

CITY OF SACRAMENTO

Permit No: 9901045

1231 I Street, Sacramento, CA 95814

Insp Area: 1

Site Address: 605 38TH ST SAC

Sub-Type: ASFR

Parcel No: 0040283027

Housing (Y/N): N

CONTRACTOR

DYER NEED CONSTRUCTION
1540 38TH ST
SACRAMENTO CA 95816

OWNER

CARLI MARIA/RICHARD CORBE
170 LAGOMARSINO WY
SACRAMENTO CA 95819

ARCHITECT

Nature of Work: SFD ADDITION 494 SQ ST REMODEL 994 SQ FT.DEMO OLD GARAGE & REBUILD IT SAME SIZE

CONSTRUCTION LENDING AGENCY: I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name Lender's Address

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class B License Number 645755 Date 2/10/99 Contractor Signature [Signature]

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. B & PC for this reason:

Date Owner Signature

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 2/10/99 Applicant/Agent Signature [Signature]

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier Policy Number Exp Date

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 2/10/99 Applicant Signature [Signature]

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

ROBERTSON ENGINEERING

8536 Elder Creek Rd., Sacramento, CA 95828
Phone: (916) 388-0866 Fax: (916) 388-0740

March 4, 1999

City of Sacramento
Building Department

Dear Sir or Madam:

The following are in response to the frame inspection at 605 38th St., Sacramento, CA:

At the ground level shear wall, 3/8" shear applied to each face of wall may be used to replace the 15/32" shear schedule currently on the plans. One face of the wall should be nailed with 8d at 6" o.c. perimeter and 12" o.c. field. The other side of the wall is to be nailed with 8d at 3" o.c. at the perimeter and 12" o.c. field.

Since roof trusses were used, there is no interior bearing weight from the roof on the post & beam framing at the floor. Therefore, the posts may be placed at 5'-0" o.c. with 24" x 24" x 12" deep footings below. (see attached calculation for floor girder)

See attached garage truss calculations. These calculations are adequate and in compliance with the design of this structure.

Anchor bolts are to be spaced per plan. As a repair for missing anchor bolts use 1/2" threaded rod embedded min. 8" into stem wall with simpson epoxy-tie adhesive.

Since the shear wall at the ground level is sheared all the way to the floor sheathing, the strap tying the floor joists together is not required nor are any connections from the floor joist to the wall.

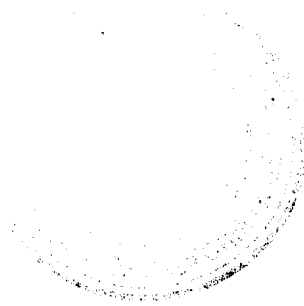
A question has arisen concerning the MST straps required at the wall top plates. The plans indicate that the straps are required "each side", this is meant as each side of the building. The straps are only required on one face of the top plate.

All simpson straps (MSTs and HPAHD22) used in this project may be nailed with 16d sinker nails.

Sincerely,

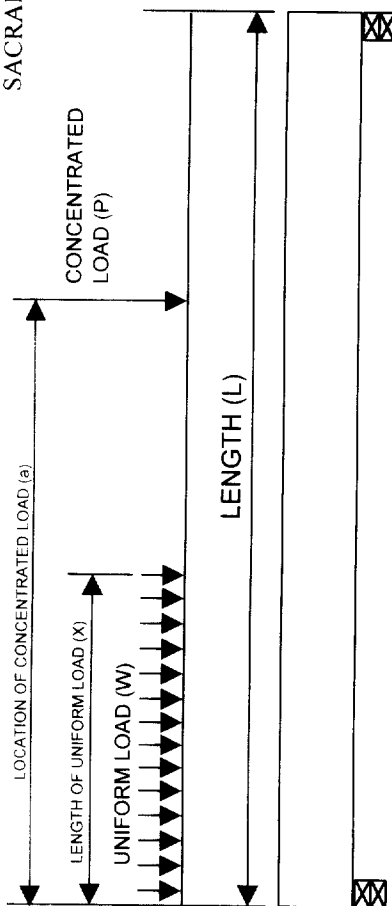


Richard M. Robertson, P.E.

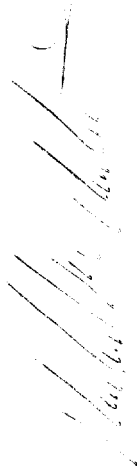


ROBERTSON ENGINEERING

8536 ELDER CREEK RD.
SACRAMENTO, CA 95828



BEAM DESCRIPTION: 605 38th St., Sacramento Floor Girder

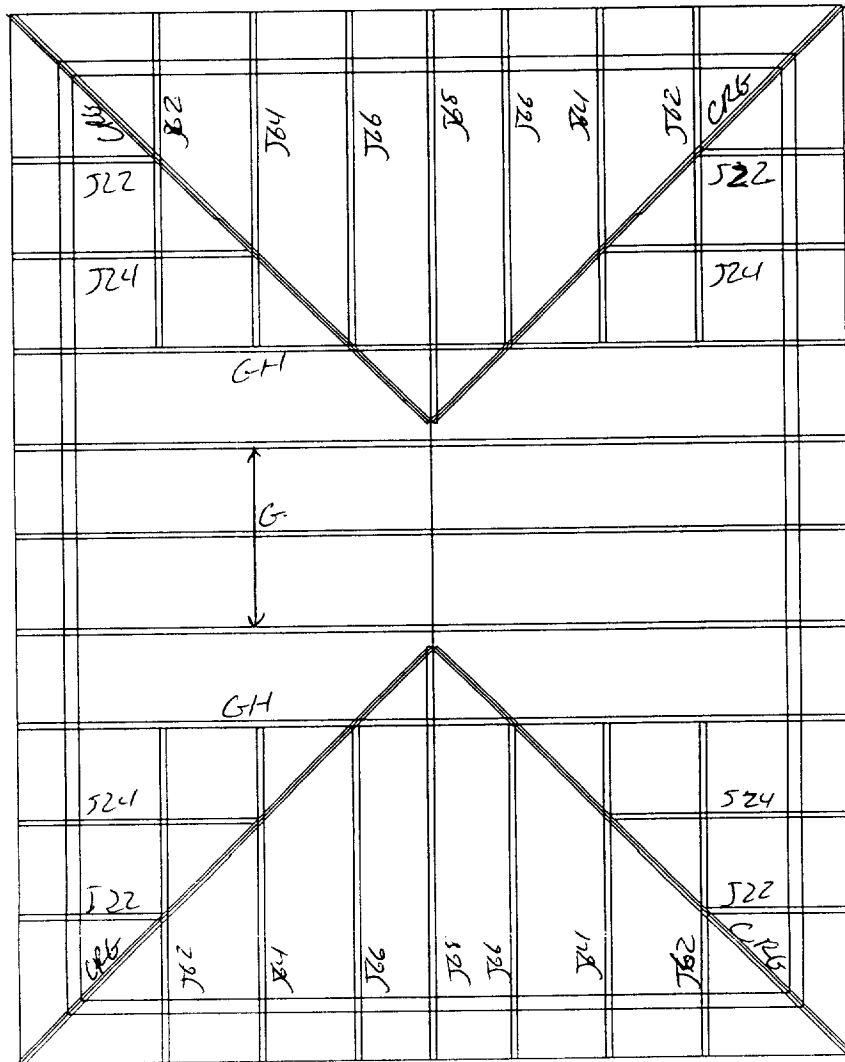


GENERAL NOTES:

1.) BEAM DESIGN IS NOT VALID WITHOUT ENGINEERS STAMP AND SEAL

	SAWN LUMBER OPTION 1	SAWN LUMBER OPTION 2	DOUBLE MICRO LAM	PARALLAM OPTION	20F-V4 GLULAM OPTION	24F-V4 GLULAM OPTION
4X8 DF NO. 2	USE DF NO. 2 OPTION	DOUBLE 1-3/4" X 5-1/2"	3-1/2" X 9-1/4"	3-1/8" X 6" 20F-V4	3-1/8" X 6" 24F-V4	
	CALCULATED	ALLOWABLE	CALCULATED	ALLOWABLE	CALCULATED	ALLOWABLE
BENDING STRESS (PSI)	733.82	1,137.50	NA	NA	1,200.00	1,200.00
BEARING CAPACITY (PSI)	244.90	625.00	NA	NA	274.29	274.29
SHEAR STRESS (PSI)	88.67	95.00	NA	NA	120.00	120.00
DEFLECTION (INCHES)	0.0474	0.1667	0.0915	0.1667	0.0938	0.1667
REACTION (R1) (LBS)						
AT LEFT END OF BEAM		1500				
REACTION (R2) (LBS)						
AT RIGHT END OF BEAM		1500				

	ROWS	SPACING(IN)
NAIL		
MICCOLLAMS TOGETHER		
NAILS	1	4
16d SINKERS		



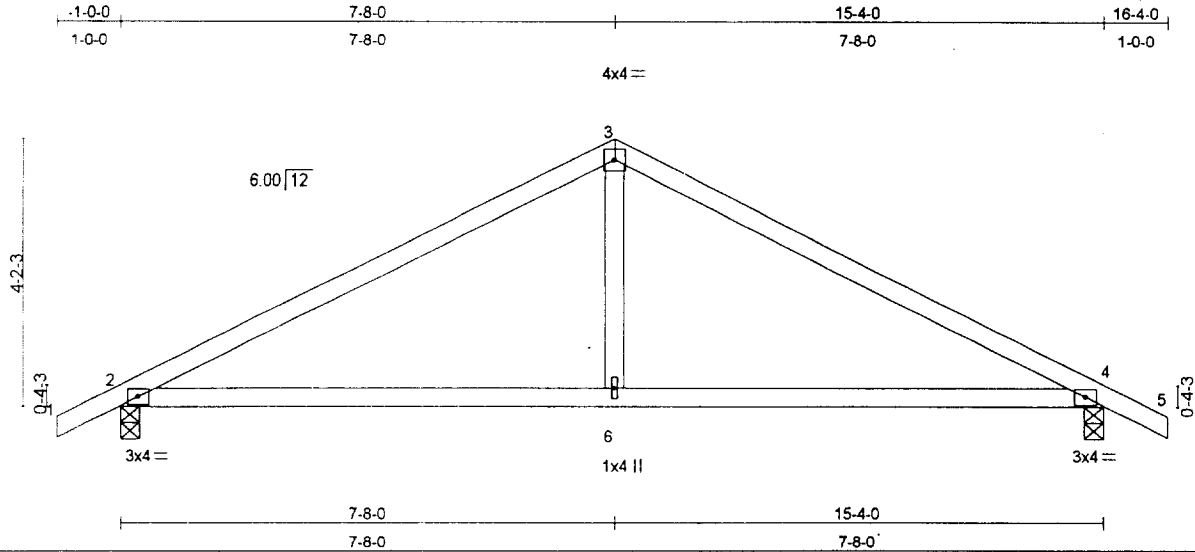
DYER NEED Construction!

GARAGE
 605 - 38TH - 1
 SACRAMENTO

Job	Truss	Truss Type	Qty	Ply	
38TH-GAR	G	KINGPOST	3	1	

BUILDERS TRUSS & LUMBER

4.0-32 s Jan 12 1999 MITek Industries, Inc. Fri Mar 05 07:27:27 1999 Page 1



LOADING (psf) TCLL 16.0 TCDL 14.0 BCLL 0.0 BCDL 7.0	SPACING 2-0-0 Plates Increase 1.00 Lumber Increase 1.25 Rep Stress Incr YES Code UBC/CBO	CSI TC 0.59 BC 0.41 WB 0.10	DEFL (in) (loc) l/defl Vert(LL) -0.08 4-6 >999 Vert(TL) -0.14 4-6 >999 Horz(TL) 0.01 4 n/a 1st LC LL Min l/defl = 360	PLATES GRIP M20 186/148 Weight: 52 lb
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LUMBER

TOP CHORD 2 X 4 DF No.1&Btr-G
BOT CHORD 2 X 4 DF No.1&Btr-G
WEBS 2 X 4 DF Std-G

BRACING

TOP CHORD Sheathed or 6-0-0 on center purlin spacing.
BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=625/0-3-8, 4=625/0-3-8

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=13, 2-3=-608, 3-4=-608, 4-5=13
BOT CHORD 2-6=541, 4-6=541
WEBS 3-6=105

NOTES

- 1) This truss has been checked for unbalanced loading conditions.
- 2) All plates are M20 plates unless otherwise indicated.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
- 4) A plate rating reduction of 20% has been applied for the green lumber members.
- 5) This truss has been designed for both UBC-94 and ANS/TPI 1-1995 plating criteria

LOAD CASE(S) Standard

MAR 04 1999



NOTE: This design is valid for use with MITek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	
38TH-GAR	GH	CAL HIP	2	1	

BUILDERS TRUSS & LUMBER

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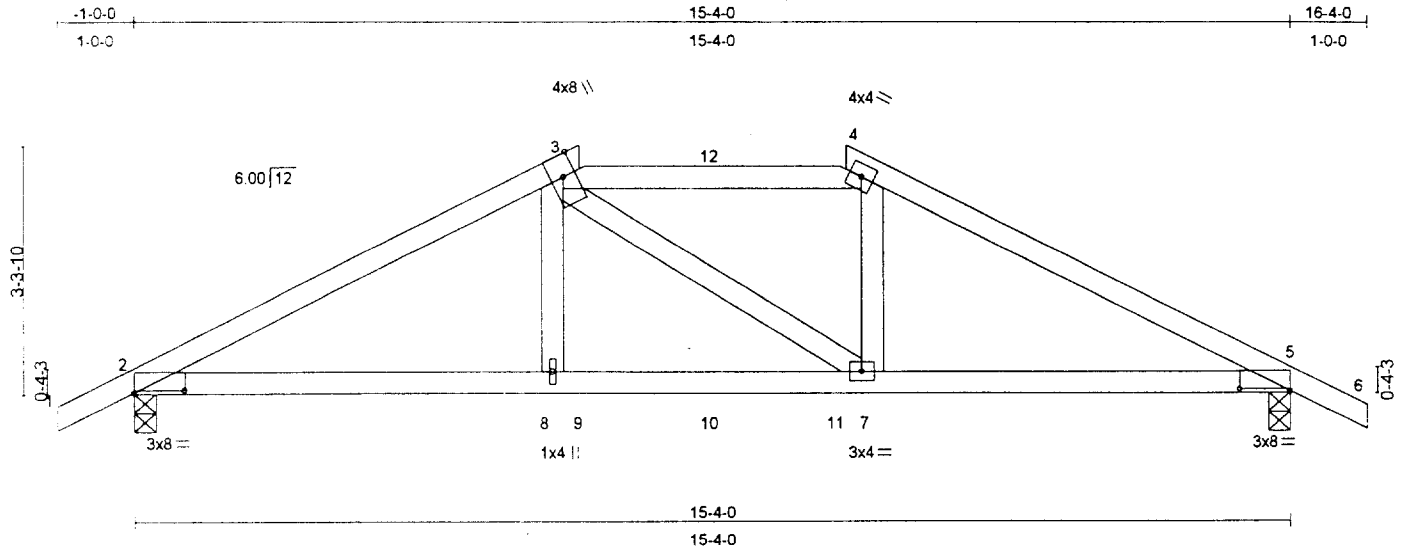


Plate Offsets (X, Y): [2 0-8-0,0-0-6], [5 0-8-0,0-0-6]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 16.0	Plates Increase 1.00	TC 0.45	Vert(LL) -0.03 8 >999	M20 186/148
TCDL 14.0	Lumber Increase 1.25	BC 0.37	Vert(TL) -0.08 7-8 >999	
BCLL 0.0	Rep Stress Incr NO	WB 0.09	Horz(TL) 0.03 5 n/a	
BCDL 7.0	Code UBC/ICBO		1st LC LL Min l/defl = 360	Weight: 61 lb

LUMBER
 TOP CHORD 2 X 4 DF No. 1&Btr-G
 BOT CHORD 2 X 4 DF No. 1&Btr-G
 WEBS 2 X 4 DF Std-G

BRACING
 TOP CHORD Sheathed or 4-3-0 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

REACTIONS (lb/size) 2=1102/0-3-8, 5=1102/0-3-8

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=13, 2-3=1802, 4-5=1802, 5-6=13, 3-12=1611, 4-12=1611
 BOT CHORD 2-8=1618, 8-9=1611, 9-10=1611, 10-11=1611, 7-11=1611, 5-7=1618
 WEBS 3-8=130, 4-7=130, 3-7=0

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) Except as shown below, special connection(s) required to support concentrated load(s). Design of connection(s) is delegated to the building designer.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) All plates are M20 plates unless otherwise indicated.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 6) A plate rating reduction of 20% has been applied for the green lumber members.
 - 7) This truss has been designed for both UBC-94 and ANSI/TPI 1-1995 plating criteria

LOAD CASE(S) Standard
 1) Regular Lumber Increase=1.25. Plate Increase=1.00
 Uniform Loads (plf)
 Vert: 1-2=-60.0, 2-3=-60.0, 4-5=-60.0, 5-6=-60.0, 2-8=-27.0, 8-9=-27.0, 9-10=-27.0, 10-11=-27.0, 7-11=-27.0, 5-7=-27.0, 3-12=-115.6,
 4-12=-115.6
 Concentrated Loads (lb)
 Vert: 3=-270 4=-270

MAR 04 1999

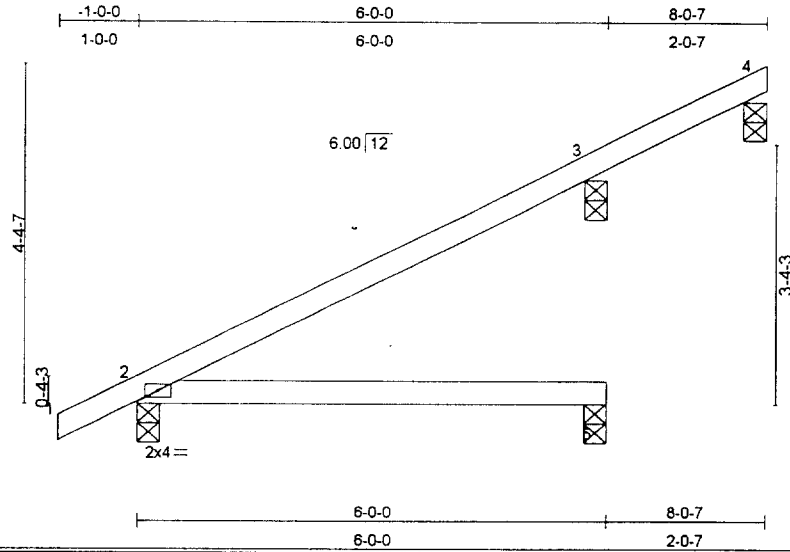


NOTE: This design is valid for use with MITek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	
38TH-GAR	J68	JACK	2	1	

BUILDERS TRUSS & LUMBER

4.0-32 s Jan 12 1999 MiTek Industries, Inc. Fri Mar 05 07:27:30 1999 Page 1



LOADING (psf)	SPACING	CSI	DEFL (in) (loc) /defl	PLATES GRIP
TCLL 16.0	2-0-0	TC 0.34	Vert(LL) -0.05 2-5 >999	M20 186/148
TCDL 14.0	Plates Increase 1.00	BC 0.21	Vert(TL) -0.08 2-5 >814	
BCLL 0.0	Lumber Increase 1.25	WB 0.00	Horz(TL) -0.00 3 n/a	
BCDL 7.0	Rep Stress Incr YES	(Matrix)	1st LC LL Min /defl = 360	Weight: 22 lb
	Code UBC/ICBO			

LUMBER
 TOP CHORD 2 X 4 DF No. 1&Btr-G
 BOT CHORD 2 X 4 DF No. 1&Btr-G

BRACING
 TOP CHORD Sheathed.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 on center bracing.

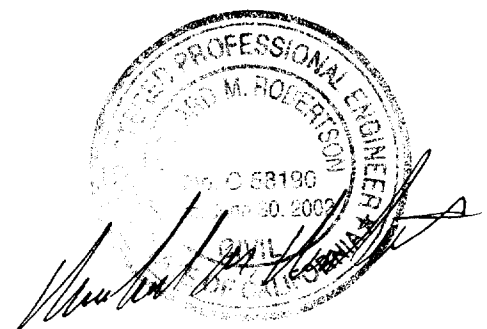
REACTIONS (lb/size) 4=61/0-3-8, 2=287/0-3-8, 5=40/0-3-8, 3=225/0-3-8
 Max Grav 5=97(load case 2), 3=225(load case 1)

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=26, 2-3=60, 3-4=22
 BOT CHORD 2-5=0

- NOTES**
- 1) All plates are M20 plates unless otherwise indicated.
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-94.
 - 3) A plate rating reduction of 20% has been applied for the green lumber members.
 - 4) This truss has been designed for both UBC-94 and ANS/TPI 1-1995 plating criteria.

LOAD CASE(S) Standard

MAR 04 1999



NOTE: This design is valid for use with MiTek connector plates only. This design is based on the parameters shown only, and is for an individual building component to be installed and loaded vertically except where noted. Applicability of design parameters and proper incorporation of this component is the responsibility of the building designer-not truss designer or truss engineer. The bracing indicated is for lateral support of the individual indicated truss member. Additional temporary and permanent bracing which is always required is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719