CITY OF SACRAMENTO

1231 I Street, Sacramento, CA 95814

Permit No: 0102429
Insp Area: 1
Thos Bros: 297H4

Site Address: 3534 D ST SAC
Parcel No: 004-0172-024

Sub-Type: NGAR
Housing (Y/N): N

CONTRACTOR
MARK BAKER
216 32ND ST
SAC CA 95816

OWNER
BAY RICHARD J
3534 D ST
SACRAMENTO CA 95816

ARCHITECT

Nature of Work: DEMO DETACHED GARAGE & BUILD NEW GARAGE WITH REDUCED SETBACK(372 SF)

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name_____ Lender'sAddress_____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

X License Class B License Number 607462 X Date 10-3-01 X Contractor Signature Mark Baker

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars ($500.00);

_____ I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

_____ I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

RECEIVED
CITY OF SACRAMENTO
OCT 04 2001
PLANNING

_____ I am exempt under Sec._____ B & PC for this reason:_____

Date_____ Owner Signature_____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and herby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

XDate 10-3-01 XApplicant/Agent Signature Mark Baker

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:
_____ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, forthe performance of work for which the permit is issued.

_____ I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier Policy Number Exp Date

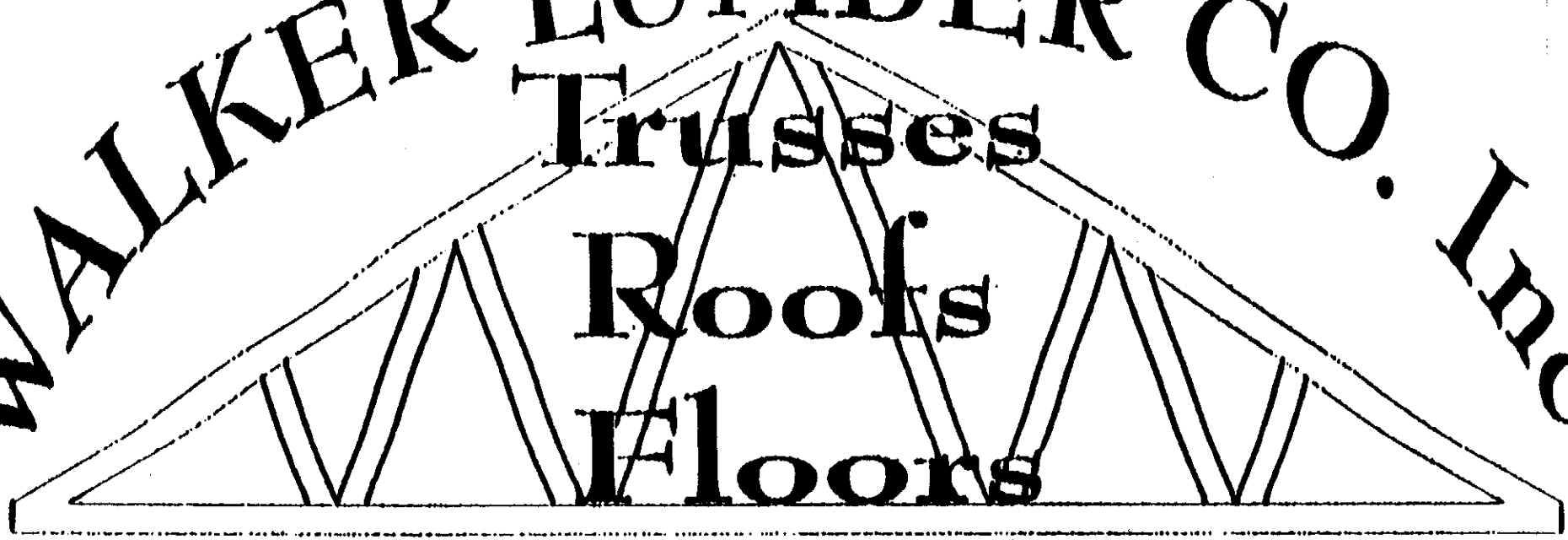
X/S (This section need not be completed if the permit is for $100 or less) I certify that in the performance of the work for which this permit is issued,I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

XDate 10-3-01 XApplicant Signature Mark Baker

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS ($100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

WALKER LUMBER CO. Inc.



Http://www.walker-lumber.com
(916) 338-2121 Fax (916) 338-5353

Where Technology Meets Common Sense

Permit
01-02429

Serving Sacramento For Over 25 Years

OK/JR

Truss Designed Specifically For	3534-D St	Job Name	Job Number
Baker Contracting		3534 C Street	LW023

WALKER LUMBER CO., Inc

TRUSSES
ROOFS
& FLOORS

Commercial, Residential or Custom

Truss Maintenance and Guarantee Instruction

Project: 3534 D Street Date: 11-7-01

Contractor: Baker Contracting

Trusses are guaranteed to meet specifications on approved drawings and can not be guaranteed unless all items listed are complied with, whether written or implied.

1. Provide proper ventilation in all areas at all times.
2. Restraint of the structures lateral bracing is to be designed, installed and verified by job engineer, contractor & owner.
3. Provide a proper and adequate vapor barrier in all appropriate locations.
4. Do not expose truss to the weather. If any part of the truss is exposed to the weather it must be properly protected and maintained.
5. Owner and contractor must protect trusses from loads above the designed loads shown on the drawings.
6. Do not cut or otherwise alter the truss, its members, or support points.
7. Provide approved walk ways to mechanical units or unit that require service.
8. Owner and contractor to verify loads on the drawings are adequate for the building department load requirements and the requirements of the application.
9. All lateral bracing indicated on the approved truss drawings must be installed prior to loading of any trusses.
10. Trusses must be properly installed right side up at the proper bearing locations with the approved bracing and fasteners.
11. Truss bottom chords should not be restrained or permanently fastened down in any manner to prevent movement different from adjacent trusses other than support points.
12. Do not over load Trusses with any materials (units of sheeting, roof materials, sheet rock etc.)
13. Failure to comply with these requirements, all local building codes, specifications on plans, construction documents and approved truss drawings will void all guarantee's.

Thank You

Mike Walker
Mike Walker Lumber Company, Inc.

PO BOX 958 6915 30th STREET NORTH HIGHLANDS, CA. 95660
916 338-2121 FAX 916 338-5353 mikew@walker-lumber.com

ICBO Evaluation Service, Inc.

5360 WORKMAN MILL ROAD • WHITTIER, CALIFORNIA 90601-2299

A subsidiary corporation of the International Conference of Building Officials

EVALUATION REPORT

ER-4994

Reissued November 1, 1997

Filing Category: FASTENERS—Steel Gusset Plates (066)

ROBBINS LOCK (RL) AND ROBBINS HIGH STRENGTH (RHS) METAL CONNECTOR PLATES FOR WOOD TRUSSES

ROBBINS MANUFACTURING CO.
POST OFFICE BOX 17939
TAMPA, FLORIDA 33682-7939

1.0 SUBJECT

Robbins Lock (RL) and Robbins High Strength (RHS) Metal Connector Plates for Wood Trusses.

2.0 DESCRIPTION

2.1 General:

Robbins Lock (RL) and Robbins High Strength (RHS) plates are metal connector plates for wood trusses. The plates are manufactured from galvanized steel in various lengths and widths, and have integral teeth that are designed to laterally transmit loads between truss wood members. Plans and calculations must be submitted to the building official for the trusses using metal connector plates described in this report.

2.2 Materials:

Robbins Lock (RL) plates are manufactured from No. 20 gage [0.035 inch (0.89 mm)], ASTM A 653, Grade 40, structural-quality steel with a hot-dipped galvanized coating designated G60.

Robbins High Strength (RHS) plates are manufactured from No. 20 gage [0.035 inch (0.89 mm)], ASTM A 653 HSLA, Type I, Grade 60, high-strength, structural-quality steel with a hot-dipped galvanized coating designated G60.

Both the Robbins Lock (RL) and Robbins High Strength (RHS) plates have slots approximately $^1/_2$ inch (12.7 mm) long by $^1/_8$ inch (3.2 mm) wide that have been punched along the longitudinal axis of the plate. Each punched slot forms two opposite-facing, sharply pointed teeth protruding at right angles from the parent metal. The punched slots are spaced approximately $^1/_4$ inch (6.4 mm) on center across the width of the plate and approximately 1 inch (25.4 mm) on center along the length of the plate, with adjacent longitudinal rows staggered $^1/_2$ inch (12.7 mm). Connector plates are available in 1-inch (25.4 mm) increments of width and length. Minimum plate width and length are 1 inch (25.4 mm) and 3 inches (76 mm), respectively. See Figure 1 for details of plate dimensions.

There are eight teeth per square inch (1.24 teeth per square centimeter) of plate surface. The length of each tooth, including the thickness of the parent metal, is approximately $^3/_8$ inch (9.5 mm), and the width of each tooth is approximately $^1/_8$ inch (3.2 mm). The shank of each tooth is concave and the tip of each tooth is twisted approximately 40 degrees with respect to the plate width. Adjacent longitudinal rows of teeth are twisted in opposite directions.

2.3 Allowable Loads:

Tables 1, 2 and 3 provide allowable lateral loads, tension loads and shear loads for the RL and RHS plate connectors. These values are based on the National Design Standard for Metal Plate Connected Wood Truss Construction, ANSI/TPI 1-1995. A copy of the ANSI/TPI 1-1995 standard must be supplied to the building department when this is requested by the building official.

2.3.1 Lateral Resistance: Each metal connector plate must be designed to transfer the required load without exceeding the allowable load per square inch of plate contact area, which is based on species, the orientation of the teeth relative to the load, and the direction of load relative to grain. Design for lateral resistance must be in accordance with Section 11.2.1 of ANSI/TPI 1-1995. Table 1 provides allowable lateral loads for the metal connector plates.

2.3.2 Tension Resistance: Each metal connector plate must be designed for tension capacity that is based on the orientation of the metal connector plate relative to the direction of load. Design for tension must be in accordance with Section 11.2.2 of ANSI/TPI 1-1995. Table 2 provides allowable tension loads for the metal connector plates. Additionally, the net section of the metal connector plates for tension joints must be designed using the allowable tensile stress values of the metal that are adjusted by the metal connector plate tensile effectiveness ratios shown in Table 2.

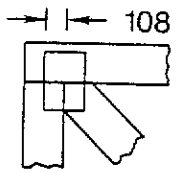
2.3.3 Shear Resistance: Each metal connector plate must be designed for shear capacity that is based on the orientation of the plate relative to all possible lines of shear. Design for shear must be in accordance with Section 11.2.3 of ANSI/TPI 1-1995. Table 3 provides allowable shear loads for the metal connector plates. Additionally, the net section of the metal connector plates for heel joints, and other joints involving shear, must be designed using the allowable shear values for the metal connector plates that are adjusted by the shear resistance effectiveness ratios shown in Table 3.

2.3.4 Metal Plate Reductions: Several allowable-load reduction factors for the metal plates, when they are applicable, must be considered cumulatively in the design of metal connector plates used in fabricated wood trusses:

1. Allowable lateral resistance values for the RL and RHS metal connector plates must be reduced by 15 percent when the plates are installed on the narrow face of truss lumber members.
2. Allowable lateral resistance values must be reduced by 20 percent when the RL and RHS metal connector plates are installed in lumber having a moisture content greater than 19 percent at the time of truss fabrication.

Symbols

PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or in IN-16ths (i.e. 108).

PLATE ORIENTATION

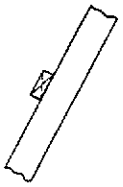
When shown, indicates direction of slots in connector plate.

PLATE SIZE

6.3 x 8.8

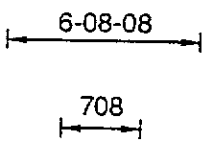
The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.

LATERAL BRACING



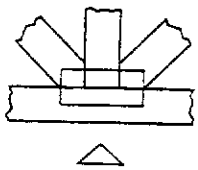
Indicates approximate location of continuous lateral bracing required for stability of individual member.

DIMENSIONS



All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).

BEARING



Indicates support (bearing) at joint.

NOTICE:

This truss designed at request and specification of customer as an individual building component, in a vertical plane, to be incorporated into building design at the specification of the building designer. Bracing shown is for lateral support of individual members only. Temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. Design and materials are in accordance with latest editions of **NDS, HUD Design Criteria for Trussed Rafters,** and **TPI** specifications. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult **Quality Control Manual, Bracing of Wood Trusses** (HIB-91) and **Recommended Code of Standard Practice**, available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

General Notes:

1. Provide copies of this truss design to the building designer and erection supervisor.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and seat securely. Avoid knots and wane at joint locations.
4. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
5. Unless specifically noted, this design is not applicable for use with fire retardant or preservative treated lumber.
6. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively.
7. Anchorage and/or tie-in components are the responsibility of others, unless shown.
8. Do not stack construction materials on floor or roof that induces loading on truss greater than design loads.
9. Do not cut or alter truss without prior approval of a professional engineer.
10. Building designer is responsible to insure that loading shown hereon is applicable to building site.
11. Care should be exercised in handling, erection, and installation of trusses.
12. Camber is a non-structural consideration and is the responsibility of the truss fabricator. General practice is to camber for dead load deflection.
13. Refer to On-Line Data's Ch. VI-B for joint details.

MIKE WALKER LUMBER
P.O. Box 958
North Highlands, CA 95660

ROBBINS ENGINEERING, INC.
Bringing you the best of both worlds

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P.O. Box 280055
Tampa, FL 33682-0055
(800)282-1299/ FAX (813)971-6117

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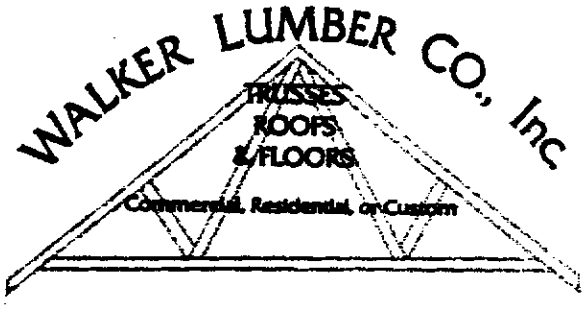
Auburn, WA 98001
(800)532-9404/ FAX (206)735-2238

1177 Rockingham
Richardson, TX 75080
(214)238-9609/ FAX (214)437-0467



July 10, 2001

To: All Customers

From: Steve Kimbrell

RE: Safe Crane Operations

In order to ensure safe crane operation and to continue to deliver trusses efficiently, we would like to clarify our current delivery procedure.

OSHA regulation 5002 requires that:

"Operations shall be conducted and the job controlled in a manner that will avoid exposure of employees to the hazard of overhead loads. Wherever loads must be passed directly over workers, occupied work spaces or occupied passageways, safety type hooks or equivalent means of preventing the loads from becoming disengages shall be used."

There are many situations when there is no one available on the plate line to set trusses and assist the crane operator in releasing the safety hooks on the rigging. In these situations, the vicinity (to include the house and within 100 feet surrounding the truck and crane) would need to be completely evacuated. As stated in the OSHA regulation, safety latches are required only when the load is being passed over workers, occupied spaces or occupied passageways.

In all other situations, the safety latches on the rigging must be used. It is the responsibility of the customer or personal in charge to provide a person on the plate line to assist with setting the trusses and unhooking the safety latches.

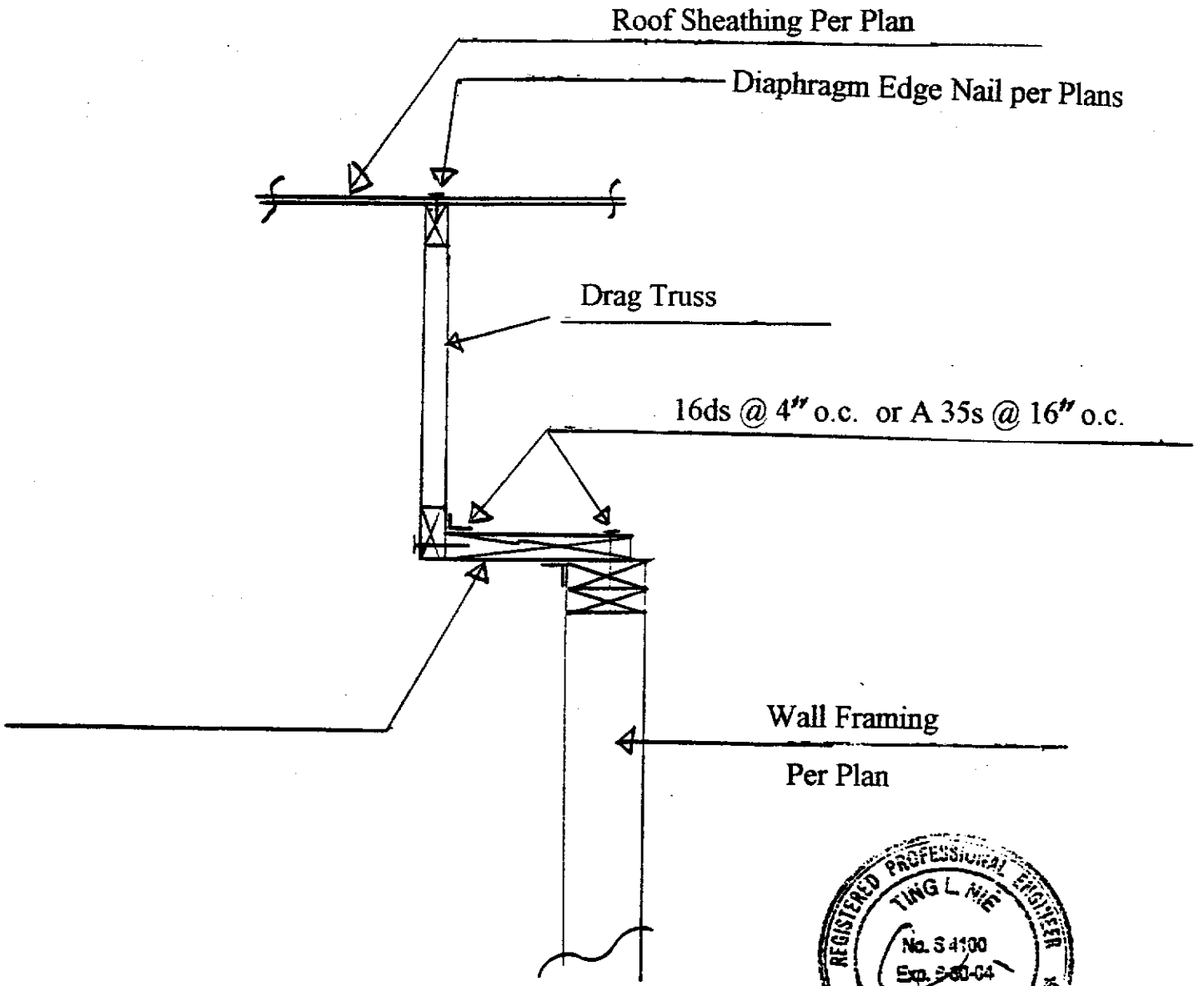
If the customer or person in charge is unable or unwilling to provide a person on the plate line, there are two options:

1) The building/house can be completely evacuated as stated above to allow for the safe operation of the crane.
2) The trusses can be placed on the ground.

You have the support of the management of Mike Walker Lumber to fully implement this policy. Please notify Steve Kimbrell of any issues or concerns you have to make all deliveries safe.

PO Box 958 6915 30th Street North Highlands, CA. 95660
916 338-2121 Fax 916 338-5353 mikew@walker-lumber.com

ALTERNATE SHEAR TRANFSER



REGISTERED PROFESSIONAL ENGINEER
TING L. NIE
No. S 4100
Exp. 3-30-04
STRUCTURAL
STATE OF CALIFORNIA

Not to Scale
By: Raymond

Note: This detail is capable of transferring
A lateral load up to 340 PLF

Certificate of Quality

Western Inspection Service, Inc.
P.O. Box 3426
Gillette, WY 82717

We are an inspection agency recognized by the International Conference of Building Officials. I.C.B.O. Report No. AA-583.

This is to certify that:

WALKER LUMBER COMPANY, INC.
NORTH HIGHLANDS, CALIFORNIA

is under our Audited Quality Control Program and has been since:

April 1, 2001

We audit the production under UBC and IBC Structural Engineering.

UBC Section **U.B.C. 97 Section 2304.4.4 & ANSI/TPI 2321.3**

IBC Section **I.B.C. Section 2303.4**

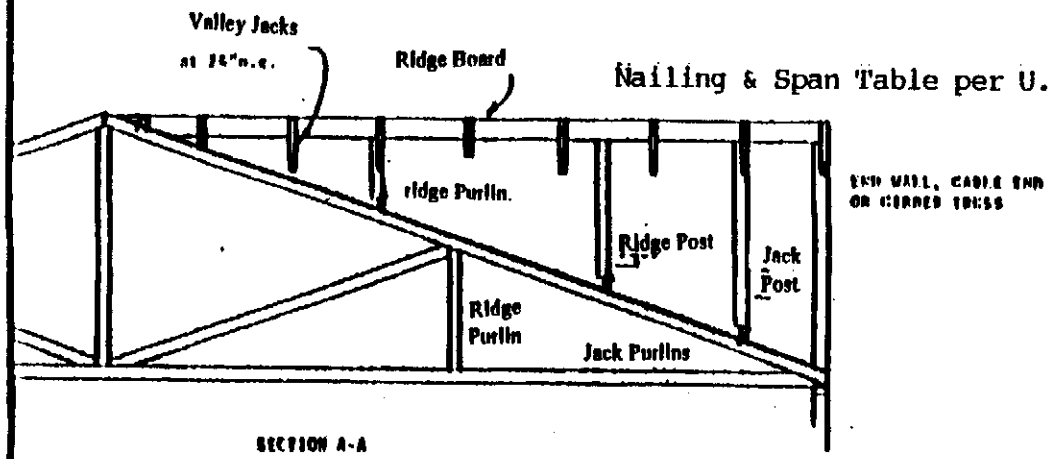
Inspections are made at least Quarterly and we make intermittent inspections of trusses in the field. We find quality observance of this plant to be in Conformance to the Standards.

WALKER LUMBER CO., INC.
Manufacturer

Keith H Anderson, Vice President
Western Inspection Services, Inc.

Detail for conventionally framed Valley Fill

Valley Jacks	Spaced 24" o.c. clear span not to exceed 8' – 0" (DFL#2 or better)
Valley Pads	See Plans
Jack Post	2 x 4's to be placed under Valley jacks at no more than 8' –0" o.c. and nailed to Jack Purlins.
Jack Purlins	2 x 4' – 0" spanning over three Trusses underneath and parallel to valley jacks to which the jack post supports are nailed.
Ridge Board	Same Species as jacks, one size greater.
Ridge Post	2 x 4's to be placed under ridge board at no more than 5' – 0" o.c. and nailed to ridge Purlin.
Ridge Purlin	2 x 4 Whose length is ½ the valley width at the point indicated by a Ridge Post.

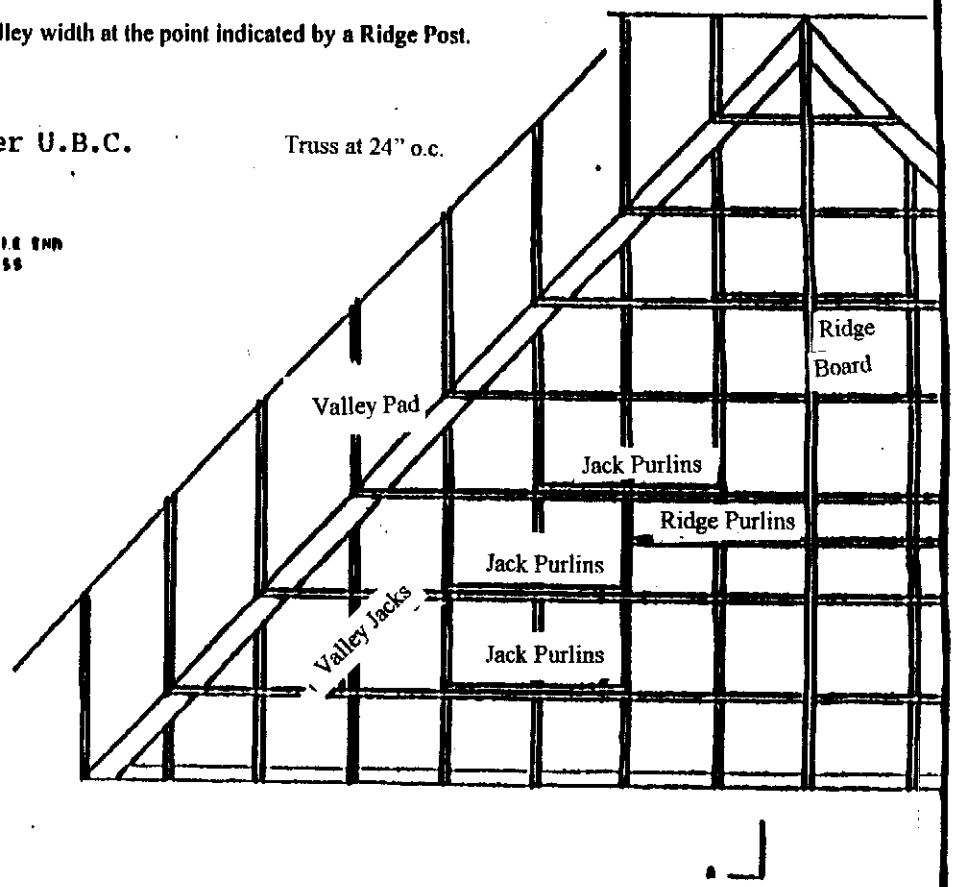


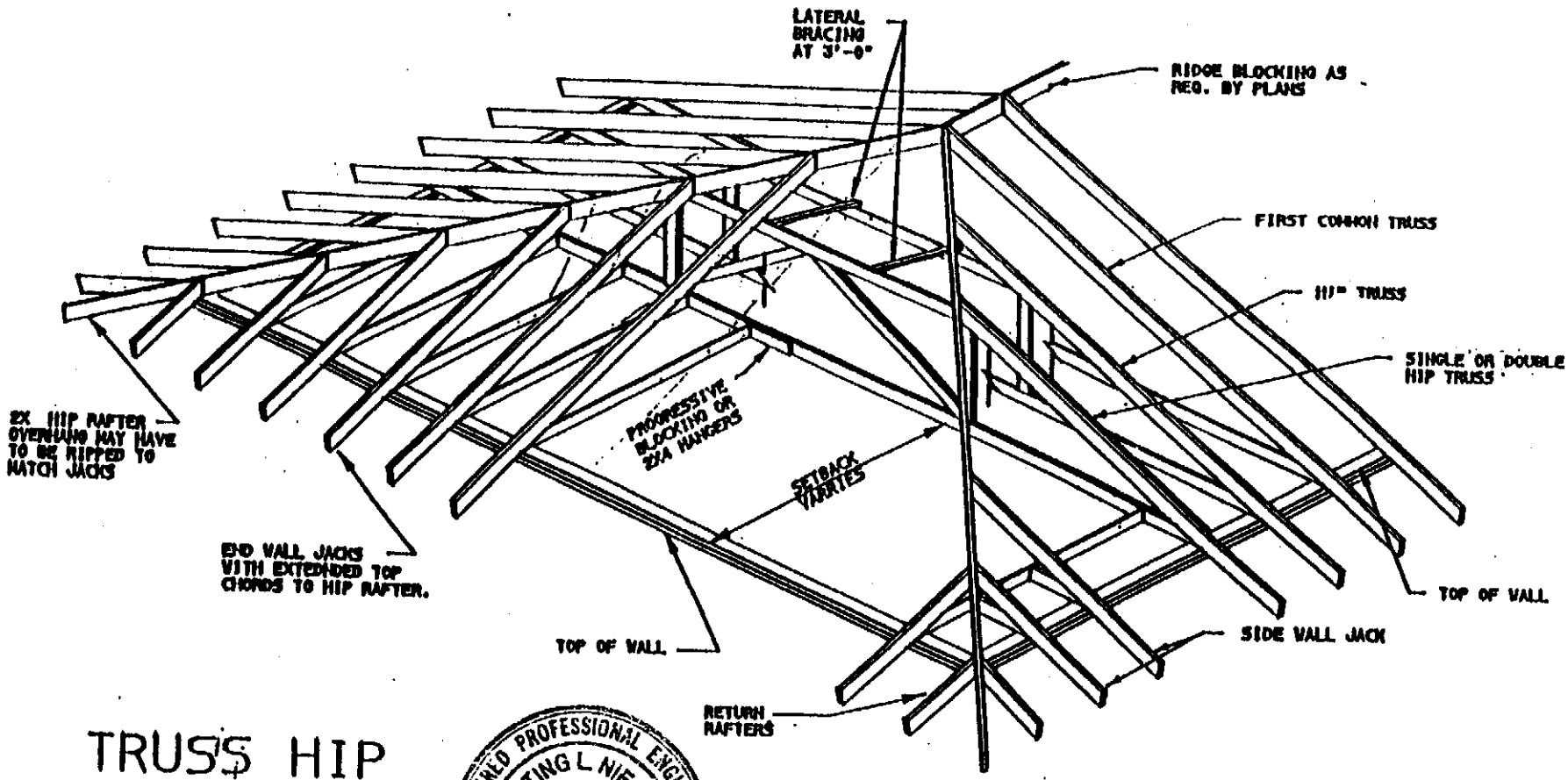
SECTION A-A

Nailing & Span Table per U.B.C.

Limitation: DL = 16 PSF

LL = 16 PSF

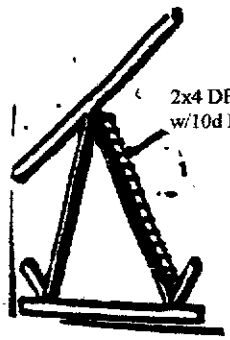




TRUSS HIP SYSTEM

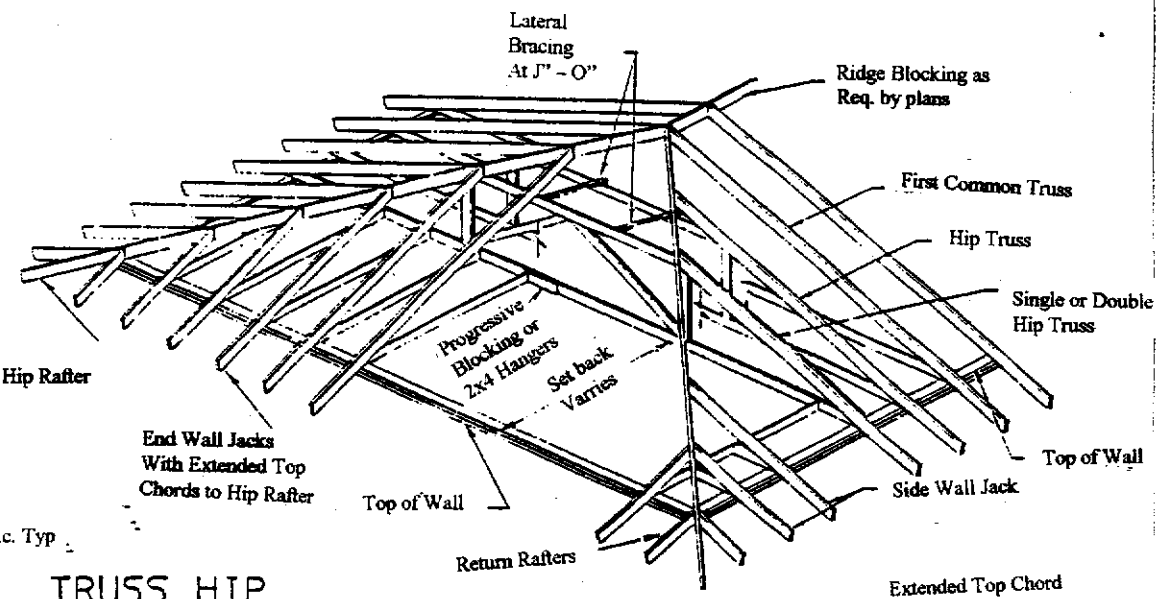
REGISTERED PROFESSIONAL ENGINEER
TING L NIE
No. S 4100
Exp. 6-30-04
STRUCTURAL
STATE OF CALIFORNIA

This Detail is to be used
As an alternate for
Continuous lateral bracing

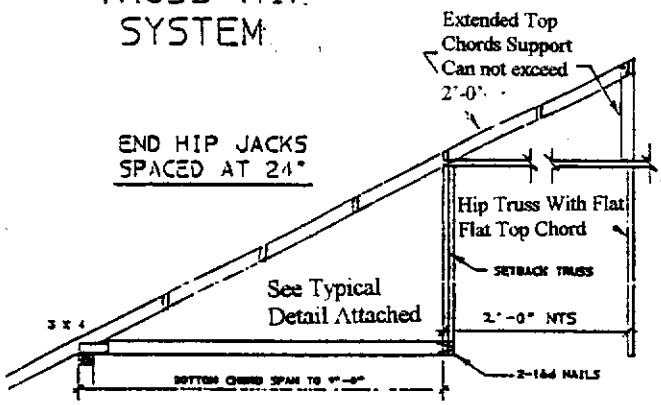


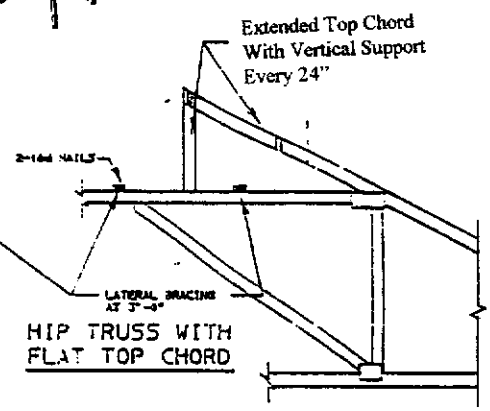
2x4 DF #2 brace
w/10d Nails @ 4" o.c. Typ

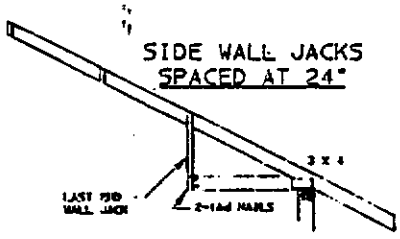
Note: Brace must be 90% the length of the member.

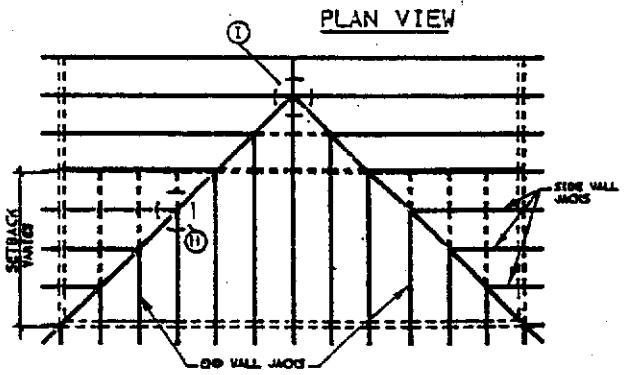


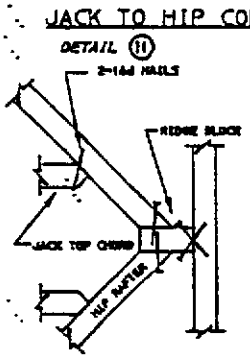
TRUSS HIP SYSTEM



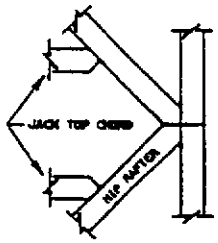




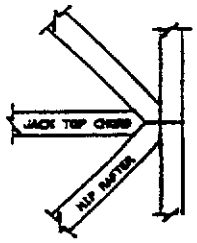




HIP TO TRUSS CONNECTION
DETAIL 1



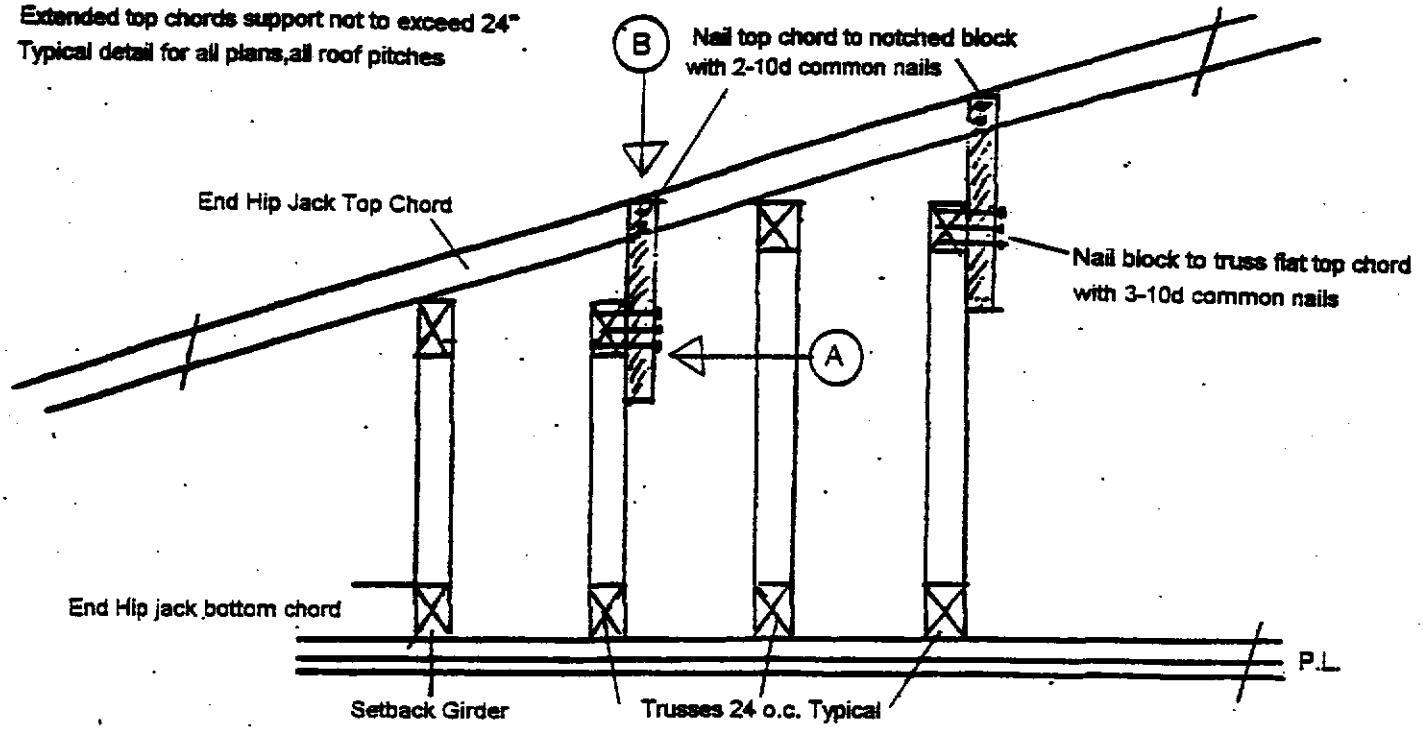
HIP TO TRUSS CONNECTION
DETAIL 1



HIP TO TRUSS CONNECTION
DETAIL 1

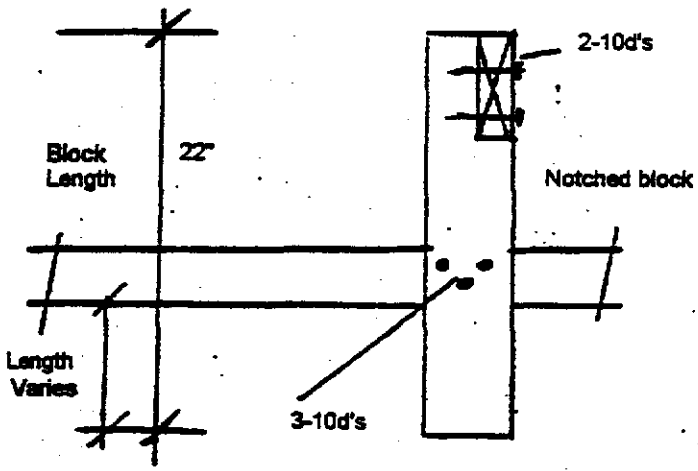
Re: End Hip Jacks Spaced At 24" O.C. Typical Top Chord Extension Support Detail.
Project: Typical for all Jobsites
Date: 12-14-00

Extended top chords support not to exceed 24"
Typical detail for all plans,all roof pitches



Elevation

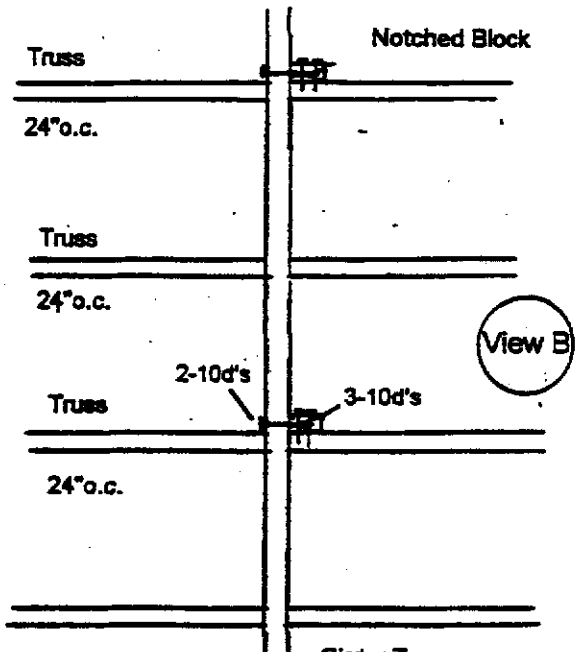
End Jack Top Chord Extension



Truss Flat Top Chord

View A

End Jack Top Chord Extension



Details Not To Scale

WARNING

DO NOT INSTALL THESE TRUSSES BEFORE READING INSTRUCTIONS.

ERECTOR

DO NOT INSTALL THESE TRUSSES BEFORE READING AND COMPLYING WITH THESE INSTRUCTIONS.

GENERAL CONTRACTOR

DO NOT LOAD THESE TRUSSES WITH PERMANENT OR TEMPORARY LOADS BEFORE READING THESE INSTRUCTIONS.

1. Before Erector These Trusses Erector Should:
   A. Obtain Study and Comply with "Bracing Wood Trusses" (BWT 76) as published by TPI. Obtain this guide from: The fabricator or dealer who provided the trusses or TPI.7411 Riggs Road. Hyattsville Maryland 20783

Alternate

   B. Obtain complete erection bracing plan from building architect. Engineer or structurally competent designer.

Caution

Erector is responsible for all damages or injury as a result of inadequate bracing failures occurring during erection and prior to installation of permanent bracing. Compliance with this industry guide is in your best interest.

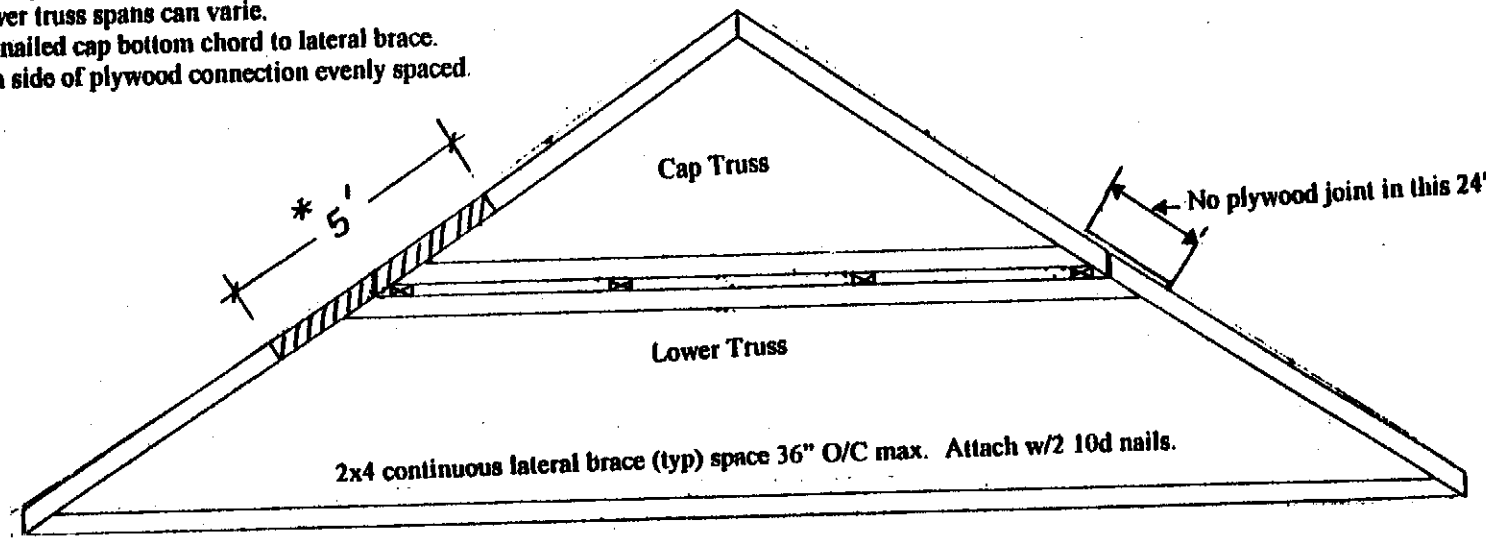
(Over)

Cap Truss Connection Details

1) This detail is for any 24" O/C truss.
2) Lower truss may have flat or pitched bottom chord.
3) Composition shake or tile loads can be used.
4) ½" CDX Plywood or O.S. B.minimum at connection for roof sheeting.
5) Use 8d nails with ½" plywood, 10d nails with ¾" plywood.
6) Both upper and lower truss spans can varie.
7) Use 1 16d nail toe-nailed cap bottom chord to lateral brace.
8) Use 3 8d nails each side of plywood connection evenly spaced.

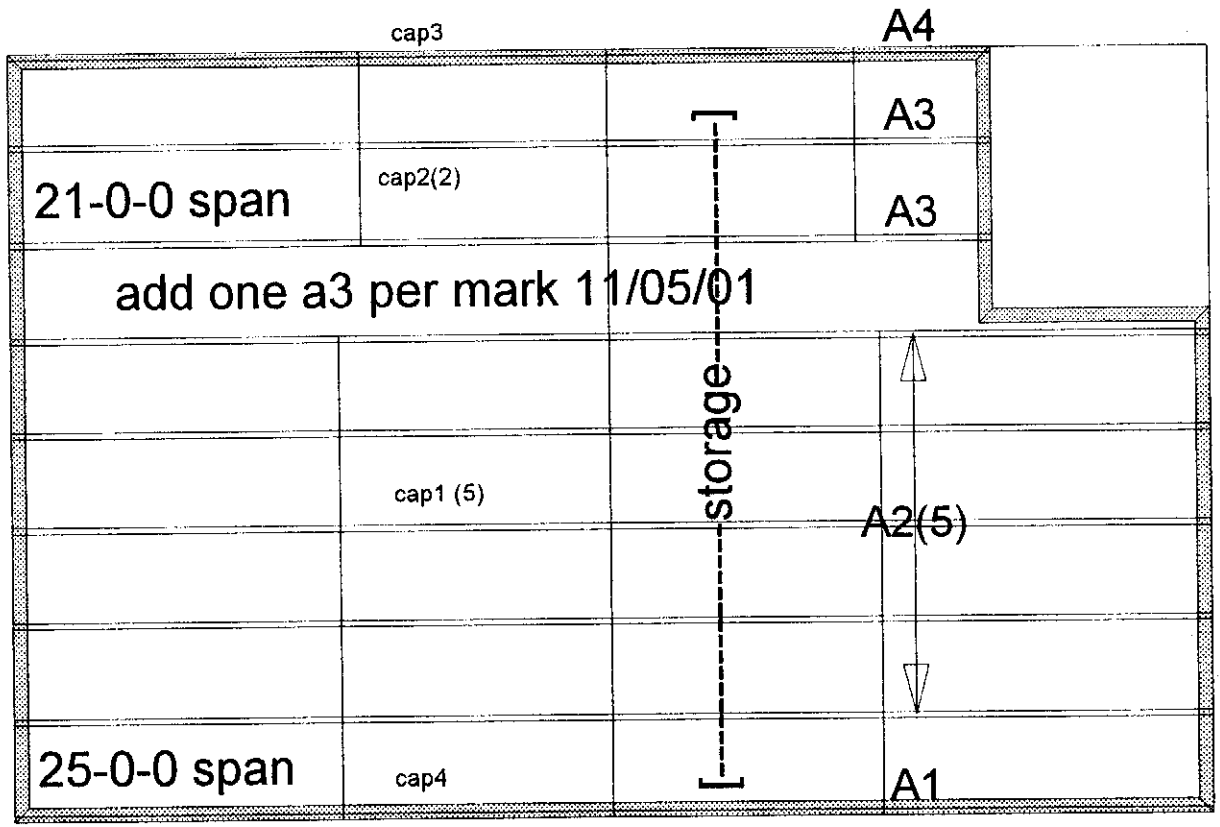
NOTES:

1) Plywood roof sheeting joint's should not occur with in 12" of intersections of upper truss to lower truss.
2) In lieu of 24" min. plywood detail, use 24" 16g. coil strap over plywood not shown. Center on cap, main truss seam. Attach w/8d comm. Nails. (fill all holes).
3)* At lateral load or collector load connection, scab one face of truss w/2x4x5 dfst as shown. Attach w/16d common nails 3" O.C.



WALKER LUMBER CO., Inc
TRUSSES
ROOFS
& FLOORS
Commercial, Residential, or Custom

PO BOX 958 6915 30th STREET NORTH HIGHLANDS, CA. 95660
916 338-2121 FAX 916 338-5353 mikew@walker-lumber.com

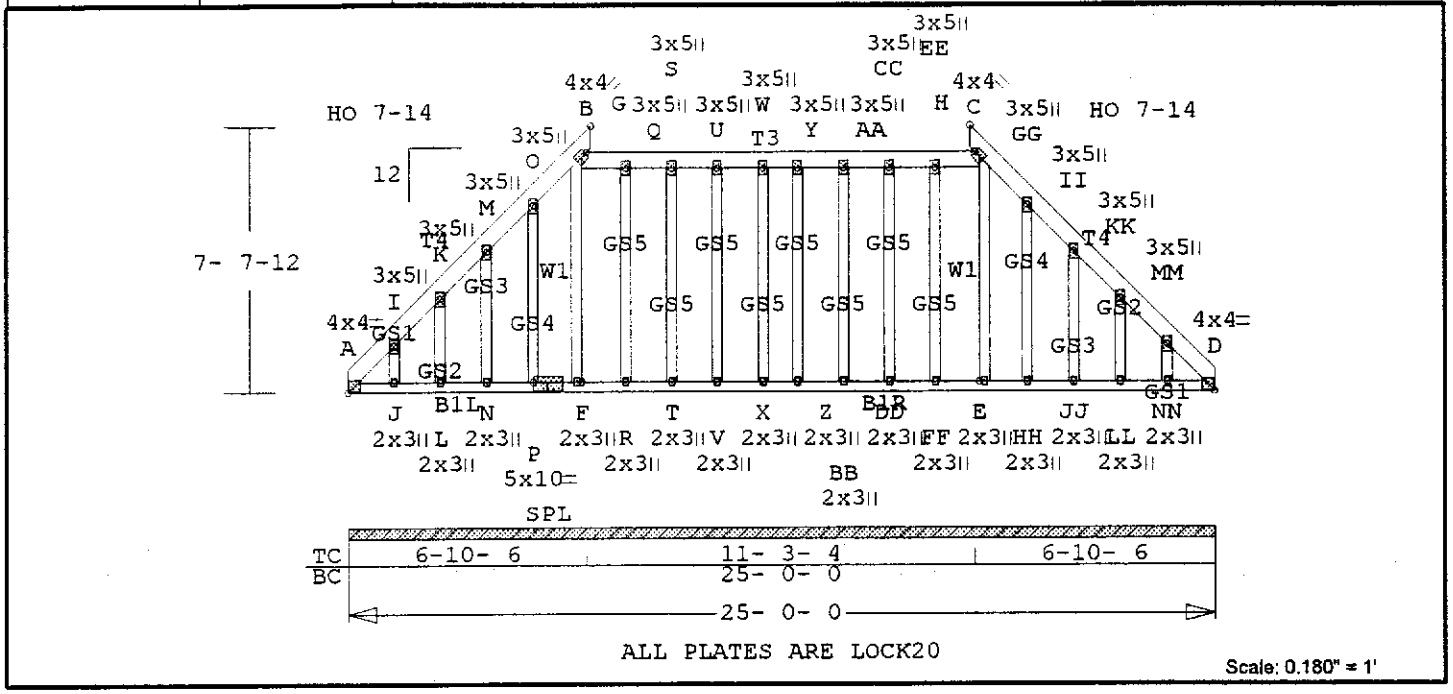


Baker Contracting
3534 D st.
cw023
10/26/01

0 4

1/4"=1'0"

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
cw023	A1	1	CAL	250000	6	0	0	



APPROX. TRUSS WEIGHT: 303.8 LBS

Online Plus -- Version 10.0.001
RUN DATE: 10-29-01

	CSI	SIZE LUMBER	FB
TOP	.05	2X 6 DFL-#2	1170
BTM	.06	2X 4 DFL-#2	1350
WBS	.06	2X 4 STA-STUD	661

EXCEPTIONS:
F-B 2X 4 DFL-#2 1350
E-C SAME AS F-B

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:
F-E 100.0 17.0
SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & D

LOAD CASE #1 UBC LL CHECK
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	.0	14.0	
BTM CHD	10.0	7.0	
TOTAL	10.0	21.0	31.0

EXCEPTIONS:
F-E 110.0 17.0
SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & D

LOAD CASE #2 WIND FROM LEFT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:
A-B 5.7N 14.0
B-C -16.9N 14.0
C-D -9.1N 14.0
F-E -105.7N 17.0
SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & D

LOAD CASE #3 WIND FROM RIGHT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:
A-B -9.1N 14.0
B-C -16.9N 14.0
C-D 5.7N 14.0
F-E -105.7N 17.0
SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & D

LOAD CASE #4 WIND // RIDGE
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:
A-B -16.9N 14.0
B-C -16.9N 14.0
C-D -16.9N 14.0
F-E -105.7N 17.0
SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & D

HEEL	LEFT 0IN - 2SX	RIGHT 0IN - 2SX

MEMBR	CSI	P(LBS)		M@1ST	M@2ND
		TOP CHORDS			
A-I	.05	201	C	369	77
I-K	.03	142	C	-77	13
K-M	.02	111	C	-13	16
M-O	.02	117	T	-12	42
O-B	.03	142	T	-42	184
B-Q	.03	140	T	-184	-51
Q-S	.02	140	T	51	-14
S-U	.02	140	T	14	10
U-W	.02	140	T	-10	12
W-Y	.02	140	T	-12	12
Y-AA	.02	140	T	-12	10
AA-CC	.02	140	T	-10	-14
CC-EE	.02	140	T	14	-51
EE-C	.03	140	T	51	184
C-GG	.03	142	T	-184	42
GG-II	.02	117	T	-42	12
II-KK	.01	57	T	-12	67
KK-MM	.01	51	C	-12	34
MM-D	.02	93	C	-34	-142
		BOTTOM CHORDS			
A-J	.06	69	T	-255	-75
J-L	.02	69	T	75	20
L-N	.01	69	T	-20	-5
N-P	.01	69	T	5	1
P-F	.01	69	T	-1	0
F-R	.01	69	T	0	0
R-T	.01	69	T	0	0
T-V	.01	69	T	0	0
V-X	.01	69	T	0	0
X-Z	.01	69	T	0	0
Z-BB	.01	69	T	0	0
BB-DD	.01	69	T	0	0
DD-FF	.01	69	T	0	0
FF-E	.01	69	T	0	0
E-HH	.01	69	T	0	0
HH-JJ	.01	69	T	0	2
JJ-LL	.01	69	T	-2	-8
LL-NN	.02	69	T	8	30
NN-D	.03	69	T	-30	-105

		WEBS			
J-I	= 100	C	L-K	= 81	C
N-M	= 80	C	P-O	= 85	C
F-B	= 74	C	R-Q	= 71	C
T-S	= 81	C	V-U	= 80	C
X-W	= 71	C	Z-Y	= 71	C
BB-AA	= 80	C	DD-CC	= 81	C
FF-EE	= 71	C	E-C	= 74	C
HH-GG	= 78	C	JJ-II	= 80	C
LL-KK	= 81	C	NN-MM	= 77	C

SPAN/DEFL (DL+LL) = 999

CHECKED NOV - 5 2001

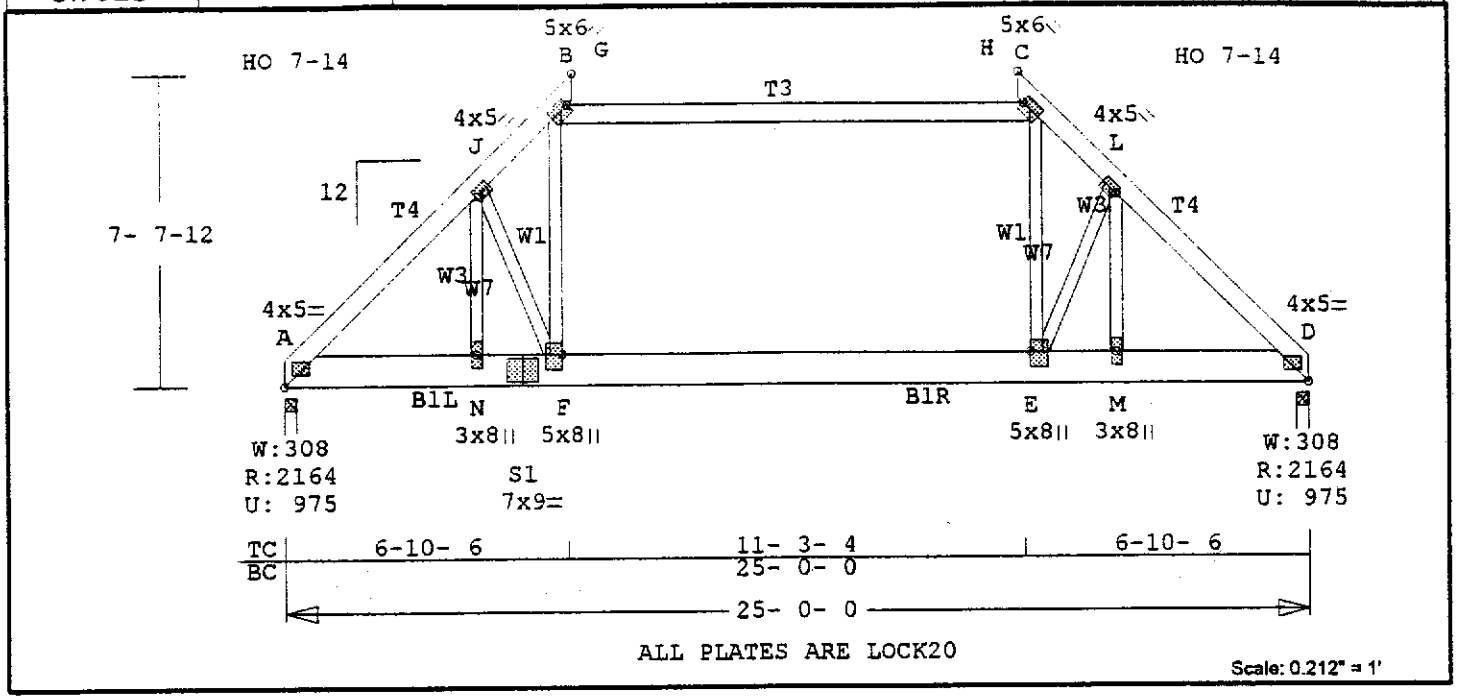
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering Cont.
cw023	A1	1	CAL	250000	6	0	0	

NOTES:
1. TRUSSES MANUFACTURED BY - Walker Lumber
2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
3. EMPIRICAL ANALOG IS USED.
4. DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
5. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   WIND SPEED - 80 MPH
   MEAN ROOF HEIGHT - 25'
   EXPOSURE CATEGORY - C
   OCCUPANCY CATEGORY - 1
   ENCLOSED BUILDING.
   OCEANLINE DIST - 100 MILES
   TC DEAD LOAD = 14.0 PSF
   BC DEAD LOAD = 7.0 PSF
6. NOTE LARGE BEARING SIZE.

8. PROVIDE DRAINAGE TO PREVENT WATER PONDING.

CHECKED NOV - 5 2001

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
cw023	A2	5	CAL	250000	6	0	0	



APPROX. TRUSS WEIGHT: 263.8 LBS

Online Plus -- Version 10.0.001
RUN DATE: 10-29-01

	CSI	SIZE	LUMBER	FB
TOP	.64	2X	6 DFL-#2	1170
BTM	.90	2X10	DFL-#1B	1320
WBS	.73	2X	4 STA-STUD	661
EXCEPTIONS:				
F-B		2X	4 DFL-#2	1350
E-C SAME AS F-B				

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
F-E	100.0	17.0	

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	2164	3- 8	D	2164	3- 8

LOAD CASE #1 UBC LL CHECK
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	.0	14.0	
BTM CHD	10.0	7.0	
TOTAL	10.0	21.0	31.0
EXCEPTIONS:			
F-E	110.0	17.0	

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	2014	3- 8	D	2014	3- 8

LOAD CASE #2 WIND FROM LEFT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
A-B	5.7N	14.0	
B-C	-16.9N	14.0	
C-D	-9.1N	14.0	
F-E	-105.7N	17.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	-179	-718	3- 8
D	HORZ RLR	0	-815	3- 8

LOAD CASE #3 WIND FROM RIGHT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
A-B	-9.1N	14.0	
B-C	-16.9N	14.0	
C-D	5.7N	14.0	
F-E	-105.7N	17.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	179	-815	3- 8
D	HORZ RLR	0	-718	3- 8

LOAD CASE #4 WIND // RIDGE
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
A-B	-16.9N	14.0	
B-C	-16.9N	14.0	
C-D	-16.9N	14.0	
F-E	-105.7N	17.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	0	-975	3- 8
D	HORZ RLR	0	-975	3- 8

	LEFT	RIGHT
HEEL	0IN - 2SX	0IN - 2SX

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
TOP CHORDS				
A-J	.25	2713 C	815	571
J-B	.55	2894 C	-571	-5577
B-C	.64	2456 C	6714	-6714
C-L	.55	2894 C	5577	571
L-D	.25	2713 C	-571	-815
BOTTOM CHORDS				
A-N	.17	1904 T	-159	1764
N-S1	.43	1904 T	-1764	-10138
S1-F	.82	1904 T	10138	-22386
F-E	.90	2456 T	24200	-24200
E-M	.82	1904 T	22387	1764
M-D	.17	1904 T	-1764	159
WEBS				
J-F	.73	1177 T	0	-618
E-L	.73	1177 T	618	0
N-J =	931 C	F-B =	1423 T	
E-C =	1423 T	M-L =	931 C	

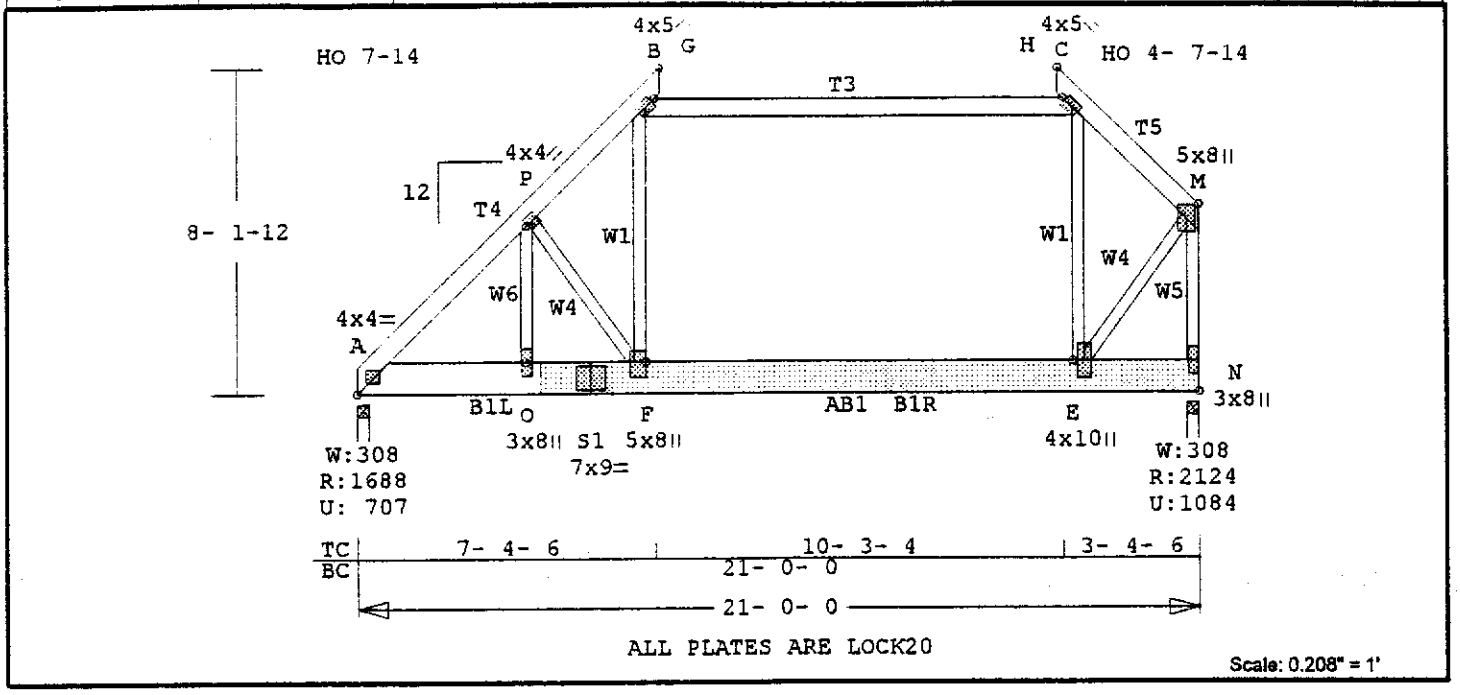
DL+LL DEFL = .25" IN B-C
LL DEFL = .21" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 999

NOTES:
1. TRUSSES MANUFACTURED BY - Walker Lumber
2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
3. EMPIRICAL ANALOG IS USED.
4. DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
5. WIND LOADS - ANSI/ASCE 7-98 TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM WIND SPEED - 80 MPH MEAN ROOF HEIGHT - 25' EXPOSURE CATEGORY - C OCCUPANCY CATEGORY - 1 ENCLOSED BUILDING. OCEANLINE DIST - 100 MILES TC DEAD LOAD = 14.0 PSF BC DEAD LOAD = 7.0 PSF
6. PREVENT [illegible] AT ALL BRACING LOCATIONS
8. PROVIDE DRAINAGE TO PREVENT WATER PONDING.

REGISTERED PROFESSIONAL ENGINEER
TING L. NIE
No. S 4100
Exp. 6-30-04
STRUCTURAL
STATE OF CALIFORNIA

CHECKED NOV - 5 2001

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
cw023	A3	3	CAL	210000	6	0	0	



APPROX. TRUSS WEIGHT: 308.0 LBS

Online Plus -- Version 10.0.001
RUN DATE: 11- 5-01

	CSI	SIZE	LUMBER	FB
TOP	.69	2X 6	DFL-#2	1170
BTM	.81	2X10	DFL-#2	990
WBS	.85	2X 4	STA-STUD	661
EXCEPTIONS:				
P-F		2X 4	DFL-#2	1350
E-M N-M SAME AS P-F				
ADDED LUMBER:				
O-S1		2X10	DFL-#1B	1320
S1-F F-E E-N SAME AS O-S1				

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:

F-E	100.0	17.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	1688	3- 8	N	2124	3- 8

LOAD CASE #1 UBC LL CHECK
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	.0	14.0	
BTM CHD	10.0	7.0	
TOTAL	10.0	21.0	31.0

EXCEPTIONS:

F-E	110.0	17.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	1562	3- 8	N	1998	3- 8

LOAD CASE #2 WIND FROM LEFT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:

A-B	5.7N	14.0	
B-C	-16.9N	14.0	
C-M	-9.1N	14.0	
F-E	-105.7N	17.0	
M-N	8.0N	.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	-191	-461	3- 8
N	HORZ RLR	0	-945	3- 8

LOAD CASE #3 WIND FROM RIGHT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:

A-B	-9.1N	14.0	
B-C	-16.9N	14.0	
C-M	5.7N	14.0	
F-E	-105.7N	17.0	
M-N	-11.3N	.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	244	-558	3- 8
N	HORZ RLR	0	-966	3- 8

LOAD CASE #4 WIND // RIDGE
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:

A-B	-16.9N	14.0	
B-C	-16.9N	14.0	
C-M	-16.9N	14.0	
F-E	-105.7N	17.0	
M-N	16.9N	.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	-11	-707	3- 8
N	HORZ RLR	0	-1084	3- 8

	LEFT	RIGHT
HEEL	0IN - 2SX	

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
TOP CHORDS				
A-P	.18	2053 C	365	238
P-B	.27	1900 C	-238	-2798
B-C	.69	1508 C	3094	-7491
C-M	.57	1812 C	5987	-7
BOTTOM CHORDS				
A-O	.21	1444 T	-124	2220
O-S1	.18	1444 T	-2219	2849
S1-F	.19	1444 T	-2849	3046
F-E	.81	1508 T	-2949	-37759
E-N	.64	70 T	-21840	-1
WEBS				
E-C	.85	847 T	1209	1089
E-M	.59	2362 T	890	0
O-P =	377 T	P-F =	522 C	
F-B =	1009 T	N-M =	3017 C	

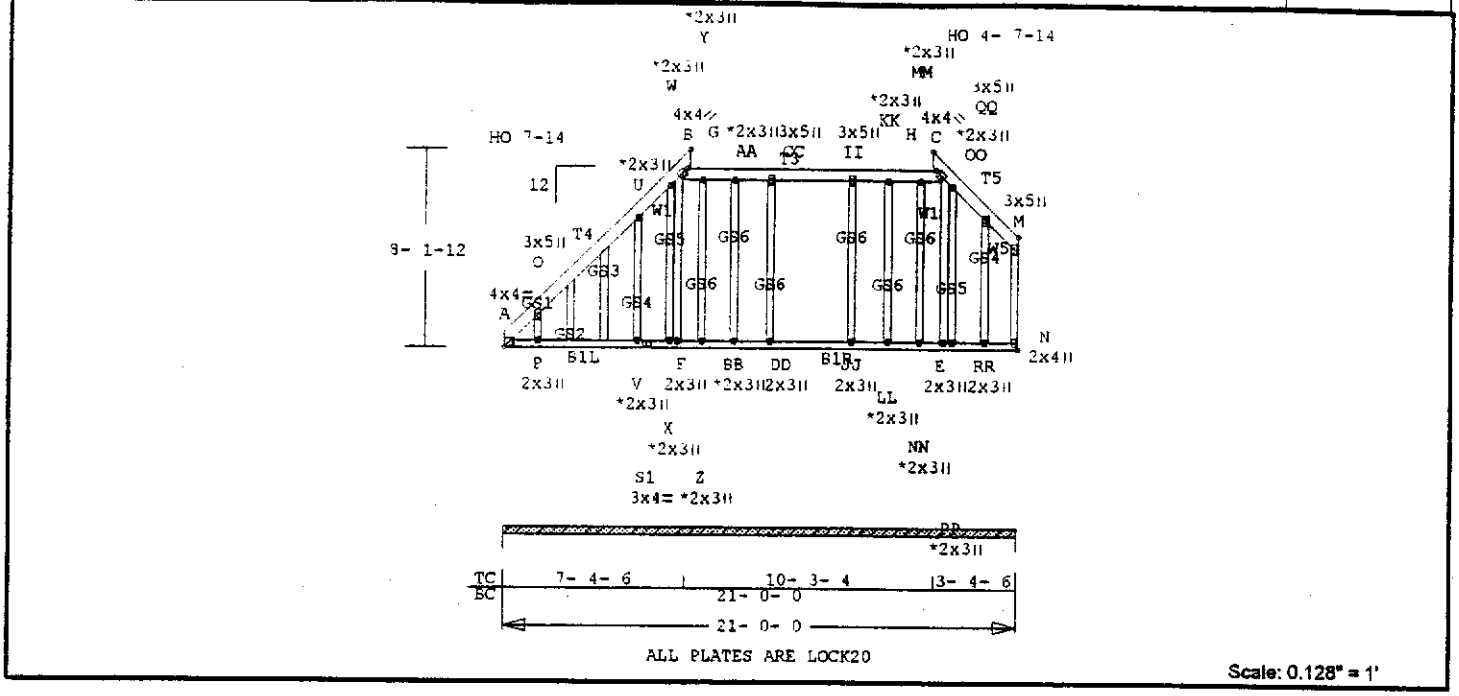
DL+LL DEFL = .22" IN B-C
LL DEFL = .13" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 999

NOTES:
1. TRUSSES MANUFACTURED BY - Walker Lumber
2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
3. EMPIRICAL ANALOG IS USED.
4. DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
5. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   WIND SPEED - 80 MPH
   MEAN ROOF HEIGHT - 25'
   EXPOSURE CATEGORY - C
   OCCUPANCY CATEGORY - 1
   ENCLOSED BUILDING.
   OCEANLINE DIST - 100 MILES
   TC DEAD LOAD = 14.0 PSF
   BC DEAD LOAD = 7.0 PSF
6. PREVENT TRUSS ROTATION AT ALL BEARING LOCATIONS.
7. FASTEN EACH ADDED SCAB WITH ROWS 10d NAILS AT IN. O.C. ALONG EN
9. PROVIDE DRAINAGE TO PREVENT WATER PONDING.

REGISTERED PROFESSIONAL ENGINEER
G.L. NIE
No. S 4100
Exp. 6-30-04
STRUCTURAL
STATE OF CALIFORNIA

CHECKED NOV - 5 2001

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
cw023	A4	1	CAL	210000	6	0	0	



APPROX. TRUSS WEIGHT: 283.9 LBS

Online Plus -- Version 10.0.001
RUN DATE: 11- 5-01

	CSI	SIZE LUMBER	FB
TOP	.19	2X 6 DFL-#2	1170
BTM	.07	2X 4 DFL-#2	1350
WBS	.29	2X 4 STA-STUD	661
EXCEPTIONS:			
N-M		2X 4 DFL-#2	1350

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
GABLE STUDS ARE CONSIDERED NON-STRUCTURAL. ALL BRACING FOR HORIZONTAL WIND LOADS ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
F-E	100.0	17.0	

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & N

LOAD CASE #1 UBC LL CHECK
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	.0	14.0	
BTM CHD	10.0	7.0	
TOTAL	10.0	21.0	31.0
EXCEPTIONS:			
F-E	110.0	17.0	

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & N

LOAD CASE #2 WIND FROM LEFT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
A-B	5.7N	14.0	
B-C	-16.9N	14.0	
C-M	-9.1N	14.0	
F-E	-105.7N	17.0	
M-N	8.0N	.0	

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & N

LOAD CASE #3 WIND FROM RIGHT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
A-B	-9.1N	14.0	
B-C	-16.9N	14.0	
C-M	5.7N	14.0	
F-E	-105.7N	17.0	
M-N	-11.3N	.0	

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & N

LOAD CASE #4 WIND // RIDGE
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
A-B	-16.9N	14.0	
B-C	-16.9N	14.0	
C-M	-16.9N	14.0	
F-E	-105.7N	17.0	
M-N	16.9N	.0	

SUPPORT CRITERIA
CONTINUOUS BETWEEN JNTS A & N

	LEFT	RIGHT
HEEL	0IN - 2SX	

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
TOP CHORDS				
A-O	.19	339 T	-416	-1747
O-B	.18	176 C	-239	103
B-CC	.16	169 T	-971	-147
CC-II	.07	169 T	147	41
II-C	.07	169 T	-41	205
C-QQ	.05	174 T	-205	51
QQ-M	.02	137 T	-51	5
BOTTOM CHORDS				
A-P	.07	74 T	0	0
P-S1	.02	74 T	0	0
S1-F	.01	74 T	0	0
F-DD	.01	74 T	0	0
DD-JJ	.01	74 T	0	0
JJ-E	.01	74 T	0	0
E-RR	.01	74 T	0	0
RR-N	.01	74 T	0	0
WEBS				
N-M	.19	100 T	0	0
P-O =	368 C	F-B =	316 C	
DD-CC=	164 C	JJ-II=	219 C	
E-C =	.173 C	RR-QQ=	92 C	

DL+LL DEFL = .02" IN O-B
LL DEFL < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 999

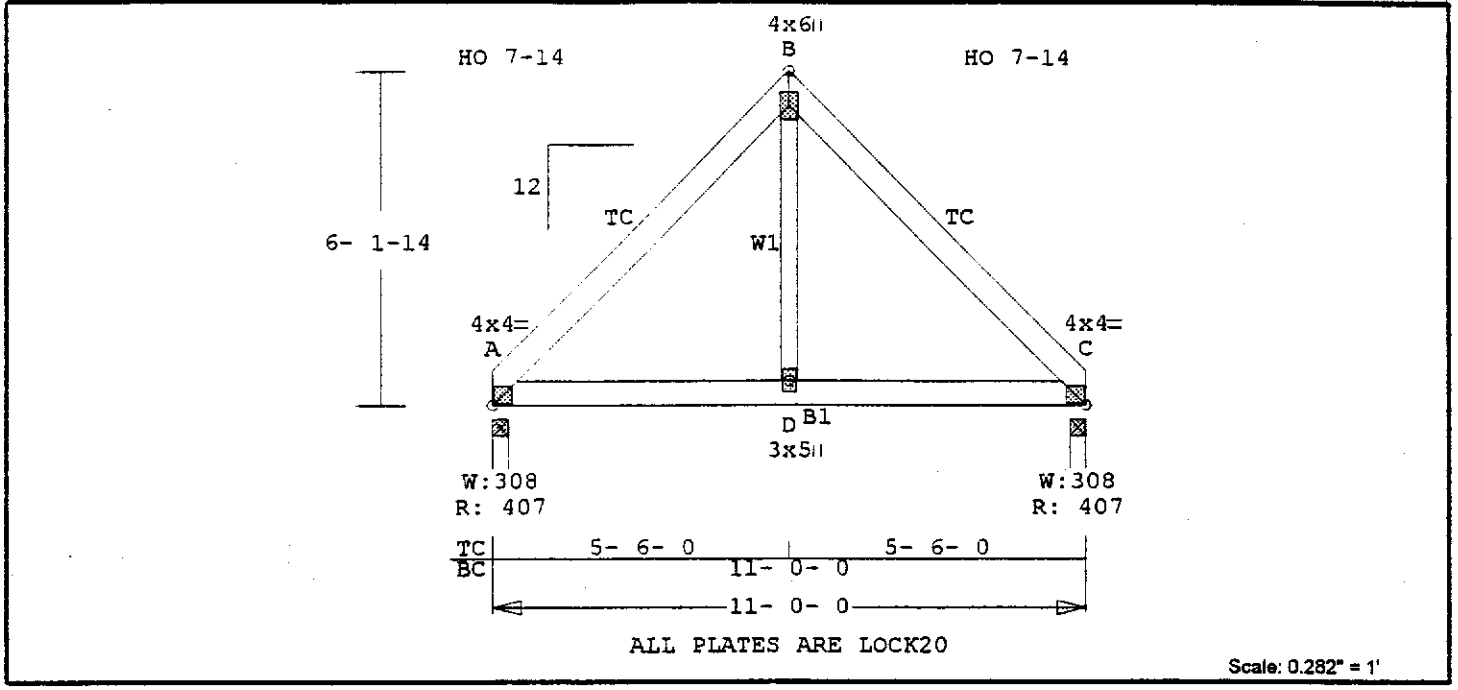
NOTES:
1. TRUSSES MANUFACTURED BY - Walker Lumber
2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
3. EMPIRICAL ANALOG IS USED.
4. DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
5. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   WIND SPEED - 80 MPH
   MEAN ROOF HEIGHT - 25'
   EXPOSURE CATEGORY - C
   OCCUPANCY CATEGORY - 1
   ENCLOSED BUILDING.
   OCEANLINE DIST - 100 MILES
   TC DEAD LOAD = 14.0 PSF
   BC DEAD LOAD = 7.0 PSF
6. NOTE LARGE BEARING SIZE.

8. PROVIDE DRAINAGE TO PREVENT WATER PONDING.

REGISTERED PROFESSIONAL ENGINEER
TING L. NIE
No. S 4100
Exp. 6-30-04
STRUCTURAL
STATE OF CALIFORNIA

CHECKED NOV - 5 2001

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
cw023	CAP1	5	2.2	110000	12	0	0	



APPROX. TRUSS WEIGHT: 87.4 LBS

Online Plus -- Version 10.0.001
RUN DATE: 10-29-01

	CSI	SIZE LUMBER	1.15FB
TOP	.12	2X 6 DFL-#2	1345
BTM	.13	2X 6 DFL-#2	1345
WBS	.09	2X 4 STA-STUD	760

REPETITIVE MEMBER INCREASES:
FB 15.0% FT .0% FC .0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	407	3- 8	C	407	3- 8

LOAD CASE #1 UBC LL CHECK
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	.0	14.0	
BTM CHD	10.0	7.0	
TOTAL	10.0	21.0	31.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	341	3- 8	C	341	3- 8

LOAD CASE #2 WIND FROM LEFT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
A-B	5.7N	14.0	
B-C	-9.1N	14.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	-153	210	3- 8
C	HORZ RLR	0	216	3- 8

LOAD CASE #3 WIND FROM RIGHT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
A-B	-9.1N	14.0	
B-C	5.7N	14.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	153	213	3- 8
C	HORZ RLR	0	208	3- 8

LOAD CASE #4 WIND // RIDGE
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD (PSF)	
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0
EXCEPTIONS:			
A-B	-16.9N	14.0	
B-C	-16.9N	14.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	0	45	3- 8
C	HORZ RLR	0	45	3- 8

	LEFT	RIGHT
HEEL	0IN - 2SX	0IN - 2SX

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
	TOP CHORDS			
A-B	.12	306 C	830	-780
B-C	.12	306 C	780	-830
	BOTTOM CHORDS			
A-D	.13	216 T	-665	-851
D-C	.12	216 T	851	665
	WEBS			
D-B	=	217 T		

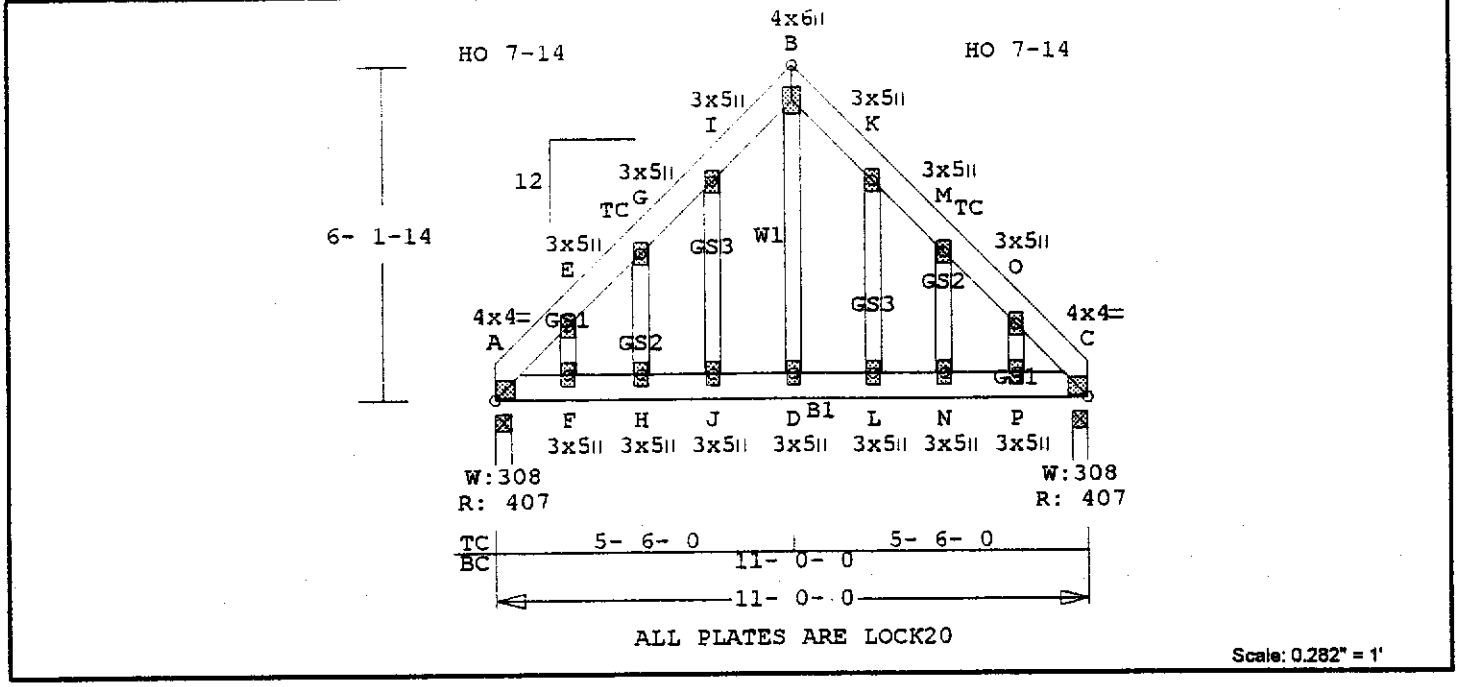
DL+LL DEFL = .03" IN A-B
LL DEFL = .00" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 999

NOTES:
1. TRUSSES MANUFACTURED BY - Walker Lumber
2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
3. EMPIRICAL ANALOG IS USED.
4. DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
5. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   WIND SPEED - 80 MPH
   MEAN ROOF HEIGHT - 25'
   EXPOSURE CATEGORY - C
   OCCUPANCY CATEGORY - 1
   ENCLOSED BUILDING.
   OCEANLINE DIST - 100 MILES
   TC DEAD LOAD = 14.0 PSF
   BC DEAD LOAD = 7.0 PSF

REGISTERED PROFESSIONAL ENGINEER
TING L. NIE
No. S 4100
Exp. 6-30-04
STRUCTURAL
STATE OF CALIFORNIA

CHECKED NOV - 5 2001

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
cw023	CAP4	1	2.2	110000	12	0	0	



APPROX. TRUSS WEIGHT: 114.2 LBS

Online Plus -- Version 10.0.001
RUN DATE: 10-29-01

	CSI	SIZE LUMBER	1.15FB
TOP	.07	2X 6 DFL-#2	1345
BTM	.17	2X 6 DFL-#2	1345
WBS	.11	2X 4 STA-STUD	760

REPETITIVE MEMBER INCREASES:
FB 15.0% FT .0% FC .0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	407	3- 8	C	407	3- 8

LOAD CASE #1 UBC LL CHECK
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	.0	14.0	
BTM CHD	10.0	7.0	
TOTAL	10.0	21.0	31.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	341	3- 8	C	341	3- 8

LOAD CASE #2 WIND FROM LEFT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:

A-B	5.7N	14.0	
B-C	-9.1N	14.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	-153	210	3- 8
C	HORZ RLR	0	216	3- 8

LOAD CASE #3 WIND FROM RIGHT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:

A-B	-9.1N	14.0	
B-C	5.7N	14.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	153	213	3- 8
C	HORZ RLR	0	208	3- 8

LOAD CASE #4 WIND // RIDGE
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:

A-B	-16.9N	14.0	
B-C	-16.9N	14.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	0	45	3- 8
C	HORZ RLR	0	45	3-

	LEFT	RIGHT
HEEL	0IN - 2SX	0IN - 2SX

MEMBR	CSI	P(LBS)		M@1ST	M@2ND
TOP CHORDS					
A-E	.05	344	C	40	430
E-G	.07	325	C	-430	648
G-I	.07	302	C	-648	346
I-B	.05	284	C	-346	-405
B-K	.05	284	C	405	346
K-M	.07	302	C	-346	648
M-O	.07	325	C	-648	430
O-C	.05	344	C	-430	-40
BOTTOM CHORDS					
A-F	.11	223	T	165	739
F-H	.13	223	T	-739	1106
H-J	.13	223	T	-1106	414
J-D	.17	223	T	-414	-1750
D-L	.17	223	T	1750	414
L-N	.12	223	T	-414	1106
N-P	.12	223	T	-1106	739
P-C	.09	223	T	-739	-165
WEBS					
F-E =	41	C	H-G =	48	C
J-I =	58	C	D-B =	262	T
L-K =	58	C	N-M =	48	C
P-O =	41	C			

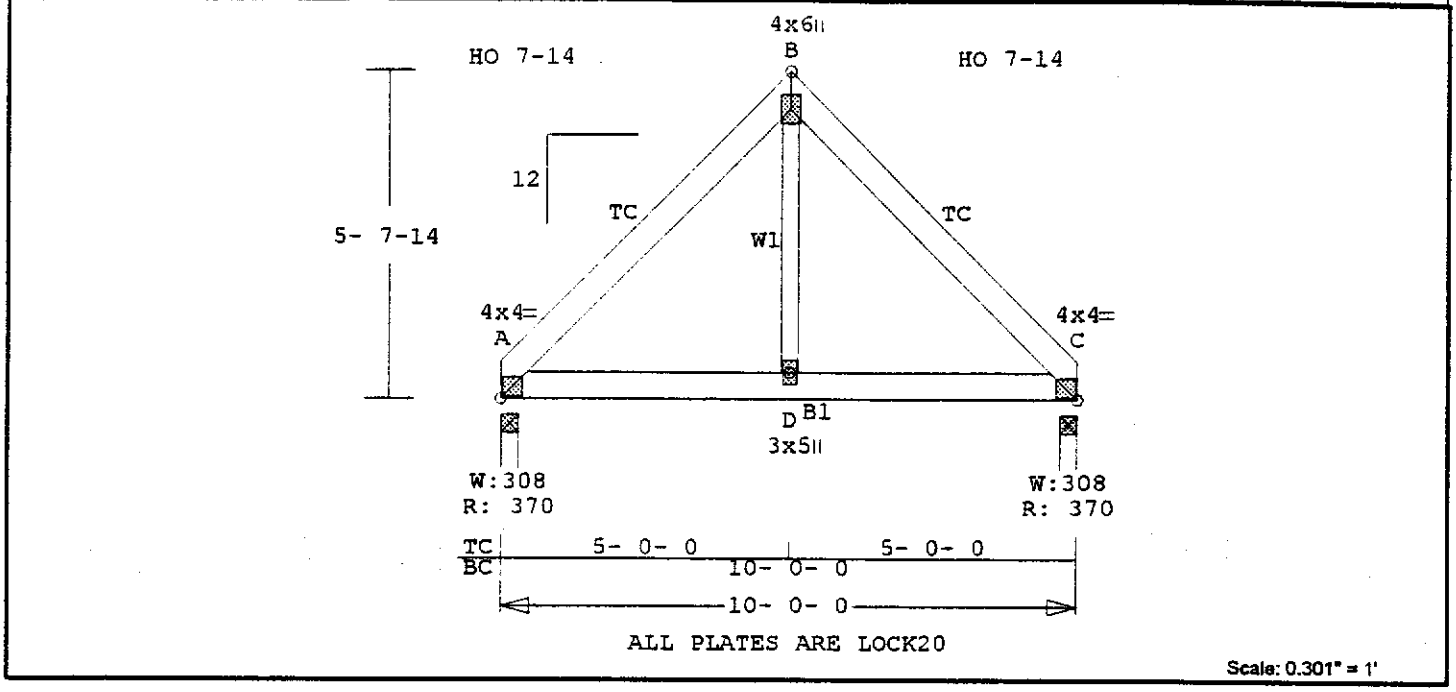
DL+LL DEFL = .02" AT G
LL DEFL = .01" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 999

NOTES:
1. TRUSSES MANUFACTURED BY - Walker Lumber
2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
3. EMPIRICAL ANALOG IS USED.
4. DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
   WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MWFRS FOR FORCE RES SYSTEM
   WIND SPEED - 80 MPH
   MEAN ROOF HEIGHT - 25'
   EXPOSURE CATEGORY - C
   OCCUPANCY CATEGORY - 1
   ENCLOSED BUILDING.
   OCEAN LINE DIST - 100 MILES
   TC DEAD LOAD = 14.0 PSF
   BC DEAD LOAD = 7.0 PSF

REGISTERED PROFESSIONAL ENGINEER
No. S
Exp. 6-30-03
STRUCTURAL
STATE OF CALIFORNIA

CHECKED NOV - 5 2001

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
cw023	CAP2	3	2.2	100000	12	0	0	



APPROX. TRUSS WEIGHT: 79.6 LBS

Online Plus -- Version 10.0.001
RUN DATE: 10-29-01

	CSI	SIZE	LUMBER	1.15FB
TOP	.10	2X 6	DFL-#2	1345
BTM	.10	2X 6	DFL-#2	1345
WBS	.08	2X 4	STA-STUD	760

REPETITIVE MEMBER INCREASES:
FB 15.0% FT .0% FC .0%

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	370	3- 8	C	370	3- 8

LOAD CASE #1 UBC LL CHECK
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE: 25.0%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	.0	14.0	
BTM CHD	10.0	7.0	
TOTAL	10.0	21.0	31.0

SUPPORT CRITERIA

JT	REACT LBS	WIDTH IN-SX	JT	REACT LBS	WIDTH IN-SX
A	310	3- 8	C	310	3- 8

LOAD CASE #2 WIND FROM LEFT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:

A-B	5.7N	14.0	
B-C	-9.1N	14.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	-138	191	3- 8
C	HORZ RLR	0	196	3- 8

LOAD CASE #3 WIND FROM RIGHT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:

A-B	-9.1N	14.0	
B-C	5.7N	14.0	

SUPPORT CRITERIA

JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	138	194	3- 8
C	HORZ RLR	0	189	3- 8

LOAD CASE #4 WIND // RIDGE
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE: 33.3%

LOADING	LIVE	DEAD	(PSF)
TOP CHD	16.0	14.0	
BTM CHD	.0	7.0	
TOTAL	16.0	21.0	37.0

EXCEPTIONS:

A-B	-16.9N	14.0	
B-C	-16.9N	14.0	

SUPPORT CRITERIA

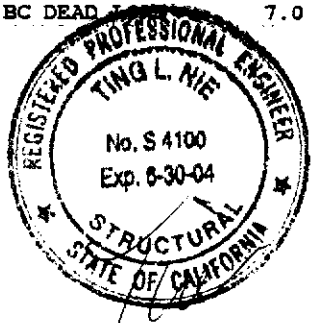
JT	TYPE	HORZ LBS	VERT LBS	WIDTH IN-SX
A	PIN	0	40	3- 8
C	HORZ RLR	0	40	3- 8

	LEFT	RIGHT
HEEL	0IN - 2SX	0IN - 2SX

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
	TOP CHORDS			
A-B	.10	275 C	656	-600
B-C	.10	275 C	600	-656
	BOTTOM CHORDS			
A-D	.10	194 T	-515	-665
D-C	.10	194 T	665	515
	WEBS			
D-B =	195 T			

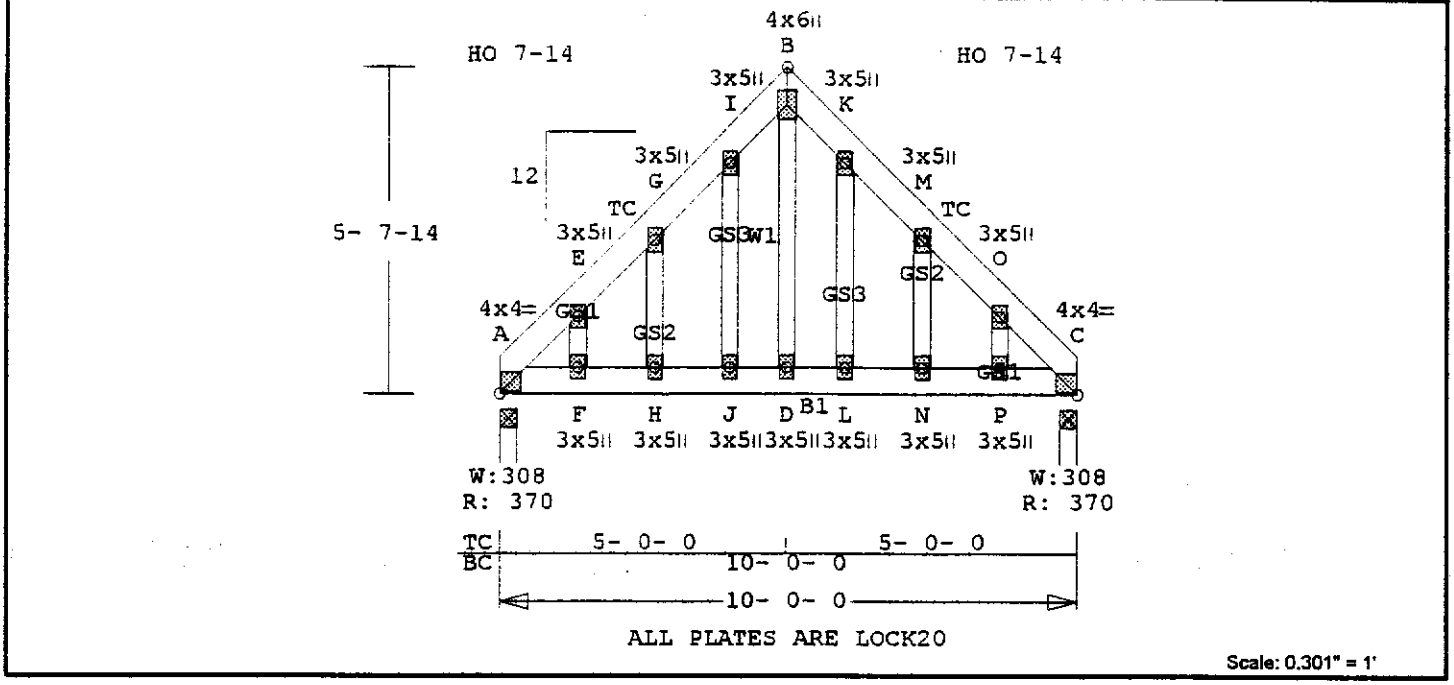
DL+LL DEFL = .02" IN A-B
LL DEFL = .00" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 999

NOTES:
1. TRUSSES MANUFACTURED BY - Walker Lumber
2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
3. EMPIRICAL ANALOG IS USED.
4. DESIGN INCLUDES CHECK FOR 10 PSF NON-CONCURRENT LIVE LOAD ON BOTTOM CHORD.
5. WIND LOADS - ANSI/ASCE 7-98
   TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM
   WIND SPEED - 80 MPH
   MEAN ROOF HEIGHT - 25'
   EXPOSURE CATEGORY - C
   OCCUPANCY CATEGORY - 1
   ENCLOSED BUILDING.
   OCEANLINE DIST - 100 MILES
   TC DEAD LOAD = 14.0 PSF
   BC DEAD LOAD = 7.0 PSF



CHECKED NOV - 5 2001

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
cw023	CAP3	1	2.2	100000	12	0	0	



APPROX. TRUSS WEIGHT: 106.4 LBS

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Online Plus -- Version 10.0.001
RUN DATE: 10-29-01

     CSI    SIZE LUMBER  1.15FB
TOP  .06    2X 6 DFL-#2    1345
BTM  .13    2X 6 DFL-#2    1345
WBS  .09    2X 4 STA-STUD   760
REPETITIVE MEMBER INCREASES:
FB 15.0%  FT   .0%  FC   .0%

LATERAL BRACING:
 TOP CHORD - CONTINUOUS
 BTM CHORD - CONTINUOUS
TRUSS SPACING -  24.0 IN.

STANDARD LOADING
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE:  25.0%
LOADING    LIVE    DEAD (PSF)
TOP CHD    16.0     14.0
BTM CHD     .0      7.0
TOTAL      16.0     21.0    37.0
SUPPORT CRITERIA
JT REACT WIDTH   JT REACT WIDTH
    LBS  IN-SX       LBS  IN-SX
A    370  3- 8  C    370  3- 8

LOAD CASE #1  UBC LL CHECK
LUMBER STRESS INCREASE: 25.0%
PLATE STRESS INCREASE:  25.0%
LOADING    LIVE    DEAD (PSF)
TOP CHD     .0     14.0
BTM CHD    10.0     7.0
TOTAL      10.0     21.0    31.0
SUPPORT CRITERIA
JT REACT WIDTH   JT REACT WIDTH
    LBS  IN-SX       LBS  IN-SX
A    310  3- 8  C    310  3- 8

LOAD CASE #2  WIND FROM LEFT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE:  33.3%
LOADING    LIVE    DEAD (PSF)
TOP CHD    16.0     14.0
BTM CHD     .0      7.0
TOTAL      16.0     21.0    37.0
EXCEPTIONS:
A-B         5.7N    14.0
B-C        -9.1N    14.0
SUPPORT CRITERIA
JT    TYPE     HORZ  VERT WIDTH
               LBS   LBS  IN-SX
A      PIN     -138   191  3- 8
C   HORZ RLR      0   196  3- 8

LOAD CASE #3  WIND FROM RIGHT
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE:  33.3%
LOADING    LIVE    DEAD (PSF)
TOP CHD    16.0     14.0
BTM CHD     .0      7.0
TOTAL      16.0     21.0    37.0
EXCEPTIONS:
A-B        -9.1N    14.0
B-C         5.7N    14.0
SUPPORT CRITERIA
JT    TYPE     HORZ  VERT WIDTH
               LBS   LBS  IN-SX
A      PIN      138   194  3- 8
C   HORZ RLR      0   189  3- 8

LOAD CASE #4  WIND // RIDGE
LUMBER STRESS INCREASE: 33.3%
PLATE STRESS INCREASE:  33.3%
LOADING    LIVE    DEAD (PSF)
TOP CHD    16.0     14.0
BTM CHD     .0      7.0
TOTAL      16.0     21.0    37.0
EXCEPTIONS:
A-B       -16.9N    14.0
B-C       -16.9N    14.0
SUPPORT CRITERIA
JT    TYPE     HORZ  VERT WIDTH
               LBS   LBS  IN-SX
A      PIN        0    40  3- 8
C   HORZ RLR      0    40  3- 8

          LEFT          RIGHT
HEEL   0IN -  2SX    0IN -  2SX

MEMBR  CSI   P(LBS) M@1ST M@2ND
          TOP CHORDS
A-E     .05   308 C     33   377
E-G     .06   288 C   -377   489
G-I     .06   272 C   -489   248
I-B     .03   245 C   -248  -329
B-K     .03   245 C    329   248
K-M     .05   272 C   -248   489
M-O     .06   288 C   -489   377
O-C     .04   308 C   -377   -33
          BOTTOM CHORDS
A-F     .09   200 T    143   663
F-H     .11   200 T   -663   912
H-J     .11   200 T   -912   -65
J-D     .13   200 T     65 -1354
D-L     .13   200 T   1354   -65
L-N     .10   200 T     65   912
N-P     .10   200 T   -912   663
P-C     .08   200 T   -663  -143
             WEBS
F-E  =    40 C  H-G  =    58 C
J-I  =    36 T  D-B  =   229 T
L-K  =    36 T  N-M  =    58 C
P-O  =    40 C

DL+LL DEFL =  .01" AT G
LL DEFL =  .01" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 999

NOTES:
 1. TRUSSES MANUFACTURED BY -
    Walker Lumber
 2. ANALYSIS CONFORMS TO
    TPI (ANSI/TPI 1-1995).
 3. EMPIRICAL ANALOG IS USED.
 4. DESIGN INCLUDES CHECK FOR
    NON-CONCURRENT
    LIVE LOAD ON BOTTOM CHORD.
    WIND LOADS - ANSI/ASCE 7-98
    TRUSS IS DESIGNED AS A
    MAIN WIND FORCE RES SYSTEM
    WIND SPEED -  80 MPH
    MEAN ROOF HEIGHT - 25'
    EXPOSURE CATEGORY - C
    OCCUPANCY CATEGORY - 1
    ENCLOSED BUILDING.
    OCEAN LINE DIST - 100 MILES
    TC DEAD LOAD =   14.0 PSF
    BC DEAD LOAD =    7.0 PSF
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REGISTERED PROFESSIONAL ENGINEER TIMOTHY L. [illegible] No. 54[illegible] Exp. 6-30-[illegible] STATE OF [illegible] STRUCTURAL

CHECKED NOV - 5 2001