

**CITY OF SACRAMENTO**

1231 I Street, Sacramento, CA 95814

Permit No: 0102429

Insp Area: 1

Thos Bros: 297H4

Site Address: 3534 D ST SAC

Parcel No: 004-0172-024

Sub-Type: NGAR

Housing (Y/N): N

CONTRACTOR

MARK BAKER  
216 32ND ST  
SAC CA 95816

OWNER

BAY RICHARD J  
3534 D ST  
SACRAMENTO CA 95816

ARCHITECT

Nature of Work: DEMO DETACHED GARAGE & BUILD NEW GARAGE WITH REDUCED SETBACK(372 SF)

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name \_\_\_\_\_ Lender's Address \_\_\_\_\_

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

X License Class B License Number 607462 X Date 10-3-01 X Contractor Signature Mark Baker

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. \_\_\_\_\_ B & PC for this reason: \_\_\_\_\_

Date \_\_\_\_\_ Owner Signature \_\_\_\_\_

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and herby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

X Date 10-3-01 X Applicant/Agent Signature Mark Baker

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier \_\_\_\_\_ Policy Number \_\_\_\_\_ Exp Date \_\_\_\_\_

X (This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

X Date 10-3-01 X Applicant Signature Mark Baker

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

# WALKER LUMBER CO. Inc.

Trusses  
Roofs  
Floors

Help: // [www.walker-lumber.com](http://www.walker-lumber.com)  
(916) 338-2121 Fax (916) 338-5353

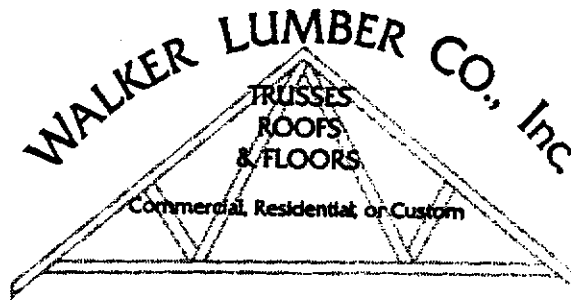
Where <sup>H<sup>1</sup></sup> ~~Per~~ ~~West~~ ~~Meals~~ Common Sense



Serving Sacramento For **Over 25 Years**

Specialty Products For **3534-D St** Job Name **2512** Job Number **10023**

*Baker Contracting* 3534 L Street



## Truss Maintenance and Guarantee Instruction

Project: 3534 D Street Date: 11-7-01

Contractor: Daker Contracting

Trusses are guaranteed to meet specifications on approved drawings and can not be guaranteed unless all items listed are complied with, whether written or implied.

1. Provide proper ventilation in all areas at all times.
2. Restraint of the structures lateral bracing is to be designed, installed and verified by job engineer, contractor & owner.
3. Provide a proper and adequate vapor barrier in all appropriate locations.
4. Do not expose truss to the weather. If any part of the truss is exposed to the weather it must be properly protected and maintained.
5. Owner and contractor must protect trusses from loads above the designed loads shown on the drawings.
6. Do not cut or otherwise alter the truss, its members, or support points.
7. Provide approved walk ways to mechanical units or unit that require service.
8. Owner and contractor to verify loads on the drawings are adequate for the building department load requirements and the requirements of the application.
9. All lateral bracing indicated on the approved truss drawings must be installed prior to loading of any trusses.
10. Trusses must be properly installed right side up at the proper bearing locations with the approved bracing and fasteners.
11. Truss bottom chords should not be restrained or permanently fastened down in any manner to prevent movement different from adjacent trusses other than support points.
12. Do not over load Trusses with any materials (units of sheeting, roof materials, sheet rock etc.)
13. Failure to comply with these requirements, all local building codes, specifications on plans, construction documents and approved truss drawings will void all guarantee's.

Thank You

Mike Walker  
Mike Walker Lumber Company, Inc.



# ICBO Evaluation Service, Inc.

5360 WORKMAN MILL ROAD • WHITTIER, CALIFORNIA 90601-2299

A subsidiary corporation of the International Conference of Building Officials

## EVALUATION REPORT

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ER-4994

Reissued November 1, 1997

**Filing Category: FASTENERS—Steel Gusset Plates (066)**

### ROBBINS LOCK (RL) AND ROBBINS HIGH STRENGTH (RHS) METAL CONNECTOR PLATES FOR WOOD TRUSSES

ROBBINS MANUFACTURING CO.  
POST OFFICE BOX 17939  
TAMPA, FLORIDA 33682-7939

#### 1.0 SUBJECT

Robbins Lock (RL) and Robbins High Strength (RHS) Metal Connector Plates for Wood Trusses.

#### 2.0 DESCRIPTION

##### 2.1 General:

Robbins Lock (RL) and Robbins High Strength (RHS) plates are metal connector plates for wood trusses. The plates are manufactured from galvanized steel in various lengths and widths, and have integral teeth that are designed to laterally transmit loads between truss wood members. Plans and calculations must be submitted to the building official for the trusses using metal connector plates described in this report.

##### 2.2 Materials:

Robbins Lock (RL) plates are manufactured from No. 20 gage [0.035 inch (0.89 mm)], ASTM A 653, Grade 40, structural-quality steel with a hot-dipped galvanized coating designated G60.

Robbins High Strength (RHS) plates are manufactured from No. 20 gage [0.035 inch (0.89 mm)], ASTM A 653 HSLA, Type I, Grade 60, high-strength, structural-quality steel with a hot-dipped galvanized coating designated G60.

Both the Robbins Lock (RL) and Robbins High Strength (RHS) plates have slots approximately  $\frac{1}{2}$  inch (12.7 mm) long by  $\frac{1}{8}$  inch (3.2 mm) wide that have been punched along the longitudinal axis of the plate. Each punched slot forms two opposite-facing, sharply pointed teeth protruding at right angles from the parent metal. The punched slots are spaced approximately  $\frac{1}{4}$  inch (6.4 mm) on center across the width of the plate and approximately 1 inch (25.4 mm) on center along the length of the plate, with adjacent longitudinal rows staggered  $\frac{1}{2}$  inch (12.7 mm). Connector plates are available in 1-inch (25.4 mm) increments of width and length. Minimum plate width and length are 1 inch (25.4 mm) and 3 inches (76 mm), respectively. See Figure 1 for details of plate dimensions.

There are eight teeth per square inch (1.24 teeth per square centimeter) of plate surface. The length of each tooth, including the thickness of the parent metal, is approximately  $\frac{3}{8}$  inch (9.5 mm), and the width of each tooth is approximately  $\frac{1}{8}$  inch (3.2 mm). The shank of each tooth is concave and the tip of each tooth is twisted approximately 40 degrees with respect

to the plate width. Adjacent longitudinal rows of teeth are twisted in opposite directions.

##### 2.3 Allowable Loads:

Tables 1, 2 and 3 provide allowable lateral loads, tension loads and shear loads for the RL and RHS plate connectors. These values are based on the National Design Standard for Metal Plate Connected Wood Truss Construction, ANSI/TPI 1-1995. A copy of the ANSI/TPI 1-1995 standard must be supplied to the building department when this is requested by the building official.

**2.3.1 Lateral Resistance:** Each metal connector plate must be designed to transfer the required load without exceeding the allowable load per square inch of plate contact area, which is based on species, the orientation of the teeth relative to the load, and the direction of load relative to grain. Design for lateral resistance must be in accordance with Section 11.2.1 of ANSI/TPI 1-1995. Table 1 provides allowable lateral loads for the metal connector plates.

**2.3.2 Tension Resistance:** Each metal connector plate must be designed for tension capacity that is based on the orientation of the metal connector plate relative to the direction of load. Design for tension must be in accordance with Section 11.2.2 of ANSI/TPI 1-1995. Table 2 provides allowable tension loads for the metal connector plates. Additionally, the net section of the metal connector plates for tension joints must be designed using the allowable tensile stress values of the metal that are adjusted by the metal connector plate tensile effectiveness ratios shown in Table 2.

**2.3.3 Shear Resistance:** Each metal connector plate must be designed for shear capacity that is based on the orientation of the plate relative to all possible lines of shear. Design for shear must be in accordance with Section 11.2.3 of ANSI/TPI 1-1995. Table 3 provides allowable shear loads for the metal connector plates. Additionally, the net section of the metal connector plates for heel joints, and other joints involving shear, must be designed using the allowable shear values for the metal connector plates that are adjusted by the shear resistance effectiveness ratios shown in Table 3.

**2.3.4 Metal Plate Reductions:** Several allowable-load reduction factors for the metal plates, when they are applicable, must be considered cumulatively in the design of metal connector plates used in fabricated wood trusses:

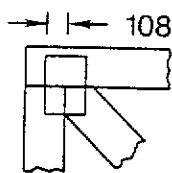
1. Allowable lateral resistance values for the RL and RHS metal connector plates must be reduced by 15 percent when the plates are installed on the narrow face of truss lumber members.
2. Allowable lateral resistance values must be reduced by 20 percent when the RL and RHS metal connector plates are installed in lumber having a moisture content greater than 19 percent at the time of truss fabrication.

*Evaluation reports of ICBO Evaluation Service, Inc., are issued solely to provide information to Class A members of ICBO, utilizing the code upon which the report is based. Evaluation reports are not to be construed as representing aesthetics or any other attributes not specifically addressed nor as an endorsement or recommendation for use of the subject report.*

*This report is based upon independent tests or other technical data submitted by the applicant. The ICBO Evaluation Service, Inc., technical staff has reviewed the test results and/or other data, but does not possess test facilities to make an independent verification. There is no warranty by ICBO Evaluation Service, Inc., express or implied, as to any "Finding" or other matter in the report or as to any product covered by the report. This disclaimer includes, but is not limited to, merchantability.*

# Symbols

## PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or in IN-16ths (i.e. 108).

## PLATE ORIENTATION



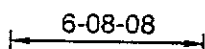
When shown, indicates direction of slots in connector plate.

## PLATE SIZE

6.3 x 8.8

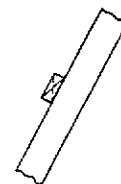
The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.

## DIMENSIONS



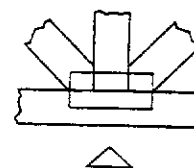
All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).

## LATERAL BRACING



Indicates approximate location of continuous lateral bracing required for stability of individual member.

## BEARING



Indicates support (bearing) at joint.

## NOTICE:

This truss designed at request and specification of customer as an individual building component, in a vertical plane, to be incorporated into building design at the specification of the building designer. Bracing shown is for lateral support of individual members only. Temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. Design and materials are in accordance with latest editions of **NDS, HUD Design Criteria for Trussed Rafters**, and **TPI specifications**. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult **Quality Control Manual, Bracing of Wood Trusses (HIB-91)** and **Recommended Code of Standard Practice**, available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

## General Notes:

1. Provide copies of this truss design to the building designer and erection supervisor.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and seat securely. Avoid knots and wane at joint locations.
4. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
5. Unless specifically noted, this design is not applicable for use with fire retardant or preservative treated lumber.
6. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively.
7. Anchorage and/or tie-in components are the responsibility of others, unless shown.
8. Do not stack construction materials on floor or roof that induces loading on truss greater than design loads.
9. Do not cut or alter truss without prior approval of a professional engineer.
10. Building designer is responsible to insure that loading shown hereon is applicable to building site.
11. Care should be exercised in handling, erection, and installation of trusses.
12. Camber is a non-structural consideration and is the responsibility of the truss fabricator. General practice is to camber for dead load deflection.
13. Refer to On-Line Data's Ch. VI-B for joint details.

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**MIKE WALKER LUMBER**  
P.O. Box 958  
North Highlands, CA 95660



**ROBBINS ENGINEERING, INC.**

*Bringing you the best of both worlds*

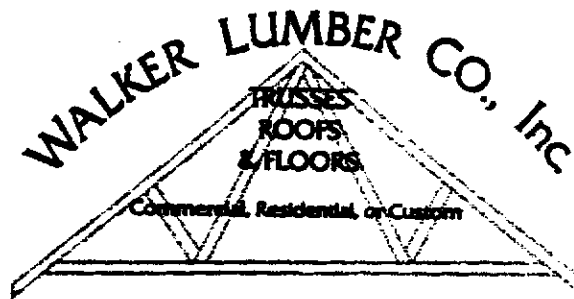
### NATIONAL HEADQUARTERS:

P.O. Box 280055  
Tampa, FL 33682-0055  
(800)282-1299/ FAX (813)971-6117

### REGIONAL OFFICES:

1402 20th Street NW Suite 4  
Auburn, WA 98001  
(800)532-9404/ FAX (206)735-2238

1177 Rockingham  
Richardson, TX 75080  
(214)238-9609/ FAX (214)437-0467



July 10, 2001

To: All Customers

From: Steve Kimbrell

RE: Safe Crane Operations

In order to ensure safe crane operation and to continue to deliver trusses efficiently, we would like to clarify our current delivery procedure.

**OSHA regulation 5002 requires that:**

**"Operations shall be conducted and the job controlled in a manner that will avoid exposure of employees to the hazard of overhead loads. Wherever loads must be passed directly over workers, occupied work spaces or occupied passageways, safety type hooks or equivalent means of preventing the loads from becoming disengages shall be used."**

There are many situations when there is no one available on the plate line to set trusses and assist the crane operator in releasing the safety hooks on the rigging. In these situations, the vicinity (to include the house and within 100 feet surrounding the truck and crane) would need to be completely evacuated. As stated in the OSHA regulation, safety latches are required only when the load is being passed over workers, occupied spaces or occupied passageways.

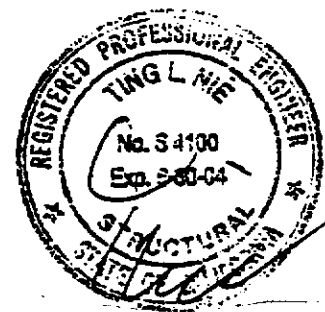
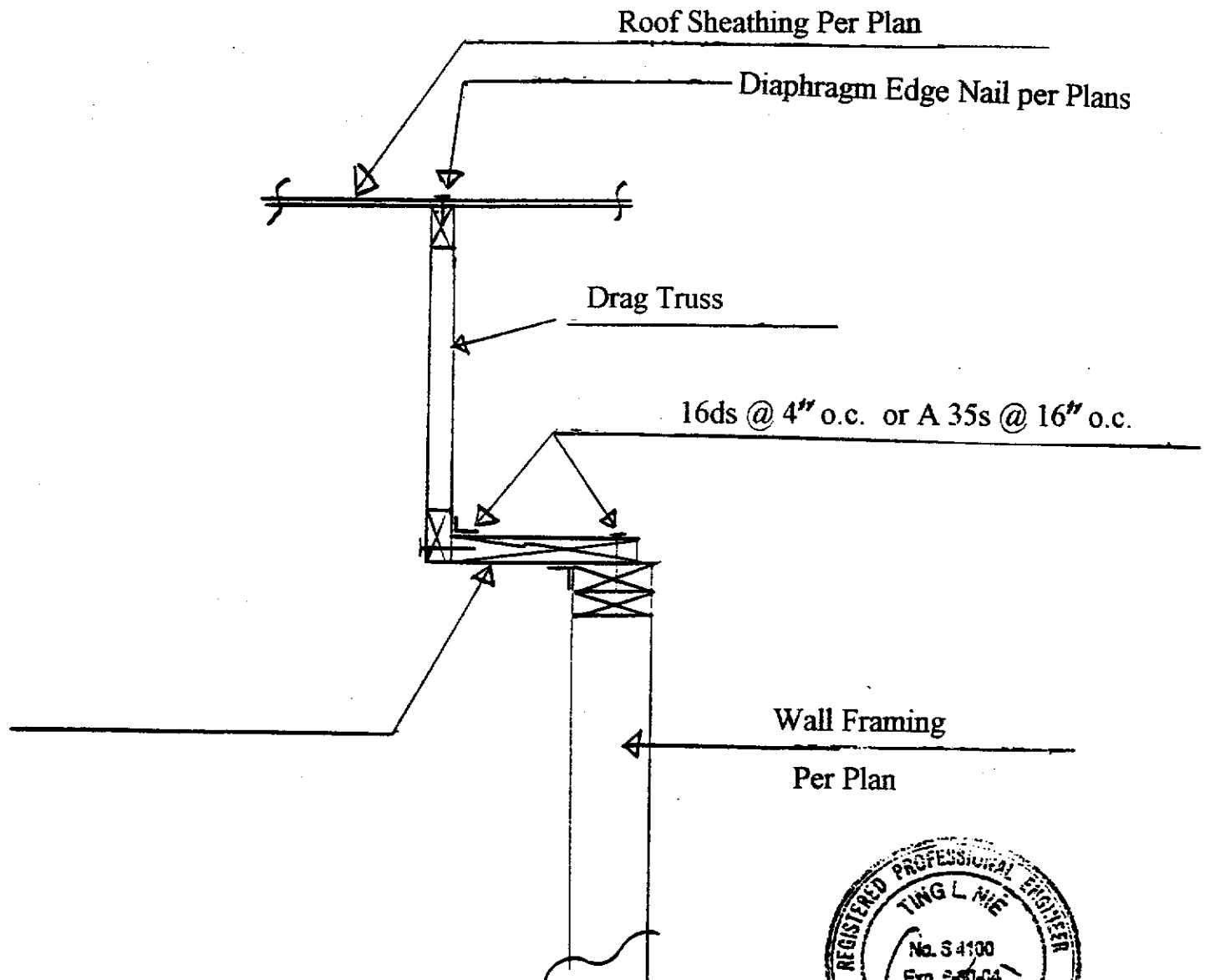
In all other situations, the safety latches on the rigging must be used. It is the responsibility of the customer or personal in charge to provide a person on the plate line to assist with setting the trusses and unhooking the safety latches.

If the customer or person in charge is unable or unwilling to provide a person on the plate line, there are two options:

- 1) The building/house can be completely evacuated as stated above to allow for the safe operation of the crane.
- 2) The trusses can be placed on the ground.

You have the support of the management of Mike Walker Lumber to fully implement this policy. Please notify Steve Kimbrell of any issues or concerns you have to make all deliveries safe.

ALTERNATE SHEAR TRANSFER



Not to Scale  
By: Raymond

Note: This detail is capable of transferring  
A lateral load up to 340 PLF



# Certificate of Quality

*Western Inspection Service, Inc.*  
P.O. Box 3426  
Gillette, WY 82717

We are an inspection agency recognized by the International Conference of Building Officials. I.C.B.O. Report No. AA-583.

*This is to certify that:*

**WALKER LUMBER COMPANY, INC.**  
**NORTH HIGHLANDS, CALIFORNIA**

*is under our Audited Quality Control Program and has been since:*

April 1, 2001

We audit the production under UBC and IBC Structural Engineering.

UBC Section U.B.C. 97 Section 2304.4.4 & ANSI/TPI 2321.3

IBC Section I.B.C. Section 2303.4

Inspections are made at least Quarterly and we make intermittent inspections of trusses in the field. We find quality observance of this plant to be in Conformance to the Standards.

**WALKER LUMBER CO., INC.**

Manufacturer

*Keith H. Anderson, Vice President*  
Western Inspection Services, Inc.



Detail for conventionally framed Valley Fill

Valley Jacks Spaced 24" o.c. clear span not to exceed 8' - 0" (DFL#2 or better)

Valley Pads See Plans

Jack Post 2 x 4's to be placed under Valley Jacks at no more than 8' - 0" o.c. and nailed to Jack Purlins.

Jack Purlins 2 x 4' - 0" spanning over three Trusses underneath and parallel to valley Jacks to which the Jack post supports are nailed.

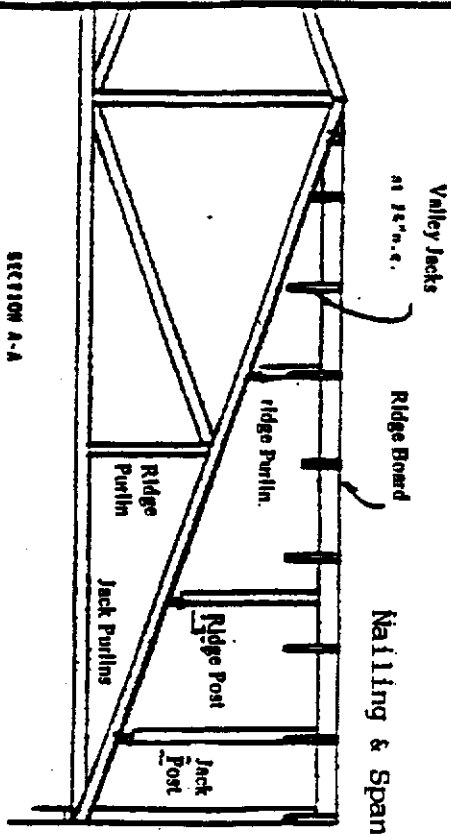
Ridge Board Same Species as Jacks, one size greater.

Ridge Post 2 x 4's to be placed under ridge board at no more than 5' - 0" o.c. and nailed to ridge Purlin.

Ridge Purlin 2 x 4 Whose length is 1/2 the valley width at the point indicated by a Ridge Post.

Nailing & Span Table per U.B.C.

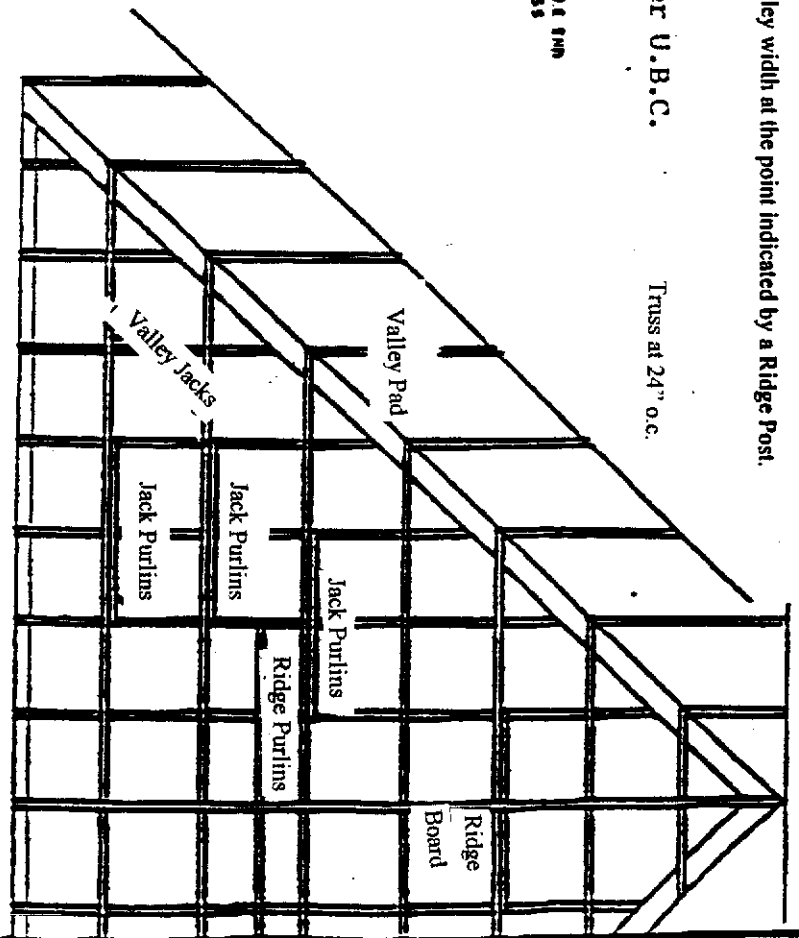
Truss at 24" o.c.

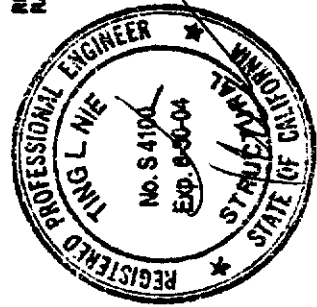
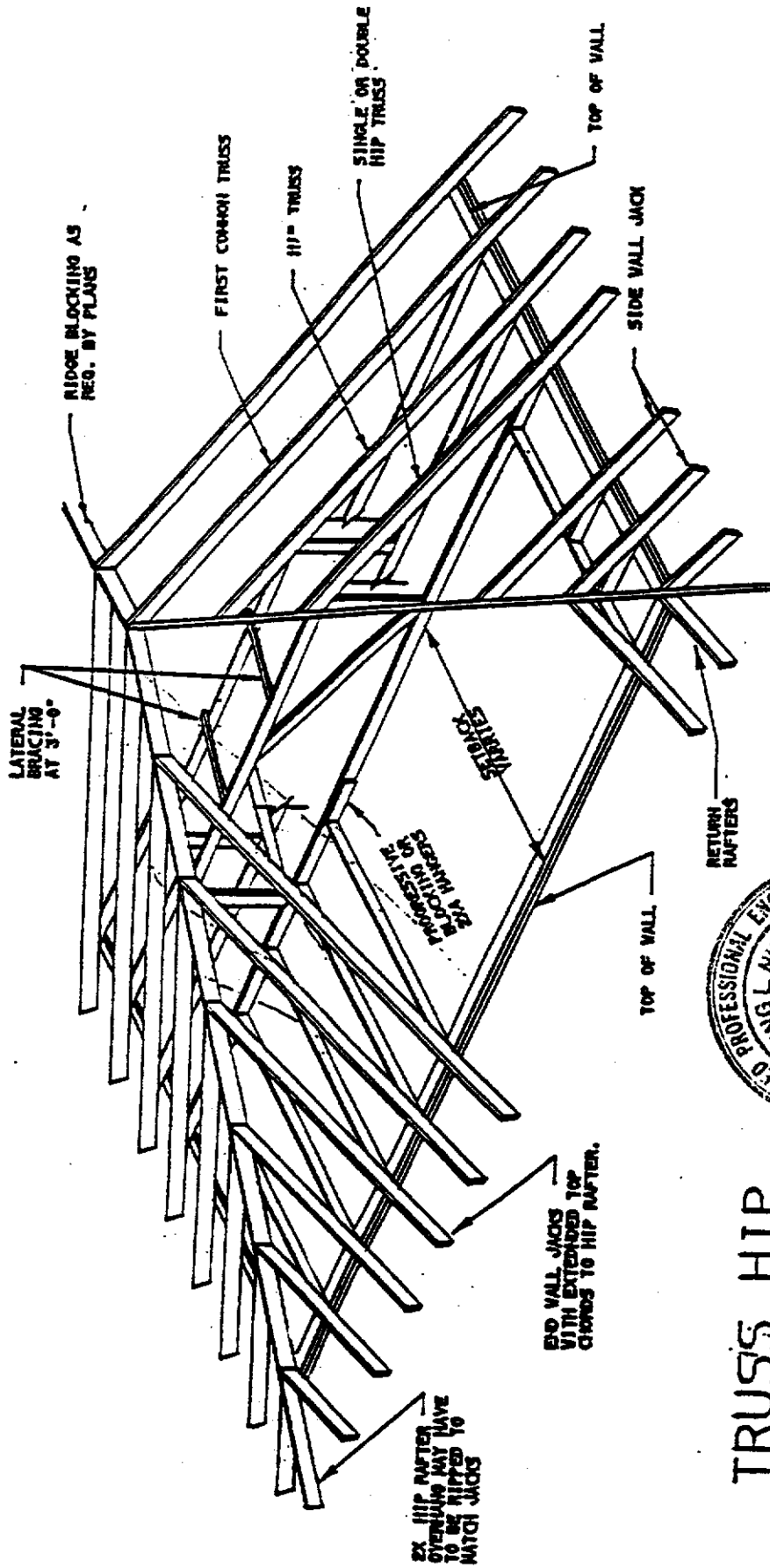


Limitation:

DL = 16 PSF

LL = 16 PSF





# TRUSS HIP SYSTEM



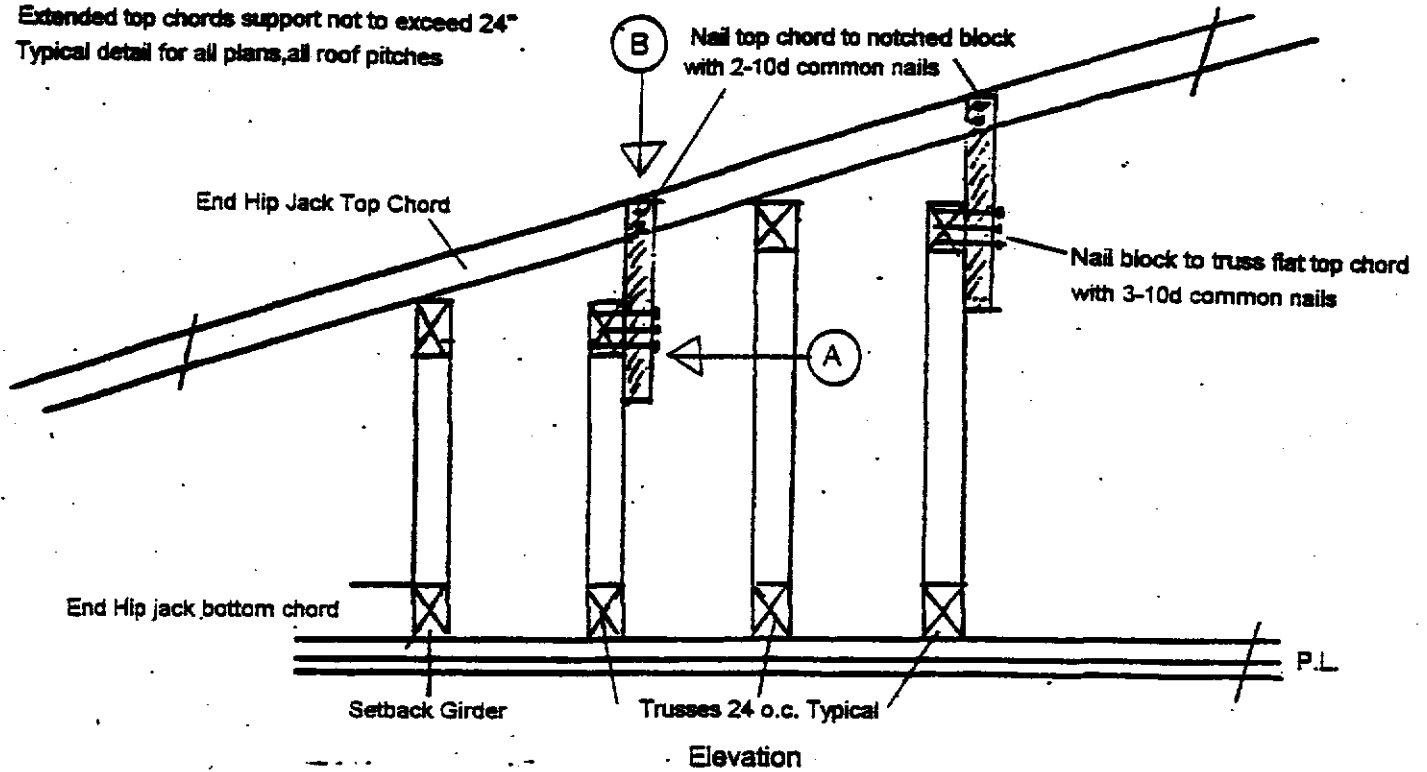
Re: End Hip Jacks Spaced At 24" O.C. Typical Top Chord Extension Support Detail.

Project: Typical for all Jobsites

Date: 12-14-00

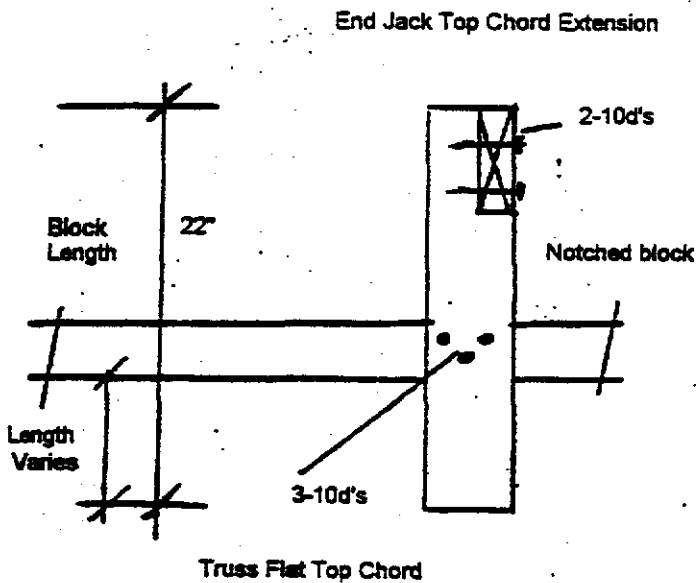
Extended top chords support not to exceed 24"

Typical detail for all plans, all roof pitches

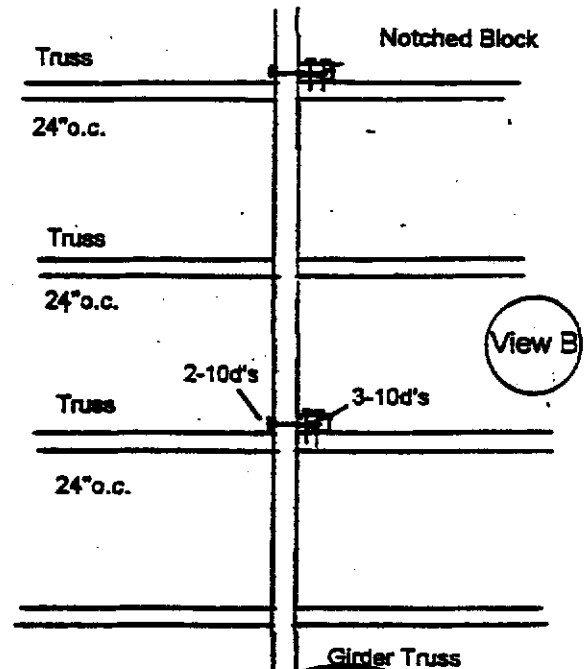


Elevation

End Jack Top Chord Extension

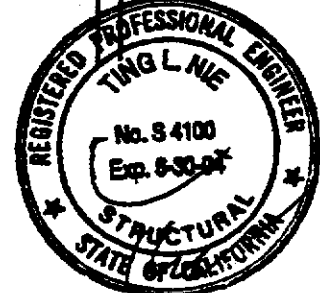


View A



View B

Details Not To Scale



# WARNING

DO NOT INSTALL THESE TRUSSES BEFORE  
READING INSTRUCTIONS.

## ERECTOR

DO NOT INSTALL THESE TRUSSES BEFORE READING  
AND COMPLYING WITH THESE INSTRUCTIONS.

## GENERAL CONTRACTOR

DO NOT LOAD THESE TRUSSES WITH PERMANENT OR  
TEMPORARY LOADS BEFORE READING THESE  
INSTRUCTIONS.

### 1. Before Erector These Trusses Erector Should:

- A. Obtain Study and Comply with "Bracing Wood Trusses" (BWT 76) as published by TPI. Obtain this guide from: The fabricator or dealer who provided the trusses or TPI. 7411 Riggs Road. Hyattsville Maryland 20783

### Alternate

- B. Obtain complete erection bracing plan from building architect. Engineer or structurally competent designer.

### Caution

Erector is responsible for all damages or injury as a result of inadequate bracing failures occurring during erection and prior to installation of permanent bracing.

Compliance with this industry guide is in your best interest.

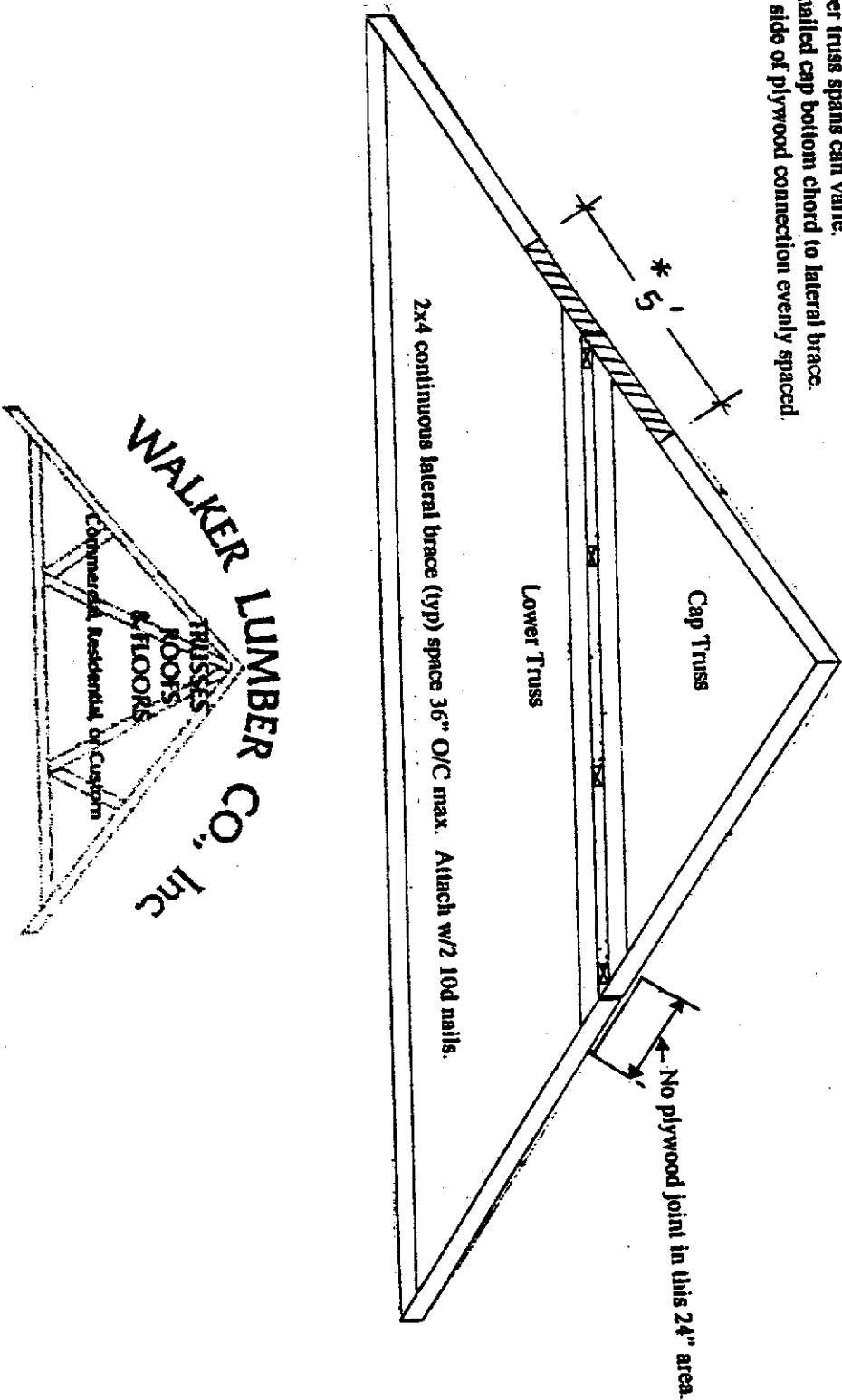
(Over)

Cap Truss Connection Details

- 1) This detail is for any 24" O/C truss.
- 2) Lower truss may have flat or pitched bottom chord.
- 3) Composition shake or tile loads can be used.
- 4) 1/2" CDX Plywood or O.S. B. minimum at connection for roof sheathing.
- 5) Use 8d nails with 1/2" plywood, 10d nails with 3/4" plywood.
- 6) Both upper and lower truss spans can vary.
- 7) Use 1 16d nail toe-nailed cap bottom chord to lateral brace.
- 8) Use 3 8d nails each side of plywood connection evenly spaced.

NOTES:

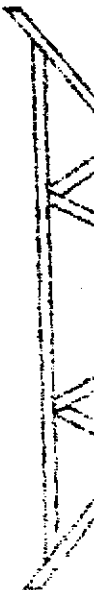
- 1) Plywood roof sheathing joint's should not occur within 12" of intersections of upper truss to lower truss.
- 2) In lieu of 24" min. plywood detail, use 24" 16g. coil strap over plywood not shown. Center on cap, main truss seam. Attach w/8d common. Nails (fill all holes).
- 3)\* At lateral load or collector load connection, scab one face of truss w/2x4x5 dist as shown. Attach w/16d common nails 3" O.C.



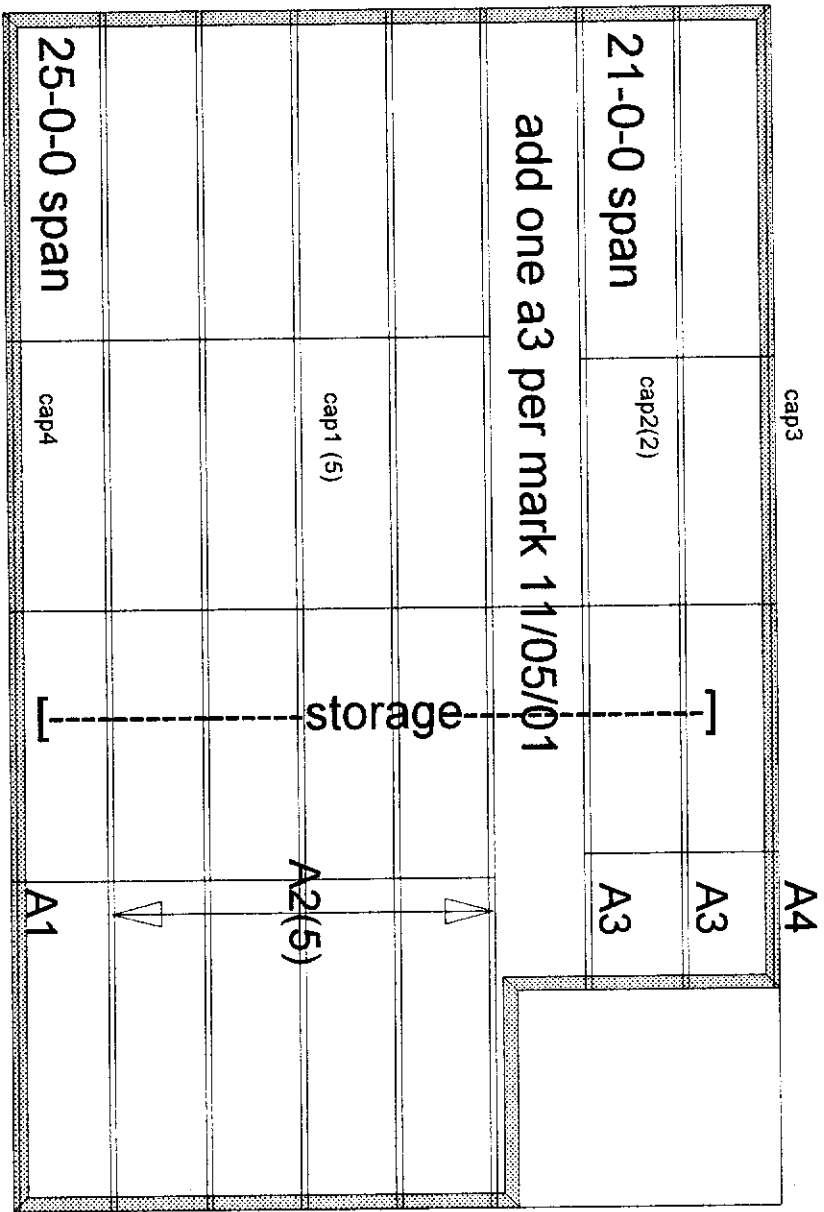
**WALKER LUMBER CO., Inc**

TRUSSES  
ROOFS  
FLOORS

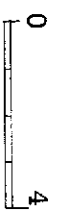
Commercial Residential or Custom

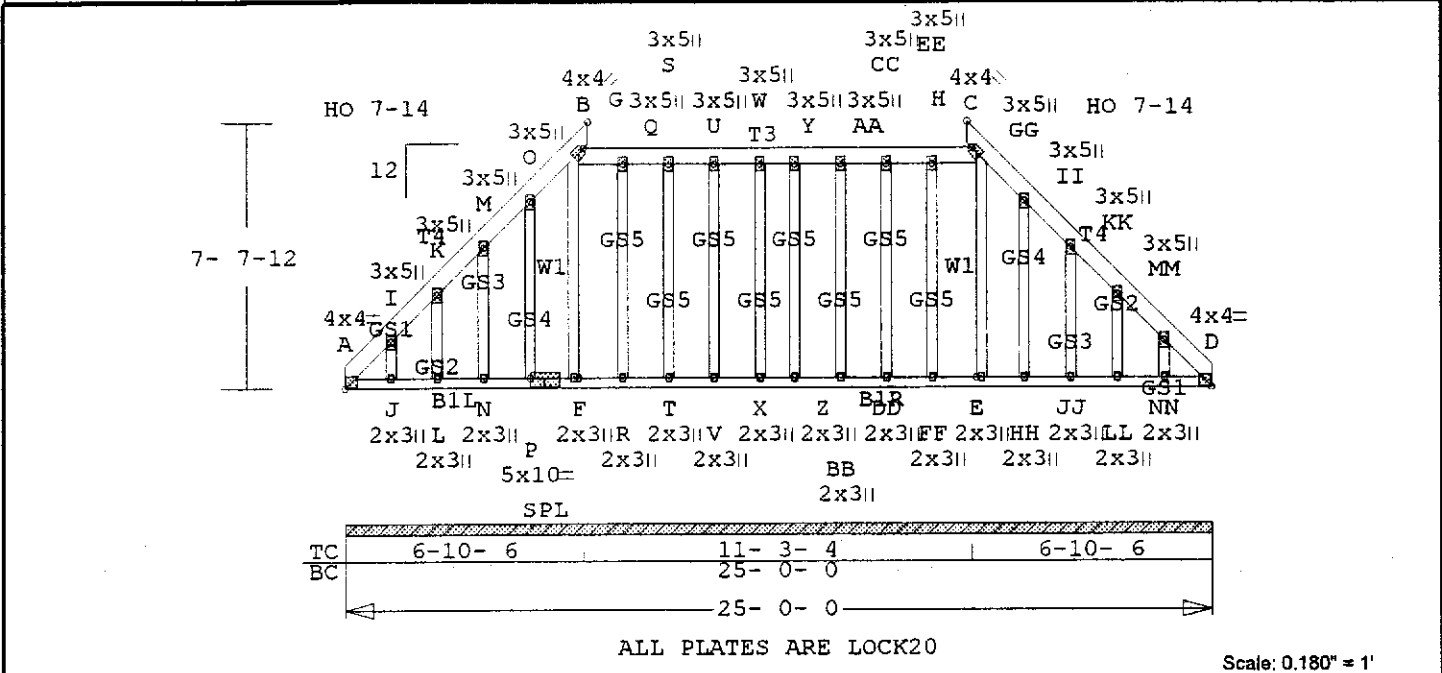


PO BOX 958 6915 30<sup>th</sup> STREET NORTH HIGHLANDS, CA. 95660  
 916 338-2121 FAX 916 338-5353 [mikew@walker-lumber.com](mailto:mikew@walker-lumber.com)



Baker Contracting  
 3534 D st.  
 cw023  
 10/26/01





Scale: 0.180" = 1'

APPROX. TRUSS WEIGHT: 303.8 LBS

Online Plus -- Version 10.0.001  
RUN DATE: 10-29-01

CSI	SIZE	LUMBER	FB
TOP	.05	2X 6 DFL-#2	1170
BTM	.06	2X 4 DFL-#2	1350
WBS	.06	2X 4 STA-STUD	661
EXCEPTIONS:			
F-B		2X 4 DFL-#2	1350
E-C SAME AS F-B			

LATERAL BRACING:  
TOP CHORD - CONTINUOUS  
BTM CHORD - CONTINUOUS  
TRUSS SPACING - 24.0 IN.

STANDARD LOADING

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
F-E	100.0	17.0	

SUPPORT CRITERIA  
CONTINUOUS BETWEEN JNTS A & D

LOAD CASE #1 UBC LL CHECK

LOADING	LIVE	DEAD	(PSF)
TOP CHD	10.0	7.0	
BTM CHD	10.0	21.0	31.0
EXCEPTIONS:			
F-E	110.0	17.0	

SUPPORT CRITERIA  
CONTINUOUS BETWEEN JNTS A & D

LOAD CASE #2 WIND FROM LEFT

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
A-B	5.7N	14.0	
B-C	-16.9N	14.0	
C-D	-9.1N	14.0	
F-E	-105.7N	17.0	

SUPPORT CRITERIA  
CONTINUOUS BETWEEN JNTS A & D

LOAD CASE #3 WIND FROM RIGHT

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
A-B	5.7N	14.0	
B-C	-16.9N	14.0	
C-D	-9.1N	14.0	
F-E	-105.7N	17.0	

SUPPORT CRITERIA  
CONTINUOUS BETWEEN JNTS A & D

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
V-X	.01	69 T	0	0
X-Z	.01	69 T	0	0
Z-BB	.01	69 T	0	0
BB-DD	.01	69 T	0	0
DD-FF	.01	69 T	0	0
FF-E	.01	69 T	0	0
E-HH	.01	69 T	0	0
HH-JJ	.01	69 T	0	2
JJ-LL	.01	69 T	-2	-8
LL-NN	.02	69 T	8	30
NN-D	.03	69 T	-30	-105

WBS

J-I	=	100 C	L-K	=	81 C
N-M	=	80 C	P-O	=	85 C
F-B	=	74 C	R-Q	=	71 C
T-S	=	81 C	V-U	=	80 C
X-W	=	71 C	Z-Y	=	71 C
BB-AA	=	80 C	DD-CC	=	81 C
FF-EE	=	71 C	E-C	=	74 C
HH-GG	=	78 C	JJ-II	=	80 C
LL-KK	=	81 C	NN-MM	=	77 C

SPAN/DEFL (DL+LL) = 999

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
LEFT				
HEEL 0IN - 2SX				
RIGHT				
HEEL 0IN - 2SX				
TOP CHORDS				
A-I	.05	201 C	369	77
I-K	.03	142 C	-77	13
K-M	.02	111 C	-13	16
M-O	.02	117 T	-12	42
O-B	.03	142 T	-42	184
B-Q	.03	140 T	-184	-51
Q-S	.02	140 T	51	-14
S-U	.02	140 T	14	10
U-W	.02	140 T	-10	12
W-Y	.02	140 T	-12	12
Y-AA	.02	140 T	-12	10
AA-CC	.02	140 T	-10	-14
CC-EE	.02	140 T	14	-51
EE-C	.03	140 T	51	184
C-GG	.03	142 T	-184	42
GG-II	.02	117 T	-42	12
II-KK	.01	57 T	-12	67
KK-MM	.01	51 C	-12	34
MM-D	.02	93 C	-34	-142
BOTTOM CHORDS				
A-J	.06	69 T	-255	-75
J-L	.02	69 T	75	20
L-N	.01	69 T	-20	-5
N-P	.01	69 T	5	1
P-F	.01	69 T	-1	0
F-R	.01	69 T	0	0
R-T	.01	69 T	0	0
T-V	.01	69 T	0	0



CHECKED NOV - 5 2001

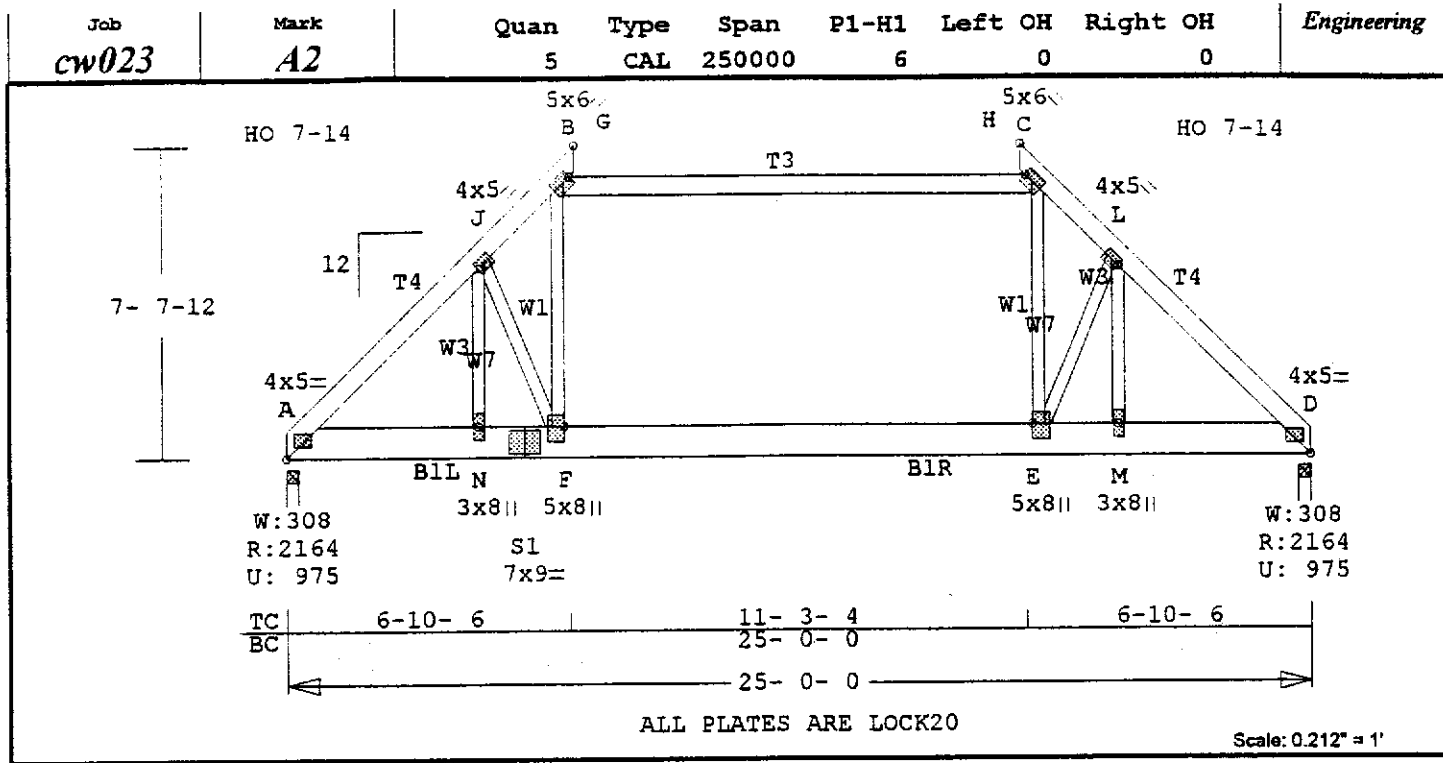


Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
cw023	A1	1	CAL	250000	6	0	0	Cont

NOTES:

1. TRUSSES MANUFACTURED BY - Walker Lumber
2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
3. EMPIRICAL ANALOG IS USED.
4. DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
5. WIND LOADS - ANSI/ASCE 7-98  
TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM  
WIND SPEED - 80 MPH  
MEAN ROOF HEIGHT - 25'  
EXPOSURE CATEGORY - C  
OCCUPANCY CATEGORY - 1  
ENCLOSED BUILDING.  
OCEANLINE DIST - 100 MILES  
TC DEAD LOAD = 14.0 PSF  
BC DEAD LOAD = 7.0 PSF
6. NOTE LARGE BEARING SIZE.
8. PROVIDE DRAINAGE TO PREVENT WATER PONDING.

CHECKED NOV - 5 2001



APPROX. TRUSS WEIGHT: 263.8 LBS

Online Plus -- Version 10.0.001  
 RUN DATE: 10-29-01

CSI	SIZE LUMBER	FB
TOP	.64 2X 6 DFL-#2	1170
BTM	.90 2X10 DFL-#1B	1320
WBS	.73 2X 4 STA-STUD	661

EXCEPTIONS:  
 F-B 2X 4 DFL-#2 1350  
 E-C SAME AS F-B

LATERAL BRACING:  
 TOP CHORD - CONTINUOUS  
 BTM CHORD - CONTINUOUS  
 TRUSS SPACING - 24.0 IN.

STANDARD LOADING  
 LUMBER STRESS INCREASE: 25.0%  
 PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)
TOP CHD	16.0	14.0
BTM CHD	.0	7.0
TOTAL	16.0	21.0 37.0

EXCEPTIONS:  
 F-E 100.0 17.0

SUPPORT CRITERIA

JT	REACT WIDTH	JT	REACT WIDTH
	LBS IN-SX		LBS IN-SX
A	2164 3- 8	D	2164 3- 8

LOAD CASE #1 UBC LL CHECK  
 LUMBER STRESS INCREASE: 25.0%  
 PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)
TOP CHD	.0	14.0
BTM CHD	10.0	7.0
TOTAL	10.0	21.0 31.0

EXCEPTIONS:  
 F-E 110.0 17.0

SUPPORT CRITERIA

JT	REACT WIDTH	JT	REACT WIDTH
	LBS IN-SX		LBS IN-SX
A	2014 3- 8	D	2014 3- 8

LOAD CASE #2 WIND FROM LEFT  
 LUMBER STRESS INCREASE: 33.3%  
 PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD (PSF)
TOP CHD	16.0	14.0
BTM CHD	.0	7.0
TOTAL	16.0	21.0 37.0

EXCEPTIONS:  
 A-B 5.7N 14.0  
 B-C -16.9N 14.0  
 C-D -9.1N 14.0  
 F-E -105.7N 17.0

SUPPORT CRITERIA

JT	TYPE	HORZ	VERT	WIDTH
		LBS	LBS	IN-SX
A	PIN	-179	-718	3- 8
D	HORZ RLR	0	-815	3- 8

LOAD CASE #3 WIND FROM RIGHT  
 LUMBER STRESS INCREASE: 33.3%  
 PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD (PSF)
TOP CHD	16.0	14.0
BTM CHD	.0	7.0
TOTAL	16.0	21.0 37.0

EXCEPTIONS:  
 A-B -9.1N 14.0  
 B-C -16.9N 14.0  
 C-D 5.7N 14.0  
 F-E -105.7N 17.0

SUPPORT CRITERIA

JT	TYPE	HORZ	VERT	WIDTH
		LBS	LBS	IN-SX
A	PIN	179	-815	3- 8
D	HORZ RLR	0	-718	3- 8

LOAD CASE #4 WIND // RIDGE  
 LUMBER STRESS INCREASE: 33.3%  
 PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD (PSF)
TOP CHD	16.0	14.0
BTM CHD	.0	7.0
TOTAL	16.0	21.0 37.0

EXCEPTIONS:  
 A-B -16.9N 14.0  
 B-C -16.9N 14.0  
 C-D -16.9N 14.0  
 F-E -105.7N 17.0

SUPPORT CRITERIA

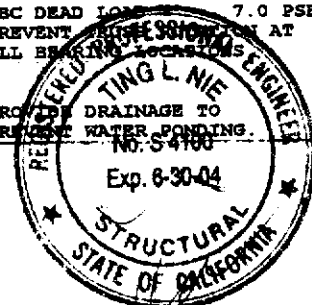
JT	TYPE	HORZ	VERT	WIDTH
		LBS	LBS	IN-SX
A	PIN	0	-975	3- 8
D	HORZ RLR	0	-975	3- 8

HEEL LEFT 0IN - 2SX RIGHT 0IN - 2SX

MEMBR	CSI	P(LBS)	M#1ST	M#2ND
TOP CHORDS				
A-J	.25	2713 C	815	571
J-B	.55	2894 C	-571	-5577
B-C	.64	2456 C	6714	-6714
C-L	.55	2894 C	5577	571
L-D	.25	2713 C	-571	-815
BOTTOM CHORDS				
A-N	.17	1904 T	-159	1764
N-S1	.43	1904 T	-1764	-10138
S1-F	.82	1904 T	10138	-22386
F-E	.90	2456 T	24200	-24200
E-M	.82	1904 T	22387	1764
M-D	.17	1904 T	-1764	159
WEBS				
J-F	.73	1177 T	0	-618
E-L	.73	1177 T	618	0
N-J	=	931 C	F-B =	1423 T
E-C	=	1423 T	M-L =	931 C

DL+LL DEFL = .25" IN B-C  
 LL DEFL = .21" < BRG-SPAN/360  
 SPAN/DEFL (DL+LL) = 999

- NOTES:
- TRUSSES MANUFACTURED BY - Walker Lumber
  - ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
  - EMPIRICAL ANALOG IS USED.
  - DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
  - WIND LOADS - ANSI/ASCE 7-98 TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM WIND SPEED - 80 MPH MEAN ROOF HEIGHT - 25' EXPOSURE CATEGORY - C OCCUPANCY CATEGORY - 1 ENCLOSED BUILDING. OCEANLINE DIST - 100 MILES TC DEAD LOAD = 14.0 PSF BC DEAD LOAD = 7.0 PSF
  - PREVENT CORROSION AT ALL BEARING LOCATIONS
  - PROVIDE DRAINAGE TO PREVENT WATER PONDING.



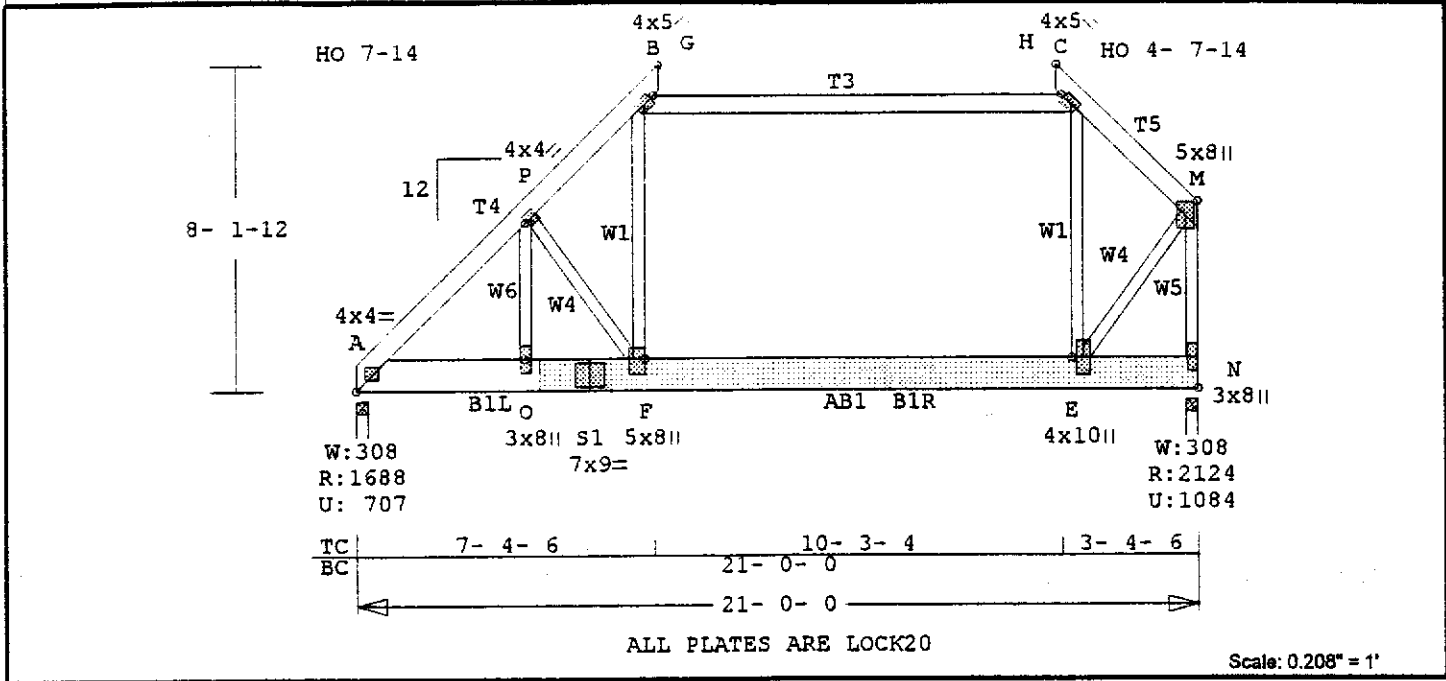
CHECKED NOV - 5 2001

Job  
cw023

Mark  
A3

Quan Type Span P1-H1 Left OH Right OH  
3 CAL 210000 6 0 0

Engineering



APPROX. TRUSS WEIGHT: 308.0 LBS

Online Plus -- Version 10.0.001  
RUN DATE: 11- 5-01

CSI	SIZE LUMBER	FB
TOP	.69 2X 6 DFL-#2	1170
BTM	.81 2X10 DFL-#2	990
WBS	.85 2X 4 STA-STUD	661

EXCEPTIONS:  
P-F 2X 4 DFL-#2 1350  
E-M N-M SAME AS P-F  
ADDED LUMBER:  
O-S1 2X10 DFL-#1B 1320  
S1-F F-E E-N SAME AS O-S1

LATERAL BRACING:  
TOP CHORD - CONTINUOUS  
BTM CHORD - CONTINUOUS  
TRUSS SPACING - 24.0 IN.

STANDARD LOADING  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
F-E 100.0 17.0  
SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 1688 3- 8 N 2124 3- 8

LOAD CASE #1 UBC LL CHECK  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD .0 14.0  
BTM CHD 10.0 7.0  
TOTAL 10.0 21.0 31.0

EXCEPTIONS:  
F-E 110.0 17.0  
SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 1562 3- 8 N 1998 3- 8

LOAD CASE #2 WIND FROM LEFT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
A-B 5.7N 14.0  
B-C -16.9N 14.0  
C-M -9.1N 14.0  
F-E -105.7N 17.0  
M-N 8.0N .0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN -191 -461 3- 8  
N HORZ RLR 0 -945 3- 8

LOAD CASE #3 WIND FROM RIGHT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
A-B -9.1N 14.0  
B-C -16.9N 14.0  
C-M 5.7N 14.0  
F-E -105.7N 17.0  
M-N -11.3N .0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN 244 -558 3- 8  
N HORZ RLR 0 -966 3- 8

LOAD CASE #4 WIND // RIDGE  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
A-B -16.9N 14.0  
B-C -16.9N 14.0  
C-M -16.9N 14.0  
F-E -105.7N 17.0  
M-N 16.9N .0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN -11 -707 3- 8  
N HORZ RLR 0 -1084 3- 8

HEEL LEFT RIGHT  
0IN - 2SX

MEMBR CSI P(LBS) M#1ST M#2ND  
TOP CHORDS

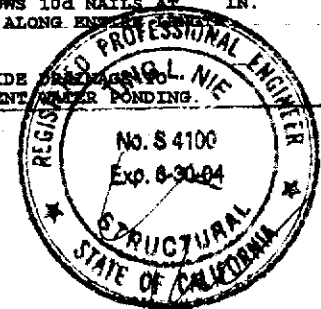
A-P	.18	2053	C	365	238
P-B	.27	1900	C	-238	-2798
B-C	.69	1508	C	3094	-7491
C-M	.57	1812	C	5987	-7

BOTTOM CHORDS  
A-O .21 1444 T -124 2220  
O-S1 .18 1444 T -2219 2849  
S1-F .19 1444 T -2849 3046  
F-E .81 1508 T -2949-37759  
E-N .64 70 T-21840 -1

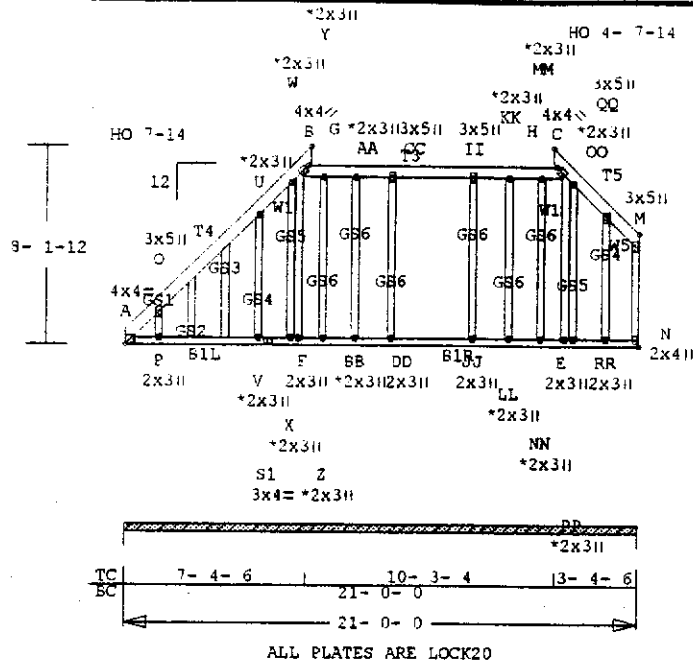
WEBS  
E-C .85 847 T 1209 1089  
E-M .59 2362 T 890 0  
O-P = 377 T P-F = 522 C  
F-B = 1009 T N-M = 3017 C

DL+LL DEFL = .22" IN B-C  
LL DEFL = .13" < BRG-SPAN/360  
SPAN/DEFL (DL+LL) = 999

- NOTES:
1. TRUSSES MANUFACTURED BY - Walker Lumber
  2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
  3. EMPIRICAL ANALOG IS USED.
  4. DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
  5. WIND LOADS - ANSI/ASCE 7-98  
TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM  
WIND SPEED - 80 MPH  
MEAN ROOF HEIGHT - 25'  
EXPOSURE CATEGORY - C  
OCCUPANCY CATEGORY - 1  
ENCLOSED BUILDING.  
OCEANLINE DIST - 100 MILES  
TC DEAD LOAD = 14.0 PSF  
BC DEAD LOAD = 7.0 PSF
  6. PREVENT TRUSS ROTATION AT ALL BEARING LOCATIONS.
  7. FASTEN EACH ADDED SCAB WITH ROWS 10d NAILS AT IN. O.C. ALONG ENDS.
  9. PROVIDE BRACING TO PREVENT MEMBER BOWING.



CHECKED NOV - 5 2001



Scale: 0.128" = 1'

APPROX. TRUSS WEIGHT: 283.9 LBS

Online Plus -- Version 10.0.001  
RUN DATE: 11- 5-01

CSI	SIZE	LUMBER	FB
TOP	.19	2X 6 DFL-#2	1170
BTM	.07	2X 4 DFL-#2	1350
WBS	.29	2X 4 STA-STUD	661

EXCEPTIONS:  
N-M 2X 4 DFL-#2 1350

LATERAL BRACING:

TOP CHORD - CONTINUOUS  
BTM CHORD - CONTINUOUS  
GABLE STUDS ARE CONSIDERED  
NON-STRUCTURAL. ALL BRACING  
FOR HORIZONTAL WIND LOADS ARE  
THE RESPONSIBILITY OF THE  
BUILDING DESIGNER.  
TRUSS SPACING - 24.0 IN.

STANDARD LOADING

LOADING	LIVE	DEAD (PSF)
TOP CHD	16.0	14.0
BTM CHD	.0	7.0
TOTAL	16.0	21.0

EXCEPTIONS:  
F-E 100.0 17.0  
SUPPORT CRITERIA  
CONTINUOUS BETWEEN JNTS A & N

LOAD CASE #1 UBC LL CHECK

LOADING	LIVE	DEAD (PSF)
TOP CHD	.0	14.0
BTM CHD	10.0	7.0
TOTAL	10.0	21.0

EXCEPTIONS:  
F-E 110.0 17.0  
SUPPORT CRITERIA  
CONTINUOUS BETWEEN JNTS A & N

LOAD CASE #2 WIND FROM LEFT

LOADING	LIVE	DEAD (PSF)
TOP CHD	16.0	14.0
BTM CHD	.0	7.0
TOTAL	16.0	21.0

EXCEPTIONS:  
A-B 5.7N 14.0  
B-C -16.9N 14.0  
C-M -9.1N 14.0  
F-E -105.7N 17.0  
M-N 8.0N .0  
SUPPORT CRITERIA  
CONTINUOUS BETWEEN JNTS A & N

LOAD CASE #3 WIND FROM RIGHT

LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)

TOP CHD	16.0	14.0
BTM CHD	.0	7.0
TOTAL	16.0	21.0

EXCEPTIONS:  
A-B -9.1N 14.0  
B-C -16.9N 14.0  
C-M 5.7N 14.0  
F-E -105.7N 17.0  
M-N -11.3N .0  
SUPPORT CRITERIA  
CONTINUOUS BETWEEN JNTS A & N

LOAD CASE #4 WIND // RIDGE

LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)

TOP CHD	16.0	14.0
BTM CHD	.0	7.0
TOTAL	16.0	21.0

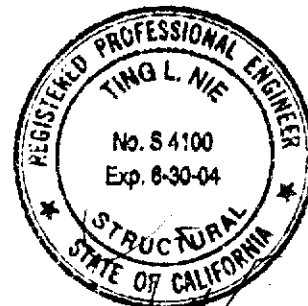
EXCEPTIONS:  
A-B -16.9N 14.0  
B-C -16.9N 14.0  
C-M -16.9N 14.0  
F-E -105.7N 17.0  
M-N 16.9N .0  
SUPPORT CRITERIA  
CONTINUOUS BETWEEN JNTS A & N

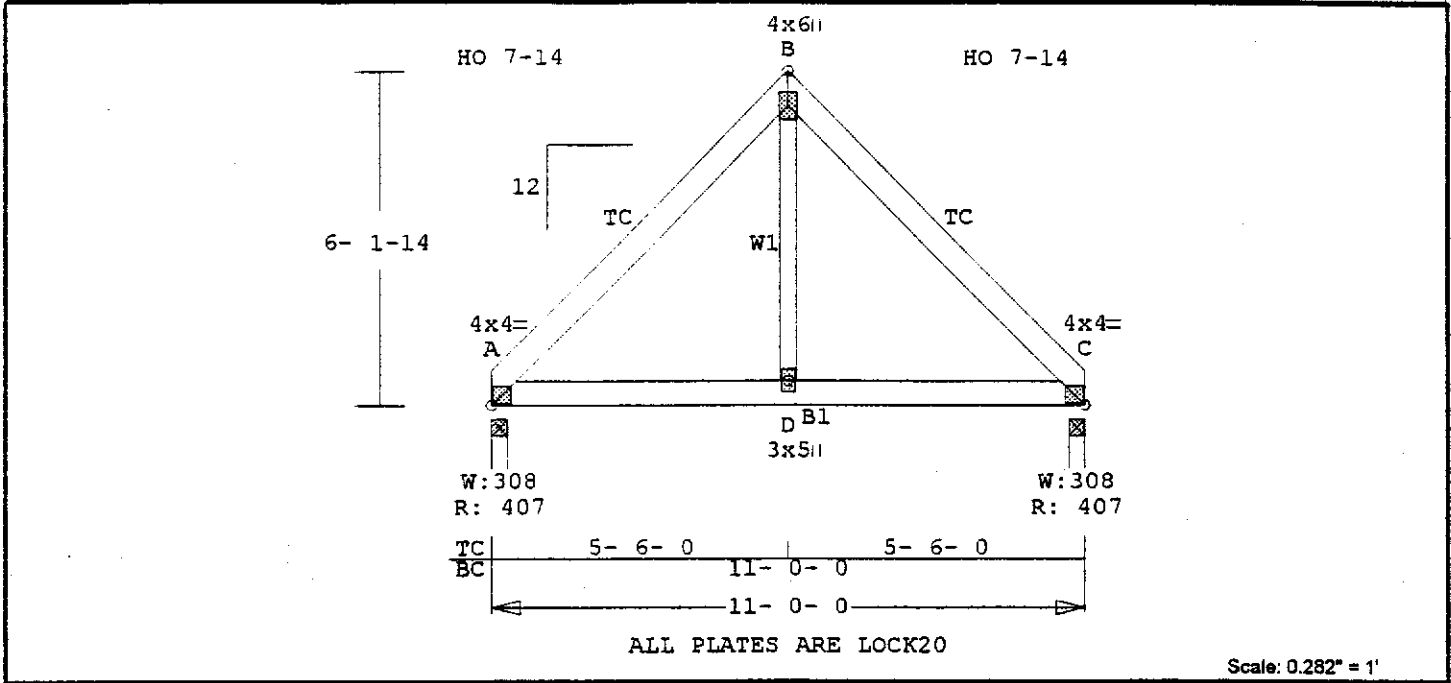
MEMBR	CSI	LEFT		RIGHT	
		P (LBS)	M21ST	M22ND	
TOP CHORDS					
A-O	.19	339 T	-416	-1747	
O-B	.18	176 C	-239	103	
B-CC	.16	169 T	-971	-147	
CC-II	.07	169 T	147	41	
II-C	.07	169 T	-41	205	
C-QQ	.05	174 T	-205	51	
QQ-M	.02	137 T	-51	5	
BOTTOM CHORDS					
A-P	.07	74 T	0	0	
P-S1	.02	74 T	0	0	
S1-F	.01	74 T	0	0	
F-DD	.01	74 T	0	0	
DD-JJ	.01	74 T	0	0	
JJ-E	.01	74 T	0	0	
E-RR	.01	74 T	0	0	
RR-N	.01	74 T	0	0	
WEBS					
N-M	.19	100 T	0	0	
P-O	=	368 C	F-B =	316 C	
DD-CC	=	164 C	JJ-II =	219 C	
E-C	=	.173 C	RR-QQ =	92 C	

DL+LL DEFL = .02" IN O-B  
LL DEFL < BRG-SPAN/360  
SPAN/DEFL (DL+LL) = 999

NOTES:

- TRUSSES MANUFACTURED BY - Walker Lumber
- ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
- EMPIRICAL ANALOG IS USED.
- DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
- WIND LOADS - ANSI/ASCE 7-98  
TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM  
WIND SPEED - 80 MPH  
MEAN ROOF HEIGHT - 25'  
EXPOSURE CATEGORY - C  
OCCUPANCY CATEGORY - 1  
ENCLOSED BUILDING.  
OCEANLINE DIST - 100 MILES  
TC DEAD LOAD = 14.0 PSF  
BC DEAD LOAD = 7.0 PSF
- NOTE LARGE BEARING SIZE.
- PROVIDE DRAINAGE TO PREVENT WATER PONDING.





APPROX. TRUSS WEIGHT: 87.4 LBS

Online Plus -- Version 10.0.001  
RUN DATE: 10-29-01

CSI	SIZE	LUMBER	1.15FB
TOP	.12	2X 6 DFL-#2	1345
BTM	.13	2X 6 DFL-#2	1345
WBS	.09	2X 4 STA-STUD	760

REPETITIVE MEMBER INCREASES:  
FB 15.0% FT .0% FC .0%

LATERAL BRACING:  
TOP CHORD - CONTINUOUS  
BTM CHORD - CONTINUOUS  
TRUSS SPACING - 24.0 IN.

STANDARD LOADING  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 407 3- 8 C 407 3- 8

LOAD CASE #1 UBC LL CHECK  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD .0 14.0  
BTM CHD 10.0 7.0  
TOTAL 10.0 21.0 31.0

SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 341 3- 8 C 341 3- 8

LOAD CASE #2 WIND FROM LEFT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
A-B 5.7N 14.0  
B-C -9.1N 14.0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN -153 210 3- 8  
C HORZ RLR 0 216 3- 8

LOAD CASE #3 WIND FROM RIGHT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
A-B -9.1N 14.0  
B-C 5.7N 14.0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN 153 213 3- 8  
C HORZ RLR 0 208 3- 8

LOAD CASE #4 WIND // RIDGE  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
A-B -16.9N 14.0  
B-C -16.9N 14.0

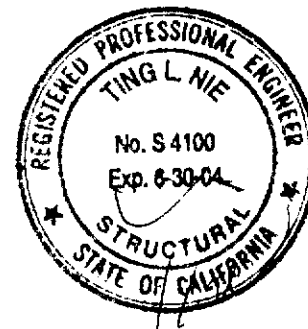
SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN 0 45 3- 8  
C HORZ RLR 0 45 3- 8

HEEL LEFT RIGHT  
0IN - 2SX 0IN - 2SX

MEMBR	CSI	P(LBS)	M#1ST	M#2ND
TOP CHORDS				
A-B	.12	306 C	830	-780
B-C	.12	306 C	780	-830
BOTTOM CHORDS				
A-D	.13	216 T	-665	-851
D-C	.12	216 T	851	665
WEBS				
D-B	=	217 T		

DL+LL DEFL = .03" IN A-B  
LL DEFL = .00" < BRG-SPAN/360  
SPAN/DEFL (DL+LL) = 999

- NOTES:
- TRUSSES MANUFACTURED BY - Walker Lumber
  - ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
  - EMPIRICAL ANALOG IS USED.
  - DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
  - WIND LOADS - ANSI/ASCE 7-98  
TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM  
WIND SPEED - 80 MPH  
MEAN ROOF HEIGHT - 25'  
EXPOSURE CATEGORY - C  
OCCUPANCY CATEGORY - 1  
ENCLOSED BUILDING.  
OCEANLINE DIST - 100 MILES  
TC DEAD LOAD = 14.0 PSF  
BC DEAD LOAD = 7.0 PSF



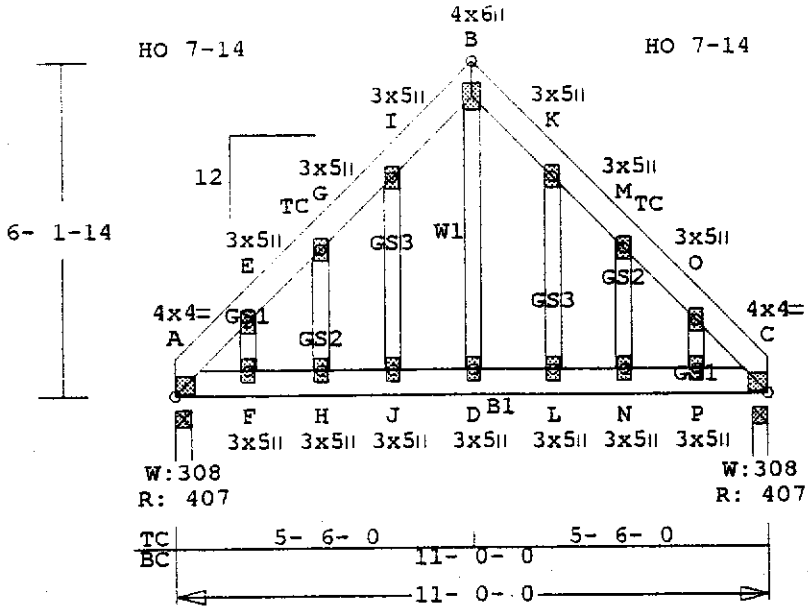
CHECKED NOV - 5 2001

Job  
cw023

Mark  
CAP4

Quan Type Span Pl-H1 Left OH Right OH  
1 2.2 110000 12 0 0

Engineering



ALL PLATES ARE LOCK20

Scale: 0.282" = 1'

APPROX. TRUSS WEIGHT: 114.2 LBS

Online Plus -- Version 10.0.001  
RUN DATE: 10-29-01

CSI	SIZE	LUMBER	1.15FB
TOP	.07	2X 6 DFL-#2	1345
BTM	.17	2X 6 DFL-#2	1345
WBS	.11	2X 4 STA-STUD	760

REPETITIVE MEMBER INCREASES:  
FB 15.0% FT .0% FC .0%

LATERAL BRACING:  
TOP CHORD - CONTINUOUS  
BTM CHORD - CONTINUOUS  
TRUSS SPACING - 24.0 IN.

STANDARD LOADING  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 407 3- 8 C 407 3- 8

LOAD CASE #1 UBC LL CHECK  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD .0 14.0  
BTM CHD 10.0 7.0  
TOTAL 10.0 21.0 31.0

SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 341 3- 8 C 341 3- 8

LOAD CASE #2 WIND FROM LEFT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
A-B 5.7N 14.0  
B-C -9.1N 14.0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN -153 210 3- 8  
C HORZ RLR 0 216 3- 8

LOAD CASE #3 WIND FROM RIGHT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
A-B -9.1N 14.0  
B-C 5.7N 14.0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN 153 213 3- 8  
C HORZ RLR 0 208 3- 8

LOAD CASE #4 WIND // RIDGE  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
A-B -16.9N 14.0  
B-C -16.9N 14.0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN 0 45 3- 8  
C HORZ RLR 0 45 3- 8

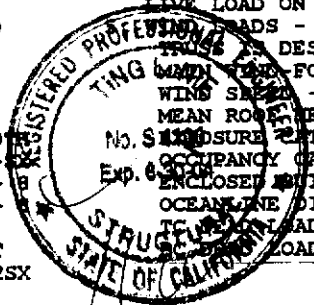
HEEL LEFT RIGHT  
0IN - 2SX 0IN - 2SX

MEMBR	CSI	P(LBS)	M#1ST	M#2ND
TOP CHORDS				
A-E	.05	344 C	40	430
E-G	.07	325 C	-430	648
G-I	.07	302 C	-648	346
I-B	.05	284 C	-346	-405
B-K	.05	284 C	405	346
K-M	.07	302 C	-346	648
M-O	.07	325 C	-648	430
O-C	.05	344 C	-430	-40
BOTTOM CHORDS				
A-F	.11	223 T	165	739
F-H	.13	223 T	-739	1106
H-J	.13	223 T	-1106	414
J-D	.17	223 T	-414	-1750
D-L	.17	223 T	1750	414
L-N	.12	223 T	-414	1106
N-P	.12	223 T	-1106	739
P-C	.09	223 T	-739	-165

WEBS  
F-E = 41 C H-G = 48 C  
J-I = 58 C D-B = 262 T  
L-K = 58 C N-M = 48 C  
P-O = 41 C

DL+LL DEFL = .02" AT G  
LL DEFL = .01" < BRG-SPAN/360  
SPAN/DEFL (DL+LL) = 999

- NOTES:
1. TRUSSES MANUFACTURED BY - Walker Lumber
  2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
  3. EMPIRICAL ANALOG IS USED.
  4. DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
- DESIGNED AS A FORCE RES SYSTEM  
WIND SPEED - 80 MPH  
MEAN ROOF HEIGHT - 25'  
OCCUPANCY CATEGORY - C  
ENCLOSED BUILDING.  
OCEAN WAVE DIST - 100 MILES  
IC LOAD = 14.0 PSF  
D LOAD = 7.0 PSF



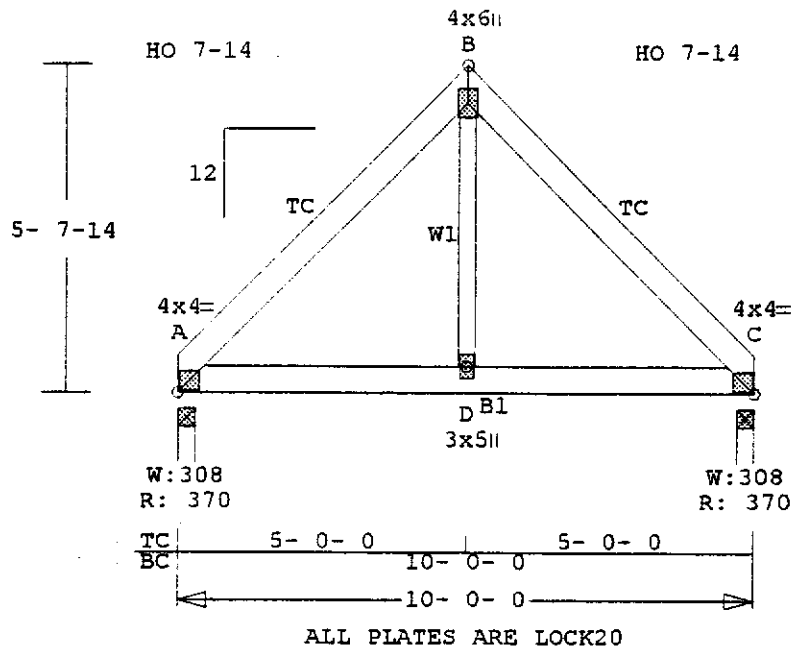
CHECKED NOV - 5 2001

Job  
cw023

Mark  
CAP2

Quan Type Span Pl-H1 Left OH Right OH  
3 2.2 100000 12 0 0

Engineering



Scale: 0.301" = 1'

APPROX. TRUSS WEIGHT: 79.6 LBS

Online Plus -- Version 10.0.001  
RUN DATE: 10-29-01

CSI	SIZE	LUMBER	1.15FB
TOP	.10	2X 6 DFL-#2	1345
BTM	.10	2X 6 DFL-#2	1345
WBS	.08	2X 4 STA-STUD	760

REPETITIVE MEMBER INCREASES:  
FB 15.0% FT .0% FC .0%

LATERAL BRACING:  
TOP CHORD - CONTINUOUS  
BTM CHORD - CONTINUOUS  
TRUSS SPACING - 24.0 IN.

STANDARD LOADING  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 370 3- 8 C 370 3- 8

LOAD CASE #1 UBC LL CHECK  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD .0 14.0  
BTM CHD 10.0 7.0  
TOTAL 10.0 21.0 31.0

SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 310 3- 8 C 310 3- 8

LOAD CASE #2 WIND FROM LEFT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0  
EXCEPTIONS:  
A-B 5.7N 14.0  
B-C -9.1N 14.0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN -138 191 3- 8  
C HORZ RLR 0 196 3- 8

LOAD CASE #3 WIND FROM RIGHT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0  
EXCEPTIONS:  
A-B -9.1N 14.0  
B-C 5.7N 14.0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN 138 194 3- 8  
C HORZ RLR 0 189 3- 8

LOAD CASE #4 WIND // RIDGE  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0  
EXCEPTIONS:  
A-B -16.9N 14.0  
B-C -16.9N 14.0

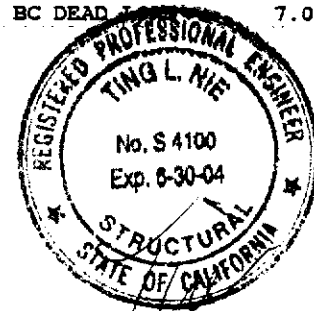
SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN 0 40 3- 8  
C HORZ RLR 0 40 3- 8

LEFT RIGHT  
HEEL 0IN - 2SX 0IN - 2SX

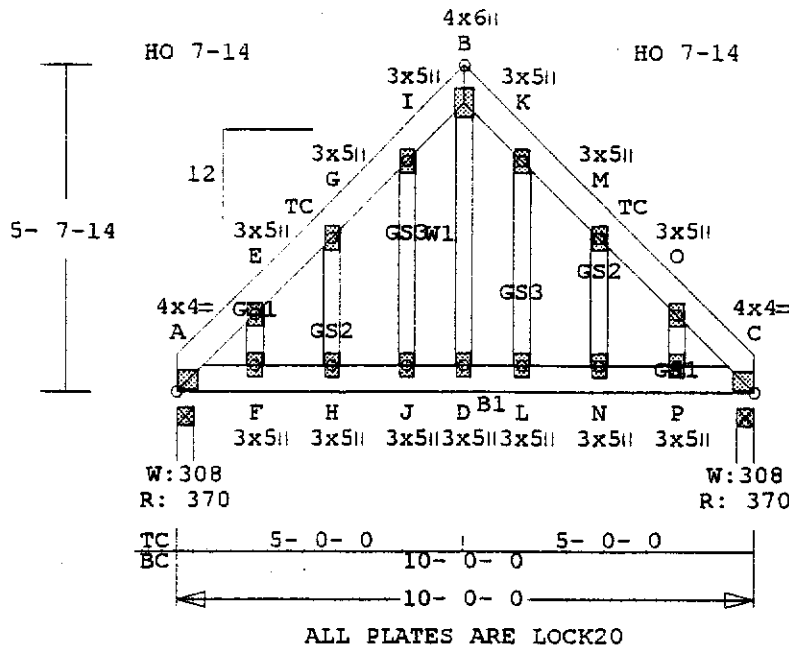
MEMBR	CSI	F(LBS)	M01ST	M02ND
TOP CHORDS				
A-B	.10	275 C	656	-600
B-C	.10	275 C	600	-656
BOTTOM CHORDS				
A-D	.10	194 T	-515	-665
D-C	.10	194 T	665	515
WEBS				
D-B	=	195 T		

DL+LL DEFL = .02" IN A-B  
LL DEFL = .00" < BRG-SPAN/360  
SPAN/DEFL (DL+LL) = 999

NOTES:  
1. TRUSSES MANUFACTURED BY - Walker Lumber  
2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).  
3. EMPIRICAL ANALOG IS USED.  
4. DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.  
5. WIND LOADS - ANSI/ASCE 7-98 TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM WIND SPEED - 80 MPH MEAN ROOF HEIGHT - 25' EXPOSURE CATEGORY - C OCCUPANCY CATEGORY - 1 ENCLOSED BUILDING. OCEANLINE DIST - 100 MILES TC DEAD LOAD = 14.0 PSF BC DEAD LOAD = 7.0 PSF



CHECKED NOV - 5 2001



Scale: 0.301" = 1'

APPROX. TRUSS WEIGHT: 106.4 LBS

Online Plus -- Version 10.0.001  
RUN DATE: 10-29-01

CSI	SIZE	LUMBER	1.15FB
TOP	.06	2X 6 DFL-#2	1345
BTM	.13	2X 6 DFL-#2	1345
WBS	.09	2X 4 STA-STUD	760

REPETITIVE MEMBER INCREASES:  
FB 15.0% FT .0% FC .0%

LATERAL BRACING:  
TOP CHORD - CONTINUOUS  
BTM CHORD - CONTINUOUS  
TRUSS SPACING - 24.0 IN.

STANDARD LOADING  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 370 3-8 C 370 3-8

LOAD CASE #1 UBC LL CHECK  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD .0 14.0  
BTM CHD 10.0 7.0  
TOTAL 10.0 21.0 31.0

SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 310 3-8 C 310 3-8

LOAD CASE #2 WIND FROM LEFT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
A-B 5.7N 14.0  
B-C -9.1N 14.0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN -138 191 3-8  
C HORZ RLR 0 196 3-8

LOAD CASE #3 WIND FROM RIGHT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
A-B -9.1N 14.0  
B-C 5.7N 14.0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN 138 194 3-8  
C HORZ RLR 0 189 3-8

LOAD CASE #4 WIND // RIDGE  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD .0 7.0  
TOTAL 16.0 21.0 37.0

EXCEPTIONS:  
A-B -16.9N 14.0  
B-C -16.9N 14.0

SUPPORT CRITERIA  
JT TYPE HORZ VERT WIDTH  
LBS LBS IN-SX  
A PIN 0 40 3-8  
C HORZ RLR 0 40 3-8

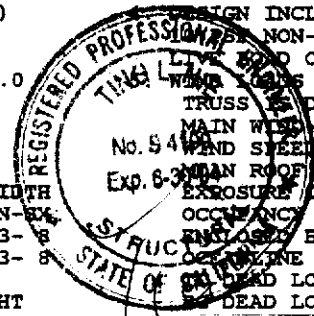
HEEL LEFT RIGHT  
0IN - 2SX 0IN - 2SX

MEMBR	CSI	P(LBS)	M01ST	M02ND
TOP CHORDS				
A-E	.05	308 C	33	377
E-G	.06	288 C	-377	489
G-I	.06	272 C	-489	248
I-B	.03	245 C	-248	-329
B-K	.03	245 C	329	248
K-M	.05	272 C	-248	489
M-O	.06	288 C	-489	377
O-C	.04	308 C	-377	-33
BOTTOM CHORDS				
A-F	.09	200 T	143	663
F-H	.11	200 T	-663	912
H-J	.11	200 T	-912	-65
J-D	.13	200 T	65	-1354
D-L	.13	200 T	1354	-65
L-N	.10	200 T	65	912
N-P	.10	200 T	-912	663
P-C	.08	200 T	-663	-143

WEBS  
F-E = 40 C H-G = 58 C  
J-I = 36 T D-B = 229 T  
L-K = 36 T N-M = 58 C  
P-O = 40 C

DL+LL DEFL = .01" AT G  
LL DEFL = .01" < BRG-SPAN/360  
SPAN/DEFL (DL+LL) = 999

- NOTES:
1. TRUSSES MANUFACTURED BY - Walker Lumber
  2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
  3. EMPIRICAL ANALOG IS USED.



DESIGN INCLUDES CHECK FOR NON-CONCURRENT LOADS ON BOTTOM CHORD.  
TRUSS IS DESIGNED AS A MAIN WIND FORCE RES SYSTEM  
WIND SPEED - 80 MPH  
SPAN ROOF HEIGHT - 25'  
EXPOSURE CATEGORY - C  
OCCUPANCY CATEGORY - 1  
FASTENING BUILDING.  
WIND DIST - 100 MILES  
DEAD LOAD = 14.0 PSF  
DEAD LOAD = 7.0 PSF

CHECKED NOV - 5 2001