CITY OF SACRAMENTO

1231 I Street, Sacramento, CA 95814

Permit No: 0100689 2

Insp Area:

Sub-Type: REP Housing (Y/N): N

Site Address: 9 SPRINGBROOK CR SAC

Parcel No: **CONTRACTOR** 030-0121-017

ARCHITECT SPHARLER LON E & G JEAN

FERGUSON GARY DBA JOINT EFFORTS 6729 WALNUT AVE

ORANGEVALLE CA 95662

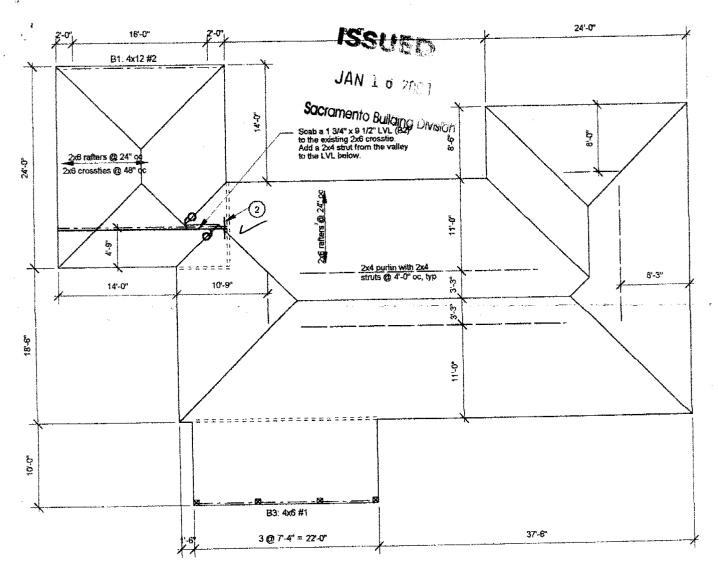
9 SPRINGBROOK CR SACRAMENTO CA 95831

OWNER

Nature of Work: TILE REROOF; 32 SQ

(commencing with section 7000) of	Lender's Address	_ ^
	Division 3 of the Business and Professions Code and my licen.	rjury that I am licensed under provisions of Chapter se is in full force and effect.
License Class D License Nu	imber 602864 Date 1 1 Contrac	ctor Signature
following reason (Sec. 7031.5, Busiany structure, prior to its issuance, a of the Contractors License Law (C.	RATION: I hereby affirm under penalty of perjury that I mess and Professions Code; any city or county which requires also requires the applicant for such permit to file a signed states thapter 9 (commencing with Section 7000) of Division 8 of the alleged exemption. Any violation of Section 7031 5 by an ed dollars (\$500.00).	a permit to construct, alter, improve, demolish, or repairment that he or she is licensed pursuant to the provision he Business and Professions Code) or that he or she is
for sale (Sec. 7044, Business and I thereon, and who does such work h	or my employees with wages as their sole compensation, will Professional Code: The Contractors License Law does not a imself or herself or through his/her own employees, provided approvement is sold within one year of completion, the owner of sale.)	apply to an owner of property who builds or improve that such improvements are not intended or offered for
The Contractors License Law contractor(s) licensed pursuant to the	am exclusively contracting with licensed contractors to cons a does not apply to an owner of property who builds or impro e Contractors License Law)	truct the project (Sec. 7044, Business and Profession ves thereon, and who contracts for such projects with
Lam exempt under Sec.	B & PC for this reason:	
Date	Owner Signature	
all measurements and locations show or private agreement relating to pern any improvement or the violation of I certify that I have read this applic relating to building construction and	ERMIT, the applicant represents, and the city relies on the re- wn on the application or accompanying drawings and that the i- missible or prohibited locations for such improvements. This b- any private agreement relating to location of improvements. ation and state that all information is correct. I agree to com- herby authorize representative(s) of his city to enter upon the Applicant/Agent Signature	improvement to be constructed does not violate any law uilding permit does not authorize any illegal location of the permit does not authorize any illegal location of the permit does not state laws.
WORKER'S COMPENSATIO I have and will maintain a cer performance of work for which the p	N DECLARATION: I hereby affirm under penalty of perjutificate of consent to self-insure for workers' compensation as termit is issued.	ary one of the following declarations: provided for by Section 3700 of the Labor Code, for the
I have and will maintain work which this permit is issued. My work	kers' compensation insurance, as required by Section 3700 of kers' compensation insurance carrier and policy number are:	f the Labor Code, for the performance of the work for
Carrier	Policy Number	Exp Date
shall not employ any person in any	repleted if the permit is for \$100 or less). I certify that in the permanner so as to become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall for mixther than the provisions of Section 3700 of the Labor Code, I shall for mixther than the provisions of Section 3700 of the Labor Code, I shall for mixther than the provisions of Section 3700 of the Labor Code, I shall for mixther than the permit is for \$100 or less).	n laws of California and agree that if I should become

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.





This set of plans and specifications must be kept on the job at all times and it is unlawful to make any changes or alterations from the same without written permission from the **Building Inspection Division.**

The approval of this plan and specification SHALL NOT be held to permit of violation of any City Ordinance or

REVIEWED BY





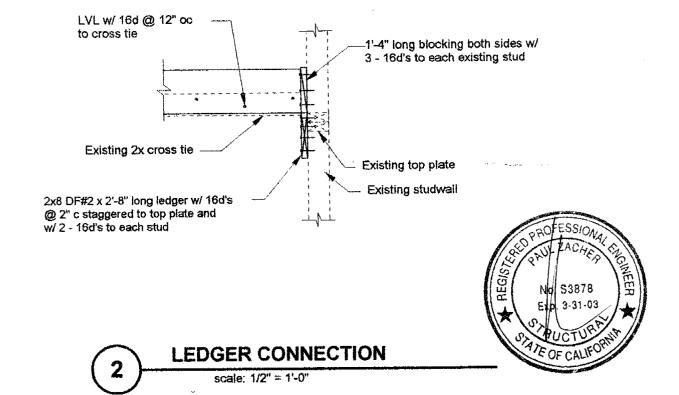
- 1. This is a reroof project. The new roofing material shall be a Light Weight Concrete Tile. The tile shall weigh less than or equal to 7.0 psf.
- All rafters are 2x6 DF#2 and hips and valleys are 2x8 DF#2 unless otherwise noted.
- 3. All existing rafter, hips, valleys, rafter ties, and purlins are braced per UBC Section 2320.12 "Roof and Ceiling Framing" unless otherwise shown.
- 4. All structural wood members that were observed appear to be in sound condition and without structural defect.



ROOF PLAN - SPHARLER

Not to Scale





Spharler

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Paul Zacher – Structural Engineers 4701 Lakeside Way Fair Oaks, CA 95628

TEL: 916.961.3960 FAX: 916.961.3960

November 8, 2000

Ventilated Roofing Systems P.O. Box 607 Orangevale, CA 95662 TEL: (916) 988-4139 FAX: (916) 987-1078

Attn.: Mr. Gary Ferguson,

re: Job 2000_384: SPHARLER



Subject: Structural Investigation Report of the Roof for the Residence located at 9 Springbrook Circle, Sacramento, CA 95831.

As requested by Mr. Gary Ferguson, this is a report to determine what needs should be addressed to correct any structural deficiencies of the roof. Paul Zacher visited the site November 8, 2000. The investigation was made to determine the existing condition of the structure. All information, data and analysis contained within this report are based on the 1997 Uniform Building Code.

The following is based on visual observations with no subsurface investigation being made.

DESCRIPTION:

Type of Facility:

Residence.

Year Built:

Estimated 1970's vintage.

Occupancy:

Residential.

No. of Stories:

One.

Dimensions:

Approximately 2500 square feet with a first story plate height of 8 feet.

CONSTRUCTION:

Roof:

The roof covering will consist of a Light Weight Concrete Tile over a batten system. The living area is conventionally framed with 2x6 rafters spaced at 24" on center with 2x4 purlins supported at no more than 4'-0" on center by 2x4 struts bearing on walls below. The garage area is framed with 2x6 rafters spaced at 24" on center and 2x6 cross ties spaced at 4'-0" on center.

CONCLUSIONS:

Roof:

The living area has sufficient structural capacity for the applied live and dead loads. The garage lacks sufficient structural capacity for the applied live and dead loads.



Paul Zacher – Structural Engineers 4701 Lakeside Way Fair Oaks, CA 95628

TEL: 916,961,3960

RECOMMENDATIONS:

If any of the following recommendations do not correspond to actual field conditions, the engineer of record shall be notified for further investigation and evaluation before continuing work.

Garage:

1. Scab a 1 3/4" x 9 ½" LVL beam to the existing 2x6 crosstie and nail together with 16d's @ 12 " oc. The ends of the LVL may be clipped as required to meet the slope of the rafters. The support at the interior wall shall be a 2x8 x 2'-8" long ledger attached to the double top plate with 16d's @ 2" oc staggered. Support the existing valley to the LVL beam with a 2x4 strut. See details 1 and 2.

It shall be noted that small hairline cracking may occur at exterior stucco and interior gypboard finished walls that are load bearing or distributing roof strut loads. These cracks are a natural occurrence as the existing structure re-distributes the new roof weight. They are cosmetic in nature and are not an indication of a structural hazard or failure.

It shall be noted that some deflection of the rafters may be evident after installation of the tile. The existing roof framing has deflected but this may not be readily evident due to the uneven nature of the existing roofing material. Concrete tile is a very consistent and uniform product and when installed in an even plane, even small deflections can become apparent. This is only a cosmetic issue and not a structural concern.

The inspection consisted of visual observation only, made solely to determine the structural capacity of the existing roof. Analysis does not determine any effects on the overall structure under lateral forces or effects on the foundation unless specifically noted in the calculations and in this document. No warranties, expressed or implied, are made or intended in conjunction with this report. The inspection was made only to the portions that were accessible. The specific items noted were those that were observable and there may be defects that are not observable, or are hidden by architectural and structural materials.

If you have any questions on the above, do not hesitate to call.

Sincerely,

Paul Zacher, P.E., S.E.

file

DESIGN LOADING:

Roof Pitch	4	in 12
Pitch Adjustment Factor	1.05	
LOCATION: ROOF BATTEN SYTEM		
MATERIAL	WEIGHT	
Light Weight Tile	7.00	psf
Roofing felt	0.30	psf
1x4 skip sht'g	1.09	psf
Batten system	1.00	psf
2x6 rafters @ 24" oc	<u>1.00</u>	psf
Load	10.4	psf
Roof Pitch Adjustment	0.56	psf
Total Load		psf

P.K. Za Job _. #: Date:	00)_1	584													F: TI	701 L air Oa EL: (9 AX: (1	aks, (916)	CA 9 961-	5628 3960		
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Paul Zacher - Structural Engineers

4701 Lakeside Way

Fair Oaks

TEL: (916) 961-3960 FAX: (916) 961-6552 Title : Dsgnr: Description : Job # Date: 5:47PM, 8 NOV 00

Scope:

Rev. 510304 User: KW-0602844, Ver 5.1.3, 22-Jun-1999, Win32 (c) 1983-99 ENERCALC

Timber Beam & Joist

c:\enercalc\test.ecw:Calculations

Description

RAFTERS AND BEAMS

Timber Member Ir	nforma	tion		Calculatio	ns are designed	to 1997 NDS and 1997 UBC Requirements
		rafter	B1	B 2	83	
Timber Section	1	2x6	4x12	LVL:1.750x	4x6	
Beam Width	in	1.500	3.500	1,750	3.500	
Beam Depth	in	5.500	11.250	9.500	5, 5 00	
Le: Unbraced Length	ft	0.00	0.00	0.00	0.00	The state of the s
Timber Grade	1	ouglas Fir - Larch, c	uglas Fir · Larch;ru	ss Joist - MacMilDo	ouglas Fir - Larch,	
Fb - Basic Allow	psi	875.0	875.0	2,600.0	875,0	
Fv - Basic Allow	psi	95.0	95.0	285.0	95.0	
Elastic Modulus	ksi	1,600.0	1,600.0	1,900.0	1,600.0	
Load Duration Factor	ļ	1,250	1,250	1,250	1.250	
Member Type		Sawn	Sawn	Manuf/Pine	Sawn	
Repetitive Status		Repetitive	No	No	No	
Center Span Data						
Span	ft	12.00	16.00	20.00	7.33	
Dead Load	#/ft	22.00	77.00		55.00	
Live Load	#/ft	32.00	112.00		80.00	
		02.00	()	202.00	*****	
Point #1 DL	lbs			396.00		
LL	lbs			576.00 16.500		
@ X	ft			10,500		
Results	Ratio =	0.9432	0.8171	0,3932	0.4336	
Mmax @ Center	in-k	11,66	72.58	33.64	10,88	
@ X =	ft	6.00	8,00	16.48	3,66	
fb : Actual	psi	1,542.3	983.0	1,277.9	616.6	
Fb : Allowable	psi	1.635.2	1.203.1	3.250.0	1,421.9	
PD . Allowable	Pai	Bending OK	Bending OK	Bending OK		
		-	-	_	-	
fv : Actual	psi	54.7	51.1	72.4	33.9	
Fv : Allowable	psi	118.8 Shear OK	118,8 Shear OK	356.3 Shear OK	118.8 Shear OK	
			SHRAF UK			
		Uniqui Vis			Silear ON	
Reactions		311031 415				
Reactions @ Left End DL	ibs	132.00	616.00	69.30	201.57	
	ibs Ibs			69.30 100.80	201.57 293.20	
@ Left End DL		132.00	616.00	69.30	201.57	
@ Left End DL LL Max. DL+LL	lbs lbs	132.00 192.00 324.00	616.00 896.00	69.30 100.80	201.57 293.20	
@ Left End DL LL Max. DL+LL @ Right End DL	lbs lbs lbs	132.00 192.00 324.00 132.00	616.00 896.00 1,512.00	69.30 100.80 170.10	201.57 293.20 494.77	
@ Left End DL LL Max. DL+LL	lbs lbs	132.00 192.00 324.00	616.00 896.00 1,512.00 616.00	69.30 100.80 170.10 326,70	201.57 293.20 494.77 201.57	
@ Left End DL LL Max. DL+LL @ Right End DL LL	lbs lbs lbs lbs	132.00 192.00 324.00 132.00 192.00	616.00 896.00 1,512.00 616.00 896.00	69.30 100.80 170.10 326.70 475.20 801.90	201.57 293.20 494.77 201.57 293.20	
@ Left End DL LL Max. DL+LL @ Right End DL LL Max. DL+LL	lbs lbs lbs lbs	132.00 192.00 324.00 132.00 192.00 324.00 Ratio OK	616.00 896.00 1,512.00 616.00 896.00 1,512.00	69.30 100.80 170.10 326.70 475.20 801.90	201.57 293.20 494.77 201.57 293.20 494.77 Deflection OK	
@ Left End DL Lt Max. DL+LL @ Right End DL LL Max. DL+LL Deflections	lbs lbs lbs lbs	132.00 192.00 324.00 132.00 192.00 324.00 Ratio OK	616.00 896.00 1,512.00 616.00 896.00 1,512.00 Deflection OK	69,30 100,80 170,10 326,70 475,20 801,90 Deflection OK	201.57 293.20 494.77 201.57 293.20 494.77 Deflection OK	
@ Left End DL LL Max. DL+LL @ Right End DL LL Max. DL+LL Deflections Center DL Defl L/Defl Ratio	lbs lbs lbs lbs	132.00 192.00 324.00 132.00 192.00 324.00 Ratio OK	616.00 896.00 1,512.00 616.00 896.00 1,512.00 Deflection OK	69.30 100.80 170.10 326.70 475.20 801.90 Deflection OK	201.57 293.20 494.77 201.57 293.20 494.77 Deflection OK	
@ Left End DL LL Max. DL+LL @ Right End DL LL Max. DL+LL Deflections Center DL Defl L/Defl Ratio Center LL Defl	lbs lbs lbs lbs	132.00 192.00 324.00 132.00 192.00 324.00 Ratio OK -0.308 466.8 -0.449	616.00 896.00 1,512.00 616.00 896.00 1,512.00 Deflection OK -0.171 1,123.6 -0.249	69.30 100.80 170.10 326.70 475.20 801.90 Deflection OK	201.57 293.20 494.77 201.57 293.20 494.77 Deflection OK -0.046 1,911.7	
@ Left End DL LL Max. DL+LL @ Right End DL LL Max. DL+LL Deflections Center DL Defl L/Defl Ratio Center LL Defl L/Defl Ratio	lbs lbs lbs lbs in	132.00 192.00 324.00 132.00 192.00 324.00 Ratio OK -0.308 466.8 -0.449 320.9	616.00 896.00 1,512.00 616.00 896.00 1,512.00 Deflection OK	69.30 100.80 170.10 326.70 475.20 801.90 Deflection OK -0.247 972.1 -0.359	201.57 293.20 494.77 201.57 293.20 494.77 Deflection OK -0.046 1,911.7 -0.067	
@ Left End DL LL Max. DL+LL @ Right End DL LL Max. DL+LL Deflections Center DL Defl L/Defl Ratio Center LL Defl	lbs lbs lbs lbs	132.00 192.00 324.00 132.00 192.00 324.00 Ratio OK -0.308 466.8 -0.449 320.9 -0.757	616.00 896.00 1,512.00 616.00 896.00 1,512.00 Deflection OK -0.171 1,123.6 -0.249 772.5	69.30 100.80 170.10 326.70 475.20 801.90 Deflection OK -0.247 972.1 -0.359 668.3	201.57 293.20 494.77 201.57 293.20 494.77 Deflection OK -0.046 1,911.7 -0.067 1,314.3	