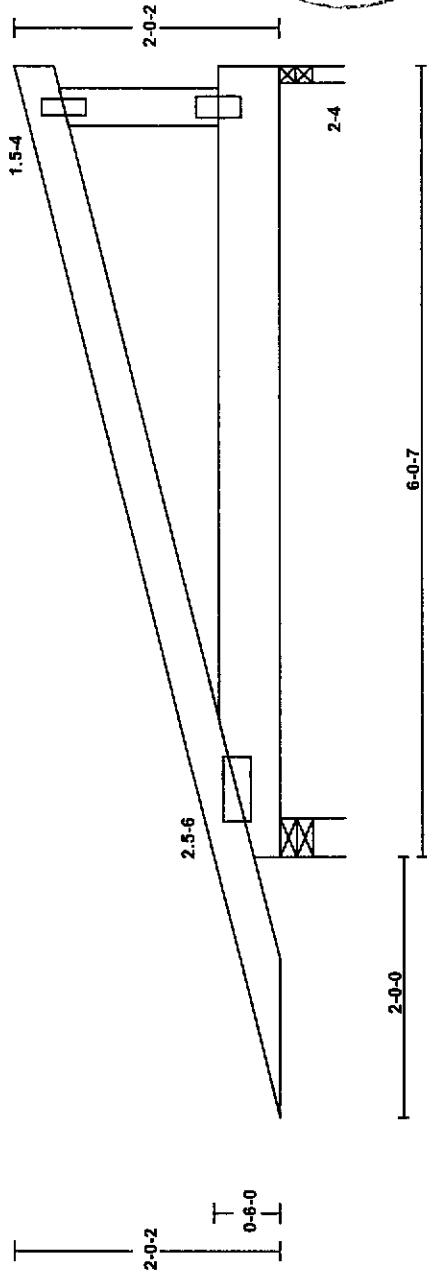
WEB	FORCE	CSI	WEB	FORCE	CSI
2-5	-196	0.02			

TC 2x4 DFL #1
BC 2x4 DFL #2
WEB 2x4 DFL STANDARD
PLATING BASED ON GREEN LUMBER VALUES.

Plating spec: ANS/ITPI - 1995
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
PLATE VALUES PER ICBO RESEARCH REPORT #1607.
** This truss is designed for 400 lbs. of moving load at any point along the BOTTOM Chord.

MAX DEFLECTION (span):
L/999 IN MEM 4-5 (LIVE)
L = 0.05" D = 0.07" T = 0.12"
MAX DEFLECTION (cant):
L/999 IN MEM 6-0 (LIVE)
L = 0.00" D = 0.00" T = 0.00"

Joint Locations
1 0-0-0 4 0-0-0
2 5-8-11 5 5-8-11
3 6-0-7 6 6-0-7



2/5/2002

Scale: 11/16" = 1'

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WESTERN WOOD FABRICATORS
3700 RIEGO ROAD, ELVERTA, CA 95628

Eng. Job: E.J. Chk: CM Dgnr: FV	JOB#: J2058 Truss ID: M3	Durfacs L=1.25 P=1.25 Rep Mbr Bnd 1.00 O.C.Spacing 2-0-0 Design Spec UBC-97 Segn T6.2.6 - 50425
TC Live 20.00 psf TC Dead 14.00 psf BC Live 0.00 psf BC Dead 10.00 psf TOTAL 44.00 psf		

Job Name: 1400 X ST BLDG-SACRAMENTO

Truss ID: M3A

Drwg: M3A

BRG	X-LOC	REACT	SIZE	REQ'D	TC	2x4 DFL #1
1	0- 1-12	621	3-50"	1-50"	BC	2x4 DFL #2
2	5- 5-12	621	3-50"	1-50"	WEB	2x4 DFL STANDARD

PLATING BASED ON GREEN LUMBER VALUES.

TC	FORCE	AXL	BND	CSI
1-2	-52	0.00	0.41	0.41
2-0	-4	0.00	0.00	0.01

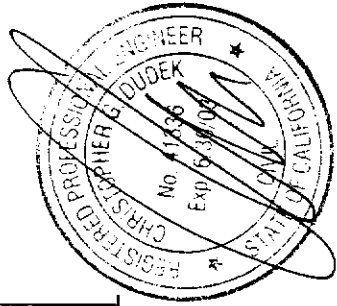
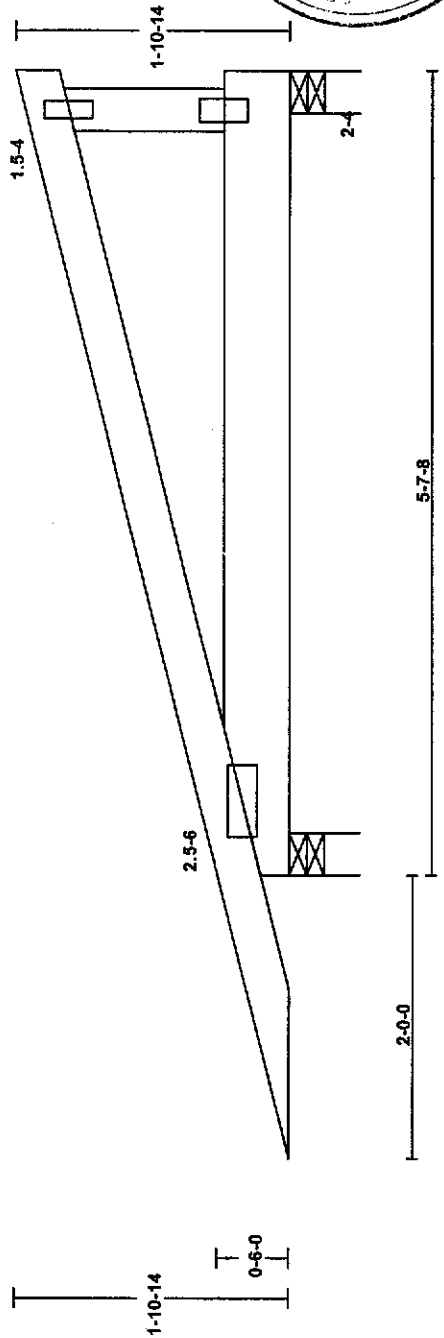
BC	FORCE	AXL	BND	CSI
3-4	15	0.00	0.64	0.65
4-0	0	0.00	0.00	0.00

WEB	FORCE	CSI	WEB	FORCE	CSI
2-4	-179	0.02			

Plating spec: ANSI/TPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 PLATE VALUES PER ICBO RESEARCH REPORT #1807.

 ** This truss is designed for 400 lbs. of
 moving load at any point along the
 BOTTOM Chord.

MAX DEFLECTION (span):
 L/999 IN MEM 3-4 (LIVE)
 L= -0.04" D= -0.05" T= -0.08"
 MAX DEFLECTION (cant):
 L/999 IN MEM 4-0 (LIVE)
 L= 0.00" D= 0.00" T= 0.01"
 ***** Joint Locations *****
 1 0-0-0 3 0-0-0
 2 5-7-8 4 5-7-8



21 5/2002

Scale: 3/4" = 1'

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WESTERN WOOD FABRICATORS
 3700 INEGO ROAD, ELVERTA, CA 95626

Eng. Job: E.J.	JOB#: J2058
Chk: CM	Truss ID: M3A
Dsgnr: FV	DurFacs L=1.25 P=1.25
TC Live 20.00 psf	Rep Mbr Bnd 1.00
TC Dead 14.00 psf	O.C.Spacing 2-0-0
BC Live 0.00 psf	Design Spec UBC-97
BC Dead 10.00 psf	Seqn T6.2.6 - 50426
TOTAL 44.00 psf	

Job Name: 1400 X ST BLDG-SACRAMENTO

Truss ID: M4

Dwg: M4

REQ	I-LOC	REACT	SIZE	BRG'D
1	0-1-12	731	3.50"	1.50"
2	8-0-7	-367	1.50"	1.50"
3	7-11-4	1170	1.50"	1.50"

TC	FORCE	AXL	END	CSI
1-2	62	0.00	0.66	0.66
2-3	-549	0.00	0.39	0.39
3-0	-133	0.00	0.39	0.39
0-0	-1	0.00	0.00	0.00

BC	FORCE	AXL	END	CSI
4-5	0	0.00	0.71	0.71
5-6	0	0.00	0.18	0.18
6-0	0	0.00	0.00	0.00

WEB	FORCE	CSI	WEB	FORCE	CSI
2-5	-804	0.10			

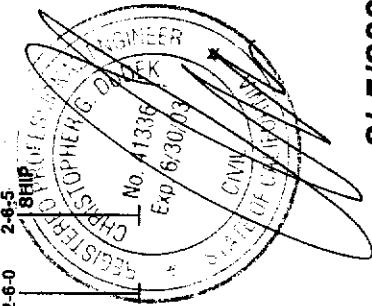
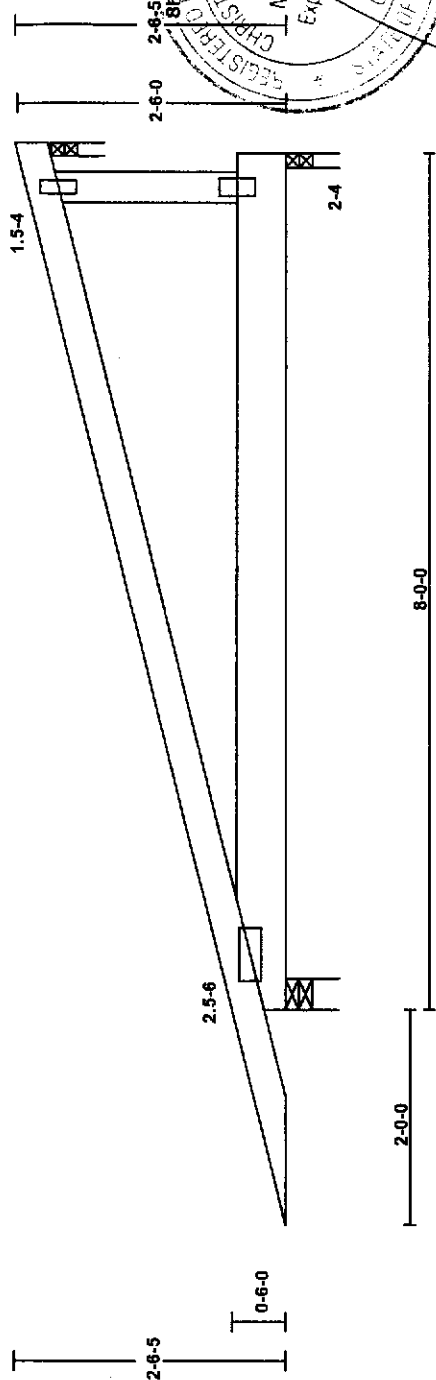
TC 2x4 DFL #1
 BC 2x6 DFL SS
 WEB 2x4 DFL SS
 Lumber shear allowances are per NDS-97.
 Interior support or temporary shoring must be in place before erecting this truss.
PLATING BASED ON GREEN LUMBER VALUES.

Plating spec: ANS/ITPI - 1985
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 PLATE VALUES PER ICBO RESEARCH REPORT #1607.
 Location of interior bearings should be clearly marked on each truss.
 Shim bearings (if needed) for req. support.

 ** This truss is designed for 400 lbs. of moving load at any point along the BOTTOM Chord.

UPLIFT REACTION(S):
 Support 2 -320 lb

MAX DEFLECTION (span) :
 L/744 IN MEM 4-5 (LIVE)
 L=-0.13" D=-0.15" T=-0.28"
 MAX DEFLECTION (cant) :
 L/999 IN MEM 6-0 (LIVE)
 L= 0.00" D= 0.00" T= 0.01"
 ===== Joint Locations =====
 1 0-0-0 4 0-0-0
 2 7-8-4 5 7-8-4
 3 8-0-0 6 8-0-0



21 5/2002

Scale: 9/16" = 1'

OVER 3 SUPPORTS

All plates are 20 gauge Trussal Connectors unless preceded by "18" for 18 gauge or "14" for 16 gauge, positioned per Joint Report, unless noted.

WESTERN WOOD FABRICATORS
 3700 RIEGO ROAD, ELVERTA, CA 95828

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
 This design is for an individual building component not truss system. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer must ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code and the particular application. The design assumes that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood to exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: 'JOINT DETAILS' by Trussal, ANS/ITPI 1, 'WTCA 1' - Wood Truss Council of America Standard Design Responsibilities, 'HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES' - (HIB-91) and 'HIB-91 SUMMARY SHEET' by TPI. The Truss Plate Institute (TPI) is located at D'Oonofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1111 19th Street, NW, Ste 800, Washington, DC 20036.

Eng. Job: .EJ.	TC Live 20.00 psf
Chk: CM	TC Dead 14.00 psf
Dsgnr: FV	BC Live 0.00 psf
	BC Dead 10.00 psf
	TOTAL 44.00 psf

JOB#: J2058	DurFacs L=1.25 P=1.25
Truss ID: M4	Rep Mbr Bnd 1.00
	O.C.Spacing 2-0-0
	Design Spec UBC-97
	Seqn T6.2.6 - 50427

City of Sac. School Dist:

01049W

Certificate of Compliance
School District Department Fee

Permit # 0106291

(Print or Type) If Printing, press hard for four copies

PART I To be completed by APPLICANT (MUST BE FILLED OUT COMPLETELY)

OWNER'S NAME Buzz City Co
OWNER'S ADDRESS 8615 Elder Creek Rd Smith 95828
PROJECT ADDRESS 1400 X Street
PARCEL NUMBER 009-0254-067 LOT NO. _____
SUBDIVISION NAME _____
NUMBER OF UNITS _____

Upon payment of the fees listed below, a 90-day approval period commences upon which the applicant paying the fees may protest such fees. Any failure to file such protest within the 90-day period shall result in forfeiture of any rights to challenge such fees, through litigation or otherwise.

APPLICANT'S SIGNATURE [Signature]
TITLE OF APPLICANT Deputy City Administrator
DATE 1 Oct 01 PHONE NUMBER 379-3842

PART II To be completed by BUILDING DEPARTMENT

PLAN IDENTIFICATION NUMBER 0106291
BUILDING TYPE
RESIDENTIAL () APARTMENT/CONDOMINIUM () COMMERCIAL/INDUSTRIAL (✓)
SQUARE FEET OF CHARGEABLE BUILDING AREA 9770
SIGNATURE [Signature]
TITLE Building Tech DATE 9-26-01

PART III To be completed by SCHOOL DISTRICT

SCHOOL DISTRICT 11110
DISTRICT CERTIFICATION NO. 72
EXEMPT _____ COMMENTS _____
RESIDENTIAL/APT/CONDO 9770 SQ FT X \$ 1.72 = \$ _____
COMMERCIAL/INDUSTRIAL ✓ SQ FT X \$.28 = \$ 2735.60
OTHER FEE _____ TYPE _____ SQ FT X \$ _____ = \$ _____
TOTAL FEES COLLECTED _____ = \$ 2,735.60

This Certification covers only the amount of square footage indicated above. Any additions or corrections to the square footage for this project will require an amendment to the Certificate of Compliance.

As the authorized school district official, I hereby certify that the requirements of Government Code Section 65995 and any other authorized requirements have been complied with by the above signed applicant.

AUTHORIZED SCHOOL DISTRICT OFFICIAL

SIGNATURE [Signature]
TITLE Deputy City Administrator DATE 10/4/01

Original: School District 1st copy: School District 2nd copy: Building Department 3rd copy: Applicant