

**CITY OF SACRAMENTO**  
1231 I Street, Sacramento, CA 95814

**Permit No: 9912074**  
**Insp Area: 2**

**Site Address: 7403 SHELBY ST SAC**  
Parcel No: 117-1350-001  
N

Sub-Type: NSFR  
Housing (Y/N):  
LOT 1 LAGUNA VEGA NORTH UNIT 1

CONTRACTOR  
L. R. HERRION INC  
1701 BELLEVUE RD STE 209  
DUBLIN CA 94568

OWNER

ARCHITECT

**Nature of Work: MP 1385 1 STORY 7 ROOM SFR**

**CONSTRUCTION LENDING AGENCY:** I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C.)

Lender's Name \_\_\_\_\_ Lender's Address \_\_\_\_\_

**LICENSED CONTRACTORS DECLARATION:** I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class: 92 License Number: 70010 Date: 1/15/19 Contractor Signature: [Signature]

**OWNER-BUILDER DECLARATION:** I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00).

I, \_\_\_\_\_, as owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code). The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. (B) however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not intend to improve for the purpose of sale.)

I, \_\_\_\_\_, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code). The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor licensed pursuant to the Contractors License Law.

I am exempt under Sec. \_\_\_\_\_ B & PC for this reason: \_\_\_\_\_

Date: \_\_\_\_\_ Owner Signature: \_\_\_\_\_

**IN ISSUING THIS BUILDING PERMIT,** the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of an improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date: 1/15/19 Applicant Agent Signature: [Signature]

**WORKER'S COMPENSATION DECLARATION:** I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier: ARGONAUT INS CO Policy Number: WC62600115505 Exp Date: 07/01/2000

This section need not be completed if the permit is for \$100 or less. I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date: 1/15/19 Applicant Signature: [Signature]

**WARNING:** FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

**THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.**

# OMEGA PRODUCTS INTERNATIONAL, INC.

DIAMOND WALL INSULATING STUCCO SYSTEM

JOB ADDRESS:

ICBO Report #4004

7403 Shelby St  
EIK Grove CA 95758

Date of Job Completion 1-21-00

PLASTERING CONTRACTOR:

Name: STUCCO WORKS

Address: 5900 WAREHOUSE WY SAC 95826

Telephone No: (916) 383 6699

Contractor Number of Diamond Wall System 2175

This is to certify that the exterior coating system on the building exterior at the above address has been installed in accordance with the evaluation report specified above and the manufacturer's instructions.

4-14-00  
Date

[Signature]  
Signature of authorized representative of  
Plastering Contractor

This installation card must be presented to the building inspector after completion of work and before final inspection.

COUNTY SANITATION DISTRICT NO. 1  
 SACRAMENTO REGIONAL COUNTY SANITATION DISTRICT  
**SEWER IMPACT FEE** 9226  
 PERMIT AND CALCULATION SHEET 11/16/99

APPLICATION NO:		BLDG PERMIT NO: City	
GENERAL INFORMATION		THIS PERMIT GOOD ONLY WHEN VALIDATED BY THE CASHIER	
<ul style="list-style-type: none"> <li>- DEPT 26 \$2,855.00</li> <li>- TRAN 401179 11/10/99</li> <li>- RECEIPT 725473 C#1 \$2,855.00</li> </ul>		2555 11/16/99 THIS PERMIT TO CONNECT EXPIRES ONE YEAR FROM DATE OF ISSUANCE	
FEE CALCULATION		BUILDING USE	
INSPECTION		RESIDENTIAL SF <input type="checkbox"/>	MF <input type="checkbox"/>
CSD-1	470	COMMERCIAL USE	UNITS
SRCSD	2385		
CONSTRUCTION			
IN-LIEU			
<b>TOTAL FEE</b>	<b>2855</b>		
APN: 117-1350-001			
DESCRIPTION/ SUBDIVISION		LAGUNA VEGA VILL. 1 LOT: 1	
PROPERTY ADDRESS 7403 Shelby St			
OWNER D.R. HORTON			
MAILING ADDRESS 110 BLUE PAVINE			
CITY-STATE-ZIP		FOLSOM CA PHONE 3551234	
ADDITIONAL FEES MAY BE DUE IF CHANGES IN USE INCREASE SEWER IMPACT.			
APPLICANT SIGNATURE <i>D.R. Horton</i>			
CONSOLIDATED UTILITY BILLING USE ONLY			
ACCT _____ INPUT _____ START _____			

INSPECTOR'S COPY

# Certification of Compliance

## School District Development Fees

*(Print or Type) If Printing, press hard for four copies*

### PART I To be completed by the APPLICANT

OWNER'S NAME D.R. HORTON  
 OWNER'S ADDRESS 110 BLUE RAUINE RD. STE 209 FOLSOM CA 95630  
 PROJECT ADDRESS 7403 SHELBY STREET SAC.  
 PARCEL NUMBER 117-1350-001 LOT NO. 1  
 SUBDIVISION NAME LACUNA VEGA  
 NUMBER OF UNITS S.F. HOME

*Upon payment of the fees listed below, a 90-day approval period commences upon which the applicant paying the fees may protest such fees. Any failure to file such protest within the 90-day period shall result in forfeiture of any rights to challenge such fees, through litigation or otherwise.*

APPLICANT'S SIGNATURE [Signature]  
 TITLE OF APPLICANT SUPP.  
 DATE 10/18/99 PHONE NUMBER 955-1234

### PART II To be completed by BUILDING DEPARTMENT

PLAN IDENTIFICATION NUMBER 991207AR  
 BUILDING TYPE  
 RESIDENTIAL  APARTMENT/CONDOMINIUM ( ) COMMERCIAL/INDUSTRIAL ( )  
 SQUARE FEET OF CHARGEABLE BUILDING AREA 1385  
 SIGNATURE [Signature]  
 TITLE Bldg Insp DATE 10/25/99

### PART III To be completed by SCHOOL DISTRICT

SCHOOL DISTRICT ELGROVE SD  
 DISTRICT CERTIFICATION NO. 24339

EXEMPT	COMMENTS			
RESIDENTIAL/APT/CONDO (1)	1385	SQ FT X \$	1.93	= \$ 2,673.05
COMMERCIAL/INDUSTRIAL		SQ FT X \$		= \$
OTHER FEE TYPE (1)	1385	SQ FT X \$	1.34	= \$ 1,855.90
TOTAL FEES COLLECTED (1)	1385		3.27	= \$ 4,528.95

This Certification covers only the amount of square footage indicated above. Any additions or corrections to the square footage for this project will require an amendment to the Certificate of Compliance.

As the authorized school district official, I hereby certify that the requirements of Government Code Section 65995 and any other authorized requirements have been complied with by the above signed applicant.

### AUTHORIZED SCHOOL DISTRICT OFFICIAL

SIGNATURE [Signature] DATE PAID  
 TITLE [Signature]

Original: School District    1st copy: School District    2nd copy: Building Department  
 Revised 12/12/96    11/10/99

**PAID**  
 NOV 10 1999  
 3rd copy: Applicant  
 Facilities Planning  
 Elk Grove Unified School District

# CERTIFICATION OF INSULATION

PART I GENERAL


PART II AREAS INSULATED

PART III

<b>ADDRESS OR TRACT</b>	<b>SACRAMENTO INSULATION CONTRACTORS</b>
D. R. HORTON  LAGUNA VERDE	<input checked="" type="checkbox"/> P.O. BOX 854, WEST SACRAMENTO, CA 95691 LIC. #202026 <input type="checkbox"/> 1309 MELODY ROAD, MARYSVILLE, CA 95901 LIC. #202026 <input type="checkbox"/> P.O. BOX 9651, FRESNO, CA 93793-9651 LIC. #202026 <input type="checkbox"/> P.O. BOX 1631, RENO, NV 89505 LIC. #10675 <input type="checkbox"/> 3326 A PONDEROSA WAY, LAS VEGAS, NV 89118 LIC. #10675
LOT # 1	DATE INSULATION COMPLETED 2-25-00

WALLS		CEILINGS			FLOORS	
SQUARE FEET:		SQUARE FEET)			( SQUARE FEET)	
TYPE OF INSULATION		TYPE OF INSULATION			TYPE OF INSULATION	
MATERIAL <b>FIBERGLASS</b>	MATERIAL <b>FIBERGLASS</b>	MATERIAL <b>FIBERGLASS</b>				
FORM <b>BATTS</b>	FORM <b>BATTS &amp; BLOW</b>	FORM <b>BATTS</b>				
MANUFACTURER'S PRODUCT I D		MANUFACTURER'S PRODUCT I D			MANUFACTURER'S PRODUCT I D	
<b>MANUFACTURER</b>		<b>MANUFACTURER</b>			<b>MANUFACTURER</b>	
<b>OCF</b>		<b>OCF</b>			<b>OCF</b>	
BAGS						
<b>R - VALUE INSTALLED</b>	<b>APPLIED THICKNESS</b>	<b>R - VALUE INSTALLED</b>	<b>APPLIED THICKNESS</b>	<b>MIN. INSTALLED WEIGHT PER SQUARE FOOT</b>	<b>R - VALUE INSTALLED</b>	<b>APPLIED THICKNESS</b>
13	3 5/8"	30 30	9 1/2" 12"			
<b>KNEE WALLS IF R-VALUE IS OTHER THAN WALLS ABOVE</b>						
MATERIAL <b>FIBERGLASS</b>	FORM <b>BATTS</b>	R VALUE 19			MANUFACTURER <b>OCF</b>	
<b>AIR INFILTRATION SEALANT</b>						
MATERIAL <b>FOAM</b>				MANUFACTURER <b>W R GRACE</b>		

**THIS IS TO CERTIFY THAT INSULATION AND/OR SEALANT HAS BEEN INSTALLED IN CONFORMANCE WITH APPLICABLE CODES, MATERIAL STANDARDS AND REGULATIONS.**

SIGNATURE - INSULATION CONTRACTOR 	TITLE MANAGER	DATE 1-7-00
SIGNATURE - GENERAL CONTRACTOR	TITLE	DATE
REMARKS		

**N**orman  
**S**cheel  
**S**tructural  
**E**ngineer

**Sacramento**  
6939 Sunrise Blvd.  
Suite 123  
Citrus Heights, CA 95610  
(916) 726-0612  
(916) 726-3189 (fax)

**NORMAN SCHEEL**  
Structural Engineer  
Email: [norm@nsse.com](mailto:norm@nsse.com)

**ROBERT COON**  
Project Manager  
Email: [rob@nsse.com](mailto:rob@nsse.com)

**PAULO IBANEZ**  
Project Manager  
Email: [paulo@nsse.com](mailto:paulo@nsse.com)

**TERRI SCHNEIDER P.E.**  
Project Engineer  
Email: [terri@nsse.com](mailto:terri@nsse.com)

**TIM SLOAN**  
Project Manager  
Email: [tim@nsse.com](mailto:tim@nsse.com)

**CASANDRA COURTILLET**  
Design Engineer  
Email: [cassandra@nsse.com](mailto:cassandra@nsse.com)

**Davis**  
1623 Fifth Street  
Suite F  
Davis, CA 95616  
(530) 753-5300  
(530) 753-5380

**TRACY HARRIS P.E.**  
Project Engineer  
Email: [tracy@nsse.com](mailto:tracy@nsse.com)

**DARRELL PEREIRA**  
Design Engineer  
Email: [darrell@nsse.com](mailto:darrell@nsse.com)

December 21, 1999

D.R. Horton  
110 Blue Ravine Road Suite 209  
Folsom, CA 95630

**Re: Laguna Vega - Plan 2 (Job #99406)**  
**Shear Walls @ Left Side of House**

To Whom It May Concern:

This letter is to clarify we have reviewed the situation where the shear walls as built are 3" shorter than specified on the plans. We reviewed the calculation and determined that the walls are still adequate as designed and constructed, See the calculation on the following pages.

If you have any questions, please contact Rob Coon.

  
**NORMAN SCHEEL**  
STRUCTURAL ENGINEER



Norman Scheel  
 Structural Engineer  
 6939 Sunrise Boulevard, Suite 123  
 Citrus Heights, CA 95810  
 (916) 726-0612

Project LAGUNA VEGA  
 Client D.R HORTON  
 Job # 99408 Project Mngr. ROB  
 Date 12/21/89  
 Page # 10 OF 13

**Chord Force Calculation**

**P5 RIGHT SIDE OF HOUSE**

Total Wind Load = 6785 # Total Seismic Load = 6382 #

Load	Length	Depth	CT	Splice Specification
6785	50	35	1212	Use 24-16c Nails each side of each splice
Chord Force @ X =		6.0	512	Use Simpson CS16 Strap
Chord Force @ X =		0.0	0	
Chord Force @ X =		0.0	0	
Chord Force @ X =		0.0	0	
Chord Force @ X =		0.0	0	

**Drag Strut Calculation**

Diaphragm Shear = 64 plf Base Shear = 322 plf

SW #	S.W. Length	Location	Strap #	Location	Drag Force	Strap Specification
SW1	2.5800	6.0000	Strap 1	6.00	1976	Use Simpson MSTC28 Strap
SW2	3.2500			0.00		
SW3	4.0800			0.00		
				0.00		

Norman Scheel  
 Structural Engineer  
 6939 Sunrise Boulevard, Suite 123  
 Citrus Heights, CA 95610  
 (916) 726-0612

Project LAGUNA VEGA  
 Client D.R. HORTON  
 Job # 99406 Project Mngr. ROB  
 Date 12/21/99  
 Page # 9 OF 13

**Lateral Analysis Calculations**

**P5 RIGHT SIDE OF HOUSE**

**Wind Load Calculations**

Total Width of Diaphragm = 35 ft  
 Additional Diaphragm Width = 0 ft  
 Plate Height = 9 ft  
 Average Height above Plate = 5 ft  
 Average Building Height = 20 ft  
 Wind Load = 14.28 psf  
 Additional Load = 0 #  
  
 Total Wind Load = 2375 #

**Diaphragm Calculations**

Length of Diaphragm = 50 ft  
 Sheathing = 15/32 in  
 Additional Load @ Diaphragm = 0 #  
 Diaphragm Case ( 1 or 3 ) = 3  
  
 Diaphragm Shear = 64 plf  
 No Blocking Required

**Seismic Load Calculation**

Total Width of Diaphragm = 35 ft  
 Total Length of Diaphragm = 50 ft  
 Additional Diaphragm Width = 0 ft  
 Additional Diaphragm Length = 0 ft  
 Building dead load = 26 #  
 V = 0.1403 #  
 Additional Load = 0 #  
 Seismic load = 3191 #  
 P = 1.0000  
 Total Seismic Load = 3191 #

Front to Back       Side to Side

**Shear Wall Calculations**

# of Shear Walls = 3  
 Total Length of Walls = 9.9167 ft  
  
 Base Shear = 322 plf

**USE SHEAR WALL TYPE 2**

**Overturning Calculations**

Length	Dead Load	Uplift Left	Location From Left	Uplift Right	Location From Left	Total Uplift Left	Total Uplift Right
2.5800	364					2583	2583
3.2500	364					2502	2502
4.0800	364					2401	2401

**Shear Wall Specifications**

#	Length	Type	Holdown Left	Holdown Right
SW1	2.5800	2	STHD10	STHD10
SW2	3.2500	2		
SW3	4.0800	2		



**N**orman  
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Email: [norm@nsse.com](mailto:norm@nsse.com)

**ROBERT COON**  
Project Manager  
Email: [rob@nsse.com](mailto:rob@nsse.com)

**PAULO IBAÑEZ**  
Project Manager  
Email: [paulo@nsse.com](mailto:paulo@nsse.com)

**TERRI SCHNEIDER P.E.**  
Project Engineer  
Email: [terri@nsse.com](mailto:terri@nsse.com)

**TIM SLOAN**  
Project Manager  
Email: [tim@nsse.com](mailto:tim@nsse.com)

**CASANDRA COURTILLET**  
Design Engineer  
Email: [casandra@nsse.com](mailto:casandra@nsse.com)

**Davis**  
1623 Fifth Street  
Suite F  
Davis, CA 95618  
(530) 753-5300  
(530) 753-5380

**TRACY HARRIS P.E.**  
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Email: [tracy@nsse.com](mailto:tracy@nsse.com)

**DARRELL PEREIRA**  
Design Engineer  
Email: [darrel@nsse.com](mailto:darrel@nsse.com)

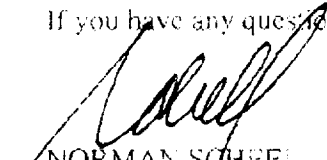
January 10, 2000

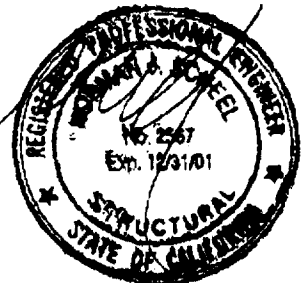
DR. Horton  
110 Blue Ravine Road  
Folsom California 95630

**Re: Laguna Vega (job # 99406)**  
**Inspection Clarifications.**

1. The 2x6 wall at the back of the garage on plan 4 only requires a mstc 28 strap across the plumbing notch. The Blocking due to the cantilever is adequate for the compression forces.
2. The drag truss note on elevation C is not required on this condition. See plan addendum with this letter.
3. Split mudsil. If the mudsil is split less than 50% of the shear wall length it may be repaired using Simpson BP5/8s washers with screws. If it is greater than 50% of the length it will require replacement.
4. Anchor bolts may be epoxied into the foundation without special inspection. We are only concerned with special inspections or testing on holdown anchors due to the high uplift forces. Anchor bolt only require 5" embedment into concrete and are only being used for shear forces which are governed by the lumber values at the sill plate.
5. The sill plate may be removed at shear walls as long as area removed is less than 20% of the shear wall length or 1 stud bay which ever is less.
6. Cut king stud & trimmer at kitchen windows. Plan 2. See sketch this package for repair.

If you have any questions please call Rob Coon.

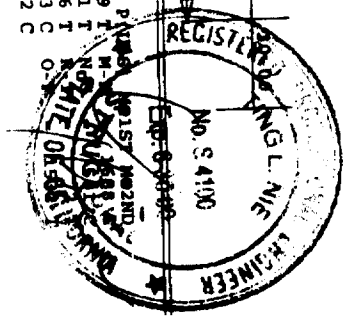
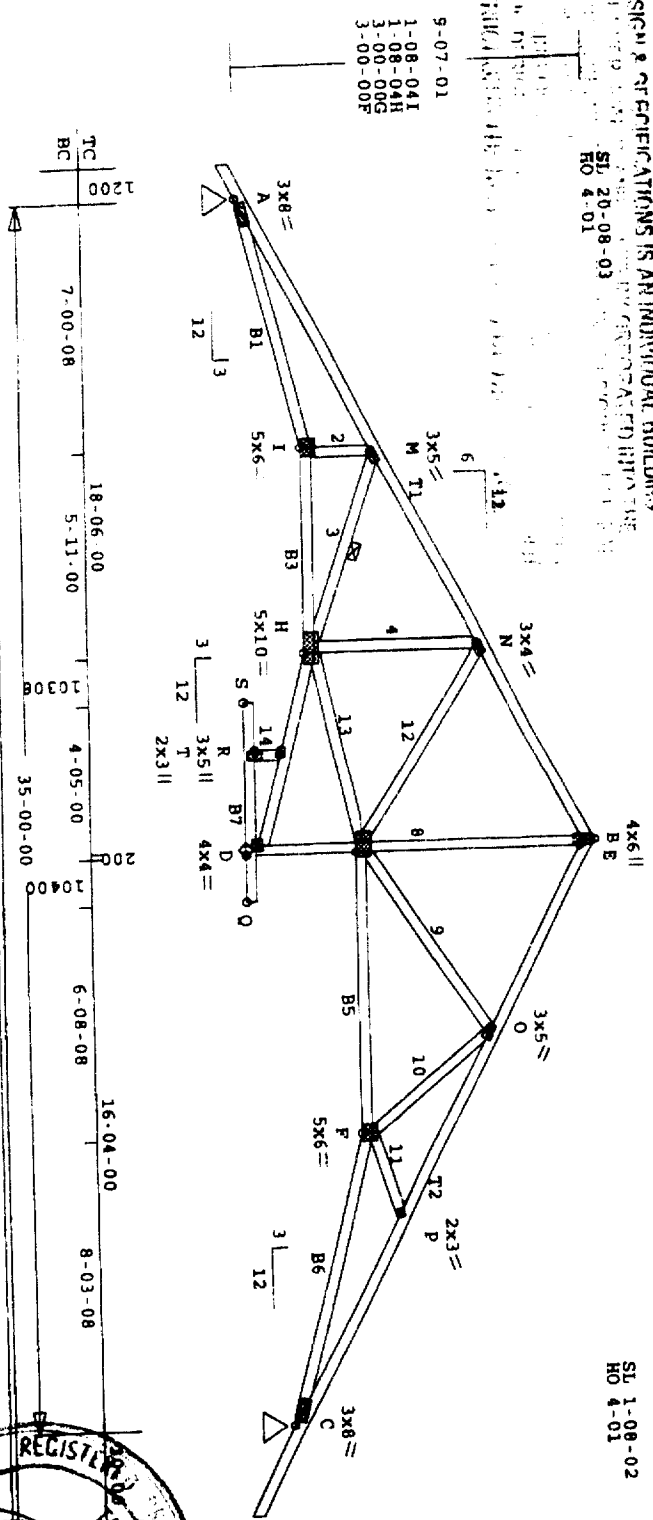
  
NORMAN SCHEEL  
STRUCTURAL ENGINEER



THIS TRUSS DESIGN & SPECIFICATIONS IS AN INDIVIDUAL BUILDING COMPONENT. IT IS NOT TO BE REPRODUCED OR COPIED WITHOUT THE WRITTEN PERMISSION OF THE ENGINEER OF RECORD. THE DESIGN IS SUBJECT TO CHANGE DURING CONSTRUCTION.

SL 20-08-03  
HO 4-01

SL 1-08-02  
HO 4-01



UnStar -- Version 40.0.110  
RUN DATE: 9-15-99

SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 1167 3- 8 C 1213 3- 8

LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD 10.0 7.0  
TOTAL 26.0 21.0 47.0

MEMBR CSI P(LBS) M@1ST M@2ND  
I-M = 959 T 1181 T 1698 T  
H-N = 221 T 2336 T 2336 T  
H-G = 2336 T 2336 T 2336 T  
G-O = 1073 C 0- 0- 0-  
F-P = 272 C

CS1 SIZE LUMBER 1.15FH  
TOP 0.70 2X 4 DFL-#1B 1985  
BTM 0.99 2X 4 DFL-#1B 1985  
WBS 0.99 2X 4 HF-STAN 635  
EXCEPTIONS: 2X 4 HF #1 1640  
H-G 2X 4 DFL-STAN 635  
D-G SAME AS D-G  
G-B SAME AS D-G  
REPETITIVE MEMBER STRESS USED.

LOAD CASE #3 WIND FROM LEFT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD 10.0 7.0  
TOTAL 26.0 21.0 47.0

EXCEPTIONS: -17.4N 14.0  
B-C -17.4N 14.0  
SUPPORT CRITERIA  
LBS LBS  
A C HORZ RLR 0 519 3- 8

DL+LL DEFL = 0.77" IN G-E  
LL DEFL = 0.31" < BRG-SPAN/360  
DL+LL HORZ = 0.50" AT C  
SPAN/DEFL (DL+LL) = 546

LATERAL BRACING:  
TOP CHORD - CONTINUOUS  
BTM CHORD - CONTINUOUS  
ONE BRACE - M-H  
TRUSS SPACING - 24.0 IN.

LOAD CASE #4 WIND FROM RIGHT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD 10.0 7.0  
TOTAL 26.0 21.0 47.0

MEMBR CSI P(LBS) M@1ST M@2ND  
A-M 0.70 4356 C 2551 -982  
M-N 0.36 2528 C 982 -2242  
N-B 0.34 2070 C 2242 -1181  
B-E 0.19 2008 C 1202 -1056  
E-O 0.29 2078 C 1056 -1869  
O-P 0.57 4142 C 1869 137  
P-C 0.60 4461 C -137 -3069

PLATING CONFORMS TO TPI-95  
PLATE VALUES MAY BE VERIFIED  
WITH ROBBINS MANUFACTURING.  
GRIP BASED ON DFL AND HF  
LUMBER USING GROSS AREA TEST.  
GRIP REDUCED 20% FOR M.C-19%  
IN LUMBER.  
PLATES - 20 GAUGE LOCK  
GRIPPIES 486-201 PSI PER PAIR  
INCLUDES 25.0% INCREASE  
TENSION 1319- 465 PLI PER PAIR  
SHEAR 784- 506 PLI PER PAIR

LOAD CASE #1  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD 10.0 7.0  
TOTAL 26.0 21.0 37.0

EXCEPTIONS: -17.4N 14.0  
B-C -11.3N 14.0  
SUPPORT CRITERIA  
LBS LBS  
A C HORZ RLR 0 593 3- 8

MEMBR CSI P(LBS) M@1ST M@2ND  
A-M 0.70 3998 T 1198 -1196  
I-H 0.99 3887 T 950 -3633  
H-T 0.39 27 C -1342 2971  
T-D 0.39 77 C -2971 -774  
R-D 0.12 0 T 0 559  
G-F 0.61 2739 T 1215 901  
F-C 0.68 4089 T 771 -981

JT TYPE PLATE SIZE X Y  
A 2101 3.00 X 6.00 2.9 2.8  
B 3091R 3.00 X 8.00 9.7 2.0  
C 2101 3.00 X 4.00 2.0 1.7  
D 4593 4.00 X 4.00 2.0 1.7  
E 7010 5.00 X 6.00 CTR 3.3  
F 7010 5.00 X 9.00 1.8 0.0  
G 8051 5.00 X 10.00 6.5 3.5  
H 7093 5.00 X 6.00 CTR 3.5  
I 7001 5.00 X 6.00 CTR 3.5  
M 1050 3.00 X 5.00 CTR CTR  
N 1050 3.00 X 4.00 CTR CTR  
O 1010 3.00 X 5.00 1.5 1.5

LOAD CASE #2  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD 10.0 7.0  
BTM CHD 10.0 7.0  
TOTAL 20.0 21.0 31.0

LOAD CASE #5 WIND // RIDGE  
LBS LBS  
A C HORZ RLR 0 686 3- 8

HEEL LEFT RIGHT  
OIN - 2SX OIN - 2SX

CHECKED [Signature]

P 1001 2.00 X 3.00 CTR CTR  
R 4000 3.00 X 5.00 CTR CTR  
P 1001 2.00 X 3.00 CTR CTR

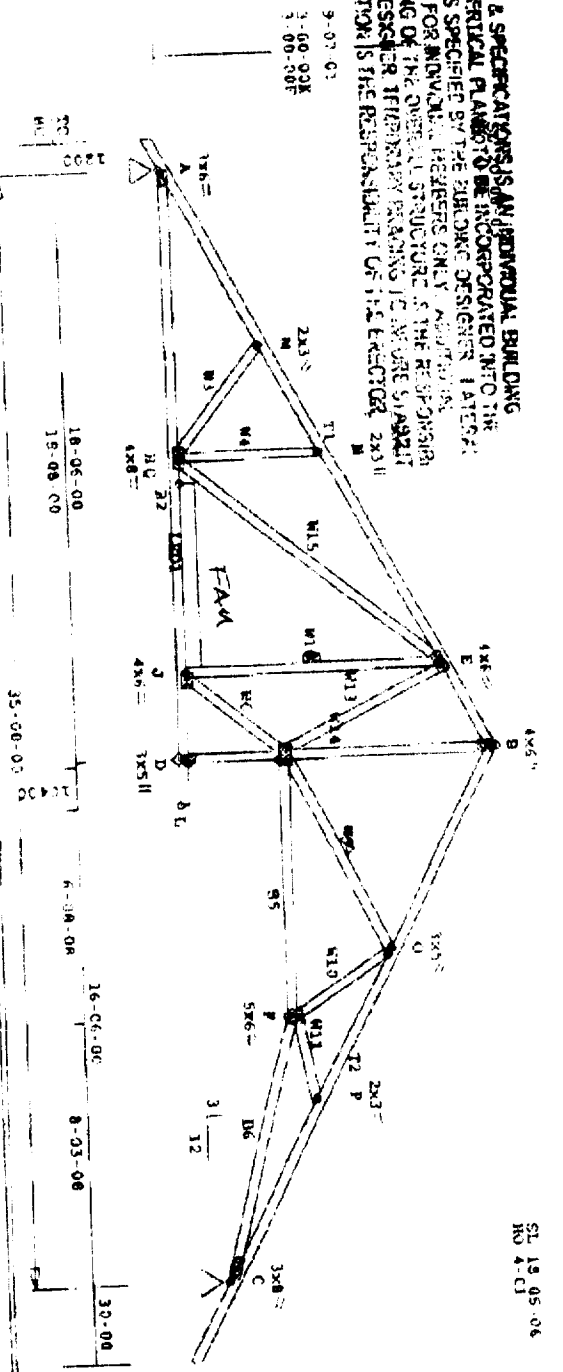
R - PLATE IS ROTATED BY 90 DEG

- NOTES:
1. TRUSSES MANUFACTURED BY WALKER LUMBER CO.
  2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
  3. EMPIRICAL ANALOG IS USED.
  4. WIND LOADS - ANSI/ASCE 7-93 TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM  
WIND SPEED - 80 MPH  
MEAN ROOF HEIGHT - 25'  
EXPOSURE CATEGORY - C  
OCCUPANCY CATEGORY - 1  
OCEANLINE DIST - 100 MILES
  5. BUILDING DESIGNER IS RESPONSIBLE FOR ADEQUATE DESIGN OF TRUSS TO BRG PLATE CONNECTION WHICH ALLOWS 0.50 INCHES OF HORZ. MOVEMENT AT JOINT C

B2R

CONFIDENTIAL

AS TRUSS DESIGN & SPECIFICATIONS IS AN INTEGRAL BUILDING COMPONENT IN A VERTICAL PLANT TO BE INCORPORATED INTO THE BUILDING DESIGN AS SPECIFIED BY THE BUILDING DESIGNER. LATERAL BRACING SHOWN IS FOR INDIVIDUAL MEMBERS ONLY AND NOT FOR THE ENTIRE TRUSS. BRACING OF THE OVERALL STRUCTURE IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. THE RESPONSIBILITY FOR THE DESIGN OF THE BUILDING TRUSS AND FOR THE RESPONSIBILITY OF THE BUILDING DESIGNER IS THE RESPONSIBILITY OF THE ENGINEER. 2/2/11



SL 19 05 06  
NO 4 CI

9-07-01  
1-00-02K  
1-00-00F

THIS DRAWING IS THE PROPERTY OF THE ENGINEER. IT IS TO BE USED ONLY FOR THE PROJECT AND SITE SPECIFICALLY IDENTIFIED HEREIN. ANY REUSE OR MODIFICATION OF THIS DRAWING WITHOUT THE WRITTEN CONSENT OF THE ENGINEER IS STRICTLY PROHIBITED. THE USER OF THIS DRAWING SHALL BE RESPONSIBLE FOR THE ACCURACY OF THE INFORMATION PROVIDED AND FOR THE RESULTS OF ANY DESIGN OR CONSTRUCTION BASED THEREON. THE ENGINEER DOES NOT WARRANT THE ACCURACY OF THE INFORMATION PROVIDED OR THE RESULTS OF ANY DESIGN OR CONSTRUCTION BASED THEREON. THE ENGINEER'S LIABILITY IS LIMITED TO THE DESIGN OF THE TRUSS SYSTEM ONLY AND DOES NOT EXTEND TO THE DESIGN OF THE BUILDING OR TO THE CONSTRUCTION OF THE TRUSS SYSTEM. THE ENGINEER'S LIABILITY IS LIMITED TO THE DESIGN OF THE TRUSS SYSTEM ONLY AND DOES NOT EXTEND TO THE DESIGN OF THE BUILDING OR TO THE CONSTRUCTION OF THE TRUSS SYSTEM.

TRUSS DATE: 9-8-98  
VERSION: 1.0  
JOB NO: 119  
JOB NAME: 119

LOAD CASE #1  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING: LIVE DEAD (PSF)  
TOP CHD: 15.0 14.0 7.0  
BTM CHD: 0.0 0.0 37.0  
TOTAL: 15.0 21.0 37.0  
EXCEPTIONS: 0.0 11.0  
K-J PIN LBS 26-3X  
K-L PIN LBS 18-3X  
K-M PIN LBS 18-3X  
K-N PIN LBS 18-3X  
K-O PIN LBS 18-3X  
K-P PIN LBS 18-3X  
K-Q PIN LBS 18-3X  
K-R PIN LBS 18-3X  
K-S PIN LBS 18-3X  
K-T PIN LBS 18-3X  
K-U PIN LBS 18-3X  
K-V PIN LBS 18-3X  
K-W PIN LBS 18-3X  
K-X PIN LBS 18-3X  
K-Y PIN LBS 18-3X  
K-Z PIN LBS 18-3X

LOAD CASE #2  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING: LIVE DEAD (PSF)  
TOP CHD: 10.0 14.0 7.0  
BTM CHD: 0.0 0.0 37.0  
TOTAL: 10.0 21.0 37.0  
EXCEPTIONS: 0.0 11.0  
K-J PIN LBS 26-3X  
K-L PIN LBS 18-3X  
K-M PIN LBS 18-3X  
K-N PIN LBS 18-3X  
K-O PIN LBS 18-3X  
K-P PIN LBS 18-3X  
K-Q PIN LBS 18-3X  
K-R PIN LBS 18-3X  
K-S PIN LBS 18-3X  
K-T PIN LBS 18-3X  
K-U PIN LBS 18-3X  
K-V PIN LBS 18-3X  
K-W PIN LBS 18-3X  
K-X PIN LBS 18-3X  
K-Y PIN LBS 18-3X  
K-Z PIN LBS 18-3X

LOAD CASE #3  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING: LIVE DEAD (PSF)  
TOP CHD: 10.0 14.0 7.0  
BTM CHD: 0.0 0.0 37.0  
TOTAL: 10.0 21.0 37.0  
EXCEPTIONS: 0.0 11.0  
K-J PIN LBS 26-3X  
K-L PIN LBS 18-3X  
K-M PIN LBS 18-3X  
K-N PIN LBS 18-3X  
K-O PIN LBS 18-3X  
K-P PIN LBS 18-3X  
K-Q PIN LBS 18-3X  
K-R PIN LBS 18-3X  
K-S PIN LBS 18-3X  
K-T PIN LBS 18-3X  
K-U PIN LBS 18-3X  
K-V PIN LBS 18-3X  
K-W PIN LBS 18-3X  
K-X PIN LBS 18-3X  
K-Y PIN LBS 18-3X  
K-Z PIN LBS 18-3X

LOAD CASE #4  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING: LIVE DEAD (PSF)  
TOP CHD: 10.0 14.0 7.0  
BTM CHD: 0.0 0.0 37.0  
TOTAL: 10.0 21.0 37.0  
EXCEPTIONS: 0.0 11.0  
K-J PIN LBS 26-3X  
K-L PIN LBS 18-3X  
K-M PIN LBS 18-3X  
K-N PIN LBS 18-3X  
K-O PIN LBS 18-3X  
K-P PIN LBS 18-3X  
K-Q PIN LBS 18-3X  
K-R PIN LBS 18-3X  
K-S PIN LBS 18-3X  
K-T PIN LBS 18-3X  
K-U PIN LBS 18-3X  
K-V PIN LBS 18-3X  
K-W PIN LBS 18-3X  
K-X PIN LBS 18-3X  
K-Y PIN LBS 18-3X  
K-Z PIN LBS 18-3X

LOAD CASE #5  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING: LIVE DEAD (PSF)  
TOP CHD: 10.0 14.0 7.0  
BTM CHD: 0.0 0.0 37.0  
TOTAL: 10.0 21.0 37.0  
EXCEPTIONS: 0.0 11.0  
K-J PIN LBS 26-3X  
K-L PIN LBS 18-3X  
K-M PIN LBS 18-3X  
K-N PIN LBS 18-3X  
K-O PIN LBS 18-3X  
K-P PIN LBS 18-3X  
K-Q PIN LBS 18-3X  
K-R PIN LBS 18-3X  
K-S PIN LBS 18-3X  
K-T PIN LBS 18-3X  
K-U PIN LBS 18-3X  
K-V PIN LBS 18-3X  
K-W PIN LBS 18-3X  
K-X PIN LBS 18-3X  
K-Y PIN LBS 18-3X  
K-Z PIN LBS 18-3X



CHECKED SEP - 8 1998

QUAN TYPE	SPAN P1-R1	OVERHANGS JOB
5 TR	370000	6 1200 1200
LAGUNA VEGA		
H4135 2		
B3 MARK		

O	1010	4.00	X	6.00	2.6	2.0
	1001	2.00	X	3.00	CTR	CTR
	4000	3.00	X	5.00	CTR	CTR
	1001	2.00	X	3.00	CTR	CTR
S	8001	1.00	X	4.00	CTR	CTR

R - PLATE IS ROTATED BY 90 DEG

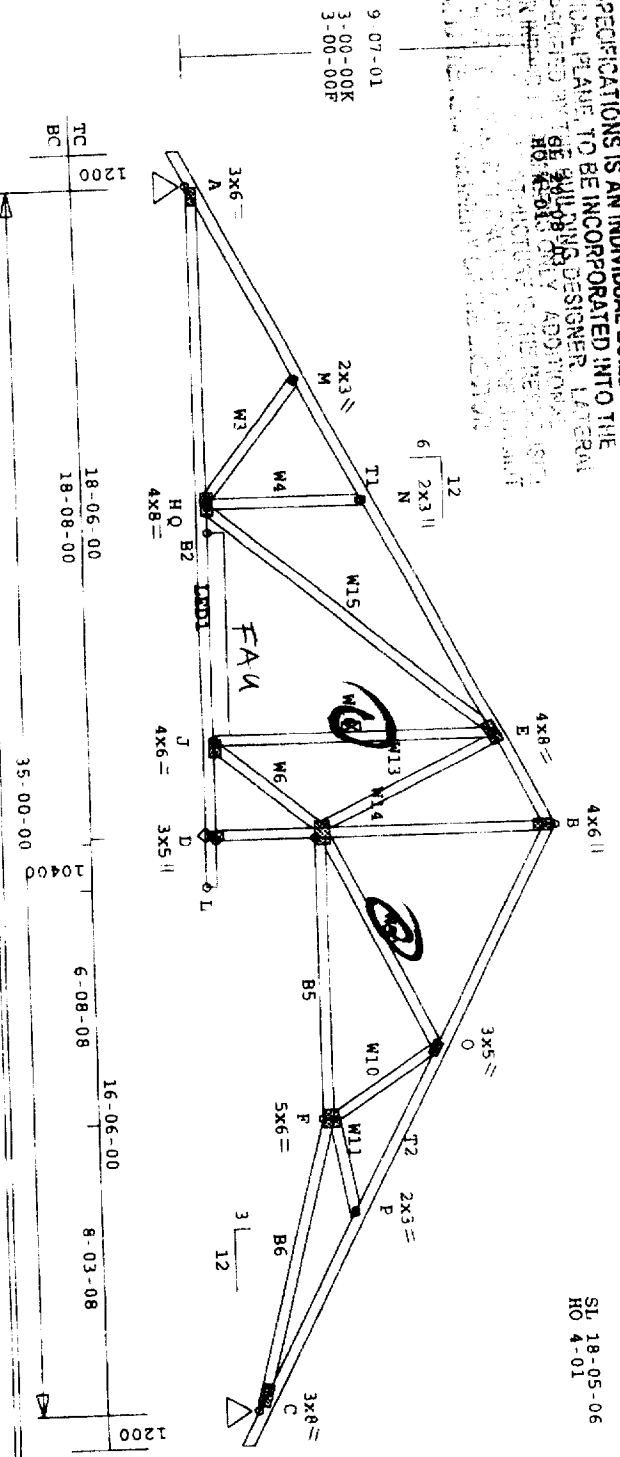
- NOTES:
1. TRUSSES MANUFACTURED BY WALKER LUMBER CO.
  2. ANALYSIS CONFORMS TO TPI-85 (NDS-91).
  3. EMPIRICAL ANALOG IS USED.
  4. WIND LOADS - ANSI/ASCE 7-93 TRUSS IS DESIGNED AS A MAIN WIND-FORCE RES SYSTEM  
WIND SPEED - 80 MPH  
MEAN ROOF HEIGHT - 25'  
EXPOSURE CATEGORY - C  
OCCUPANCY CATEGORY - 1  
OCEAN/LAKE DIST - 100 MILES
  5. BUILDING DESIGNER IS RESPONSIBLE FOR ADEQUATE DESIGN OF TRUSS TO BRG PLATE CONNECTION WHICH ALLOWS 0.49 INCHES OF HORIZ. MOVEMENT AT JOINT C

RECEIVED JUN 16 1999

B3

SL 18-05-06  
HO 4-01

SS DESIGN & SPECIFICATIONS IS AN INDIVIDUAL BUILDING  
ENGINEER IN A VERTICAL PLANE TO BE INCORPORATED INTO THE  
GENERAL DESIGN AND SPECIFICATIONS OF THE BUILDING DESIGNER. LATERAL  
LOADS SHALL BE AS SPECIFIED BY THE BUILDING DESIGNER. ADDITIONAL  
LOADS SHALL BE AS SPECIFIED BY THE BUILDING DESIGNER. THE DESIGNER  
SHALL BE RESPONSIBLE FOR THE DESIGN OF THE BUILDING AND THE  
DESIGN OF THE FOUNDATION. THE DESIGNER SHALL BE RESPONSIBLE FOR  
THE DESIGN OF THE FOUNDATION. THE DESIGNER SHALL BE RESPONSIBLE FOR  
THE DESIGN OF THE FOUNDATION.



Unistat -- Version 40.0.110  
RUN DATE: 9-8-99

H-J 10.0 11.0  
SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 1152 3-8 C 1136 3-8

CSI SIZE LUMBER 1.15FB  
TOP 0.60 2X 4 DFL-#1B 1985  
BTM 0.66 2X 4 DFL-#1B 1985  
WBS 0.85 2X 4 HF-STAN 635  
EXCEPTIONS: 2X 4 HF-STUD 855  
J-K K B D-K SAME AS J-K  
REPETITIVE MEMBER STRESS USED.  
LATERAL BRACING:  
TOP CHORD - CONTINUOUS  
BTM CHORD - CONTINUOUS  
ONE BRACE - J-E K-O  
TRUSS SPACING - 24.0 IN.

LOAD CASE #1  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
BTM CHD 16.0 7.0  
TOTAL 16.0 21.0 37.0  
EXCEPTIONS: 0.0 11.0  
H-J 11.0  
SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 1394 3-8 C 1378 3-8

LOAD CASE #3 WIND FROM LEFT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD 10.0 7.0  
TOTAL 26.0 21.0 47.0  
EXCEPTIONS:  
H-J 10.0 11.0  
A-B -17.4N 14.0  
B-C -17.4N 14.0  
SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 81 0 568 3-8  
C 0 0 568 3-8

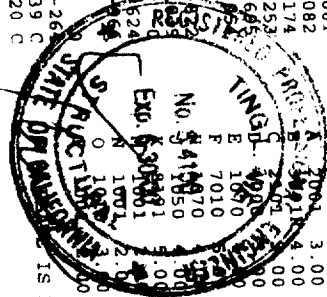
LOAD CASE #5 WIND // RIDGE  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD 10.0 7.0  
TOTAL 26.0 21.0 47.0  
EXCEPTIONS:  
H-J 10.0 11.0  
A-B -17.4N 14.0  
B-C -17.4N 14.0  
SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
A 34 0 494 3-8  
C 0 0 494 3-8

MEMBER CSI P(LBS) M@1ST M@2ND  
A-M 0.36 2215 C 1131 .772  
M-N 0.34 2214 C 773 -2061  
N-F 0.46 2228 C 2061 -2082  
F-G 0.33 2076 C 2082 -1174  
G-H 0.35 2073 C 1075 -2253  
H-I 0.47 4158 C 2253 -695  
I-J 0.60 4458 C -695  
BOTTOM CHORDS  
A-H 0.54 2156 T 512  
H-I 0.46 1470 T 822  
I-J 0.20 32 C 1424  
J-K 0.60 2992 T 916  
K-L 0.66 4082 T 653  
WEBS  
K-B 0.79 1817 T .651  
D-K 0.33 14 C 0  
M-H 213 C H-N 339 C  
H-E = 802 T J-E = 1520 C

MEMBER CSI P(LBS) M@1ST M@2ND  
A-M 0.36 2215 C 1131 .772  
M-N 0.34 2214 C 773 -2061  
N-F 0.46 2228 C 2061 -2082  
F-G 0.33 2076 C 2082 -1174  
G-H 0.35 2073 C 1075 -2253  
H-I 0.47 4158 C 2253 -695  
I-J 0.60 4458 C -695  
BOTTOM CHORDS  
A-H 0.54 2156 T 512  
H-I 0.46 1470 T 822  
I-J 0.20 32 C 1424  
J-K 0.60 2992 T 916  
K-L 0.66 4082 T 653  
WEBS  
K-B 0.79 1817 T .651  
D-K 0.33 14 C 0  
M-H 213 C H-N 339 C  
H-E = 802 T J-E = 1520 C

MEMBER CSI P(LBS) M@1ST M@2ND  
A-M 0.36 2215 C 1131 .772  
M-N 0.34 2214 C 773 -2061  
N-F 0.46 2228 C 2061 -2082  
F-G 0.33 2076 C 2082 -1174  
G-H 0.35 2073 C 1075 -2253  
H-I 0.47 4158 C 2253 -695  
I-J 0.60 4458 C -695  
BOTTOM CHORDS  
A-H 0.54 2156 T 512  
H-I 0.46 1470 T 822  
I-J 0.20 32 C 1424  
J-K 0.60 2992 T 916  
K-L 0.66 4082 T 653  
WEBS  
K-B 0.79 1817 T .651  
D-K 0.33 14 C 0  
M-H 213 C H-N 339 C  
H-E = 802 T J-E = 1520 C

MEMBER CSI P(LBS) M@1ST M@2ND  
A-M 0.36 2215 C 1131 .772  
M-N 0.34 2214 C 773 -2061  
N-F 0.46 2228 C 2061 -2082  
F-G 0.33 2076 C 2082 -1174  
G-H 0.35 2073 C 1075 -2253  
H-I 0.47 4158 C 2253 -695  
I-J 0.60 4458 C -695  
BOTTOM CHORDS  
A-H 0.54 2156 T 512  
H-I 0.46 1470 T 822  
I-J 0.20 32 C 1424  
J-K 0.60 2992 T 916  
K-L 0.66 4082 T 653  
WEBS  
K-B 0.79 1817 T .651  
D-K 0.33 14 C 0  
M-H 213 C H-N 339 C  
H-E = 802 T J-E = 1520 C



CHECK

- NOTES:
1. TRUSSES MANUFACTURED BY -  
WALKER LUMBER CO.
  2. ANALYSIS CONFORMS TO  
TPI (ANSI/TPI 1-1995).
  3. EMPIRICAL ANALOG IS USED.
  4. WIND LOADS - ANSI/ASCE 7-93  
TRUSS IS DESIGNED AS A  
MAIN WIND-FORCE RES SYSTEM  
WIND SPEED - 80 MPH  
MEAN ROOF HEIGHT - 25'  
EXPOSURE CATEGORY - C  
OCCUPANCY CATEGORY - 1  
OCEANLINE DIST - 100 MILES
  5. BUILDING DESIGNER IS  
RESPONSIBLE FOR ADEQUATE  
DESIGN OF TRUSS TO BRG  
PLATE CONNECTION WHICH  
ALLOWS 0.35 INCHES OF  
HORZ. MOVEMENT AT JOINT C

83H

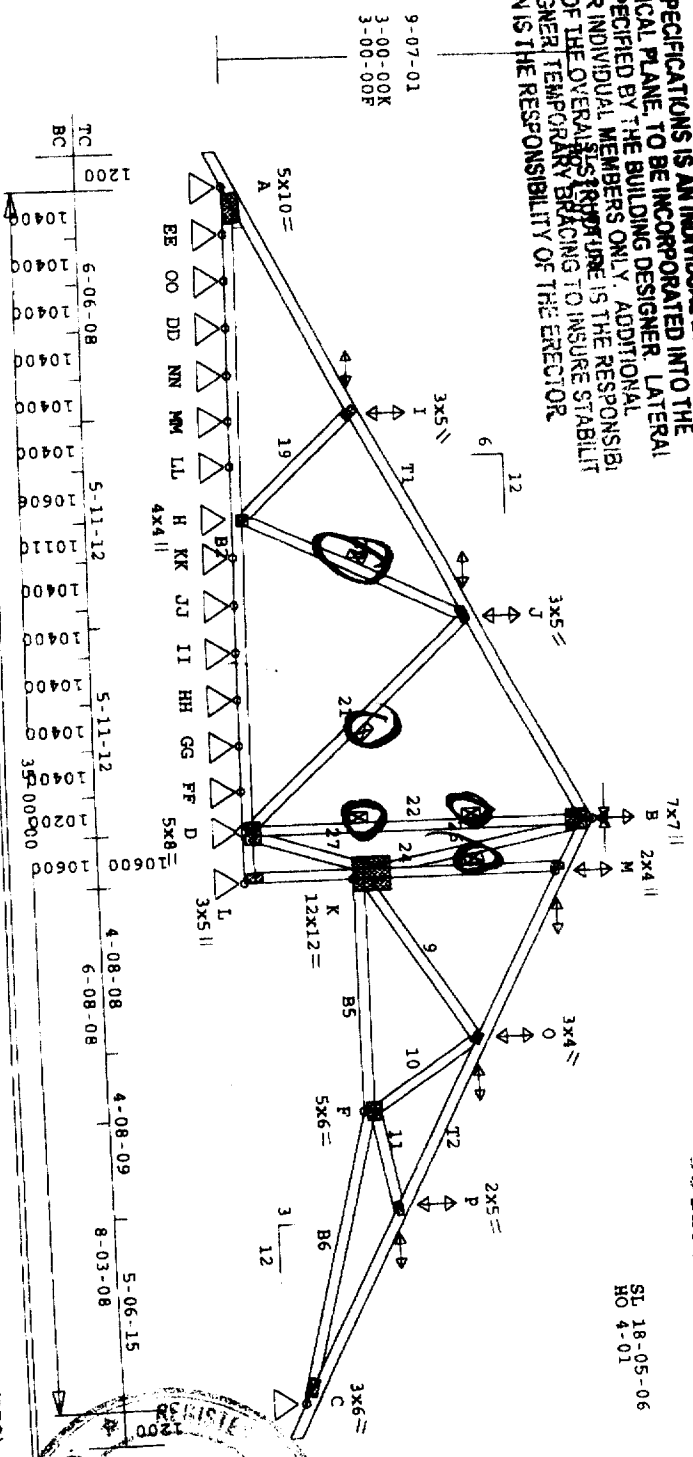
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103

COLLECTOR LOAD 5216 #.

SL 18-05-06  
HO 4-01

DESIGN & SPECIFICATIONS IS AN INDIVIDUAL BUILDING  
MENT, IN A VERTICAL PLANE, TO BE INCORPORATED INTO THE  
ING DESIGN AS SPECIFIED BY THE BUILDING DESIGNER. LATERAL  
ING SHOWS IS FOR INDIVIDUAL MEMBERS ONLY. ADDITIONAL  
PERMANENT BRACING OF THE OVERALL STRUCTURE IS THE RESPONSIBI  
THE BUILDING DESIGNER IS THE RESPONSIBILITY OF THE ERECTOR  
DURING CONSTRUCTION IS THE RESPONSIBILITY OF THE ERECTOR



QUAN TYPE SPAN P1-H1 OVERHANGS JOB  
1 TR 370000 6 1200 1200 LAGUNA VEGA  
H4135 2

UniStar Version 40.0.110  
RUN DATE: 9-9-99

CS1 SIZE LUMBER 1.15FB  
TOP 0.74 2X 4 DFL-#1B 1985  
BTM 0.49 2X 4 DFL-#1B 1985  
WBS 0.74 2X 4 HF-STAN 635  
WDG@A 2X 4 HF-STAN  
EXCEPTIONS: 2X 4 HF-#2 1465  
K-M SAME AS K-M  
L-K SAME AS K-M  
REPETITIVE MEMBER STRESS USBD.

LATERAL BRACING:  
TOP CHORD - CONTINUOUS  
BTM CHORD - CONTINUOUS  
ONE BRACE - H-J J-D B-K  
TWO BRACES - D-B  
TRUSS SPACING - 24.0 IN.

LOAD CASE #1  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD 0.0 7.0  
TOTAL 16.0 21.0 37.0

SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
C 351 3-8

CONTINUOUS BETWEEN JNTS A & L

LOAD CASE #2  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD 0.0 7.0  
TOTAL 16.0 21.0 37.0

TOP CHD 0.0 14.0  
BTM CHD 10.0 7.0  
TOTAL 10.0 21.0 31.0

SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
C 312 3-8

CONTINUOUS BETWEEN JNTS A & L

LOAD CASE #3  
LUMBER STRESS INCREASE: 33.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD 0.0 14.0  
BTM CHD 0.0 7.0  
TOTAL 0.0 21.0 21.0

CONCENTRATED LOADS (LBS)  
I 1773 RIGHT I 886 UP  
J 1110 RIGHT J 585 UP  
K 694 RIGHT K 207  
L 576 RIGHT L 318  
M 874 RIGHT M 437  
N 1473 RIGHT N 738

SUPPORT CRITERIA  
HORZ VERT WIDTH  
LBS LBS IN-SX  
C HORZ RLR 0 466 3-8

CONTINUOUS BETWEEN JNTS A & L

MEMBER CS1 P(LBS) MOIST MO2ND  
A-I 0.74 4675 C 0 -1255  
I-J 0.50 2812 T 1304 -1229  
J-K 0.54 3189 T 1229 0  
K-L 0.46 2735 T 0 -999  
L-M 0.39 2116 T 999 -515  
M-O 0.24 650 C 636 -1053  
O-P 0.25 1540 C 550 0

MEMBER CS1 P(LBS) MOIST MO2ND  
GG-FF 0.19 1744 C 26 -17  
FF-D 0.19 1744 C 17 -35  
D-L 0.01 11 C 35 0  
K-E 0.30 888 C 0 0  
F-C 0.49 1424 T 0 0

I-H = 380 C H-J = 1581 C  
J-D = 1134 C D-B = 1664 C  
B-K = 1266 C D-K = 2612 C  
K-M = 255 C L-K = 3210 C  
K-O = 553 C O-F = 597 T  
F-P = 456 C

DL+LL DEFL. = 0.35\* IN F C  
LL DEFL. = 0.20\* < BRG-SPAN/360  
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.  
PLATE VALUES MAY BE VERIFIED  
WITH ROBBINS MANUFACTURING.  
GRIP BASED ON DFL AND HF  
LUMBER USING GROSS AREA TEST.  
GRIP REDUCED 20% FOR M.C.19%  
IN LUMBER.  
PLATES - 20 GAUGE LOCK  
GRIPPING 466-201 PSI PER PAIR  
INCLUDING 25.0% INCREASE  
TENSION 1339-465 PSI PER PAIR  
SHEAR 784-506 PSI PER PAIR

JT TYPE PLATE SIZE X Y  
A 2016 5.00 X10.00 4.7 3.0  
J 3050R 7.00 X 7.00 2.1 3.4  
C 2101 3.00 X 6.00 8.7 2.8  
D 1070 5.00 X 8.00 4.8 CTR  
F 7010 5.00 X 6.00 CTR

MEMBER CS1 P(LBS) MOIST MO2ND  
A-I 0.74 4675 C 0 -1255  
I-J 0.50 2812 T 1304 -1229  
J-K 0.54 3189 T 1229 0  
K-L 0.46 2735 T 0 -999  
L-M 0.39 2116 T 999 -515  
M-O 0.24 650 C 636 -1053  
O-P 0.25 1540 C 550 0

MEMBER CS1 P(LBS) MOIST MO2ND  
GG-FF 0.19 1744 C 26 -17  
FF-D 0.19 1744 C 17 -35  
D-L 0.01 11 C 35 0  
K-E 0.30 888 C 0 0  
F-C 0.49 1424 T 0 0

I-H = 380 C H-J = 1581 C  
J-D = 1134 C D-B = 1664 C  
B-K = 1266 C D-K = 2612 C  
K-M = 255 C L-K = 3210 C  
K-O = 553 C O-F = 597 T  
F-P = 456 C

DL+LL DEFL. = 0.35\* IN F C  
LL DEFL. = 0.20\* < BRG-SPAN/360  
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.  
PLATE VALUES MAY BE VERIFIED  
WITH ROBBINS MANUFACTURING.  
GRIP BASED ON DFL AND HF  
LUMBER USING GROSS AREA TEST.  
GRIP REDUCED 20% FOR M.C.19%  
IN LUMBER.  
PLATES - 20 GAUGE LOCK  
GRIPPING 466-201 PSI PER PAIR  
INCLUDING 25.0% INCREASE  
TENSION 1339-465 PSI PER PAIR  
SHEAR 784-506 PSI PER PAIR

JT TYPE PLATE SIZE X Y  
A 2016 5.00 X10.00 4.7 3.0  
J 3050R 7.00 X 7.00 2.1 3.4  
C 2101 3.00 X 6.00 8.7 2.8  
D 1070 5.00 X 8.00 4.8 CTR  
F 7010 5.00 X 6.00 CTR

MEMBER CS1 P(LBS) MOIST MO2ND  
A-I 0.74 4675 C 0 -1255  
I-J 0.50 2812 T 1304 -1229  
J-K 0.54 3189 T 1229 0  
K-L 0.46 2735 T 0 -999  
L-M 0.39 2116 T 999 -515  
M-O 0.24 650 C 636 -1053  
O-P 0.25 1540 C 550 0

MEMBER CS1 P(LBS) MOIST MO2ND  
GG-FF 0.19 1744 C 26 -17  
FF-D 0.19 1744 C 17 -35  
D-L 0.01 11 C 35 0  
K-E 0.30 888 C 0 0  
F-C 0.49 1424 T 0 0

I-H = 380 C H-J = 1581 C  
J-D = 1134 C D-B = 1664 C  
B-K = 1266 C D-K = 2612 C  
K-M = 255 C L-K = 3210 C  
K-O = 553 C O-F = 597 T  
F-P = 456 C

DL+LL DEFL. = 0.35\* IN F C  
LL DEFL. = 0.20\* < BRG-SPAN/360  
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI.  
PLATE VALUES MAY BE VERIFIED  
WITH ROBBINS MANUFACTURING.  
GRIP BASED ON DFL AND HF  
LUMBER USING GROSS AREA TEST.  
GRIP REDUCED 20% FOR M.C.19%  
IN LUMBER.  
PLATES - 20 GAUGE LOCK  
GRIPPING 466-201 PSI PER PAIR  
INCLUDING 25.0% INCREASE  
TENSION 1339-465 PSI PER PAIR  
SHEAR 784-506 PSI PER PAIR

JT TYPE PLATE SIZE X Y  
A 2016 5.00 X10.00 4.7 3.0  
J 3050R 7.00 X 7.00 2.1 3.4  
C 2101 3.00 X 6.00 8.7 2.8  
D 1070 5.00 X 8.00 4.8 CTR  
F 7010 5.00 X 6.00 CTR

CHECKED BY: 0 4/99

BS



1010R 4.00 X 4.00 CTR 2.5  
 1001 3.00 X 5.00 3.2 CTR  
 1010 3.00 X 5.00 1.6 CTR  
 K 8191 12.00 X 12.00 6.0 1.7  
 L 4000 3.00 X 5.00 CTR CTR  
 M 1001 2.00 X 4.00 CTR CTR  
 O 1010 3.00 X 4.00 CTR CTR  
 P 1001 2.00 X 5.00 3.5 CTR

DD  
 EE  
 FF  
 GG  
 HH  
 II  
 JJ  
 KK  
 LL  
 MM  
 NN  
 OO

R - PLATE IS ROTATED BY 90 DEG

- NOTES:
1. TRUSSES MANUFACTURED BY - WALKER LUMBER CO.
  2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
  3. TIE-IN LOADS SHOWN WITHOUT DAMAGE TO TRUSS.
  4. PREVENT TRUSS ROTATION AT ALL BEARING LOCATIONS.
  5. SHIM EACH BEARING WALL AS REQUIRED TO INSURE FULL BEARING CONTACT WITH TRUSS.
  6. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 6499 LBS.

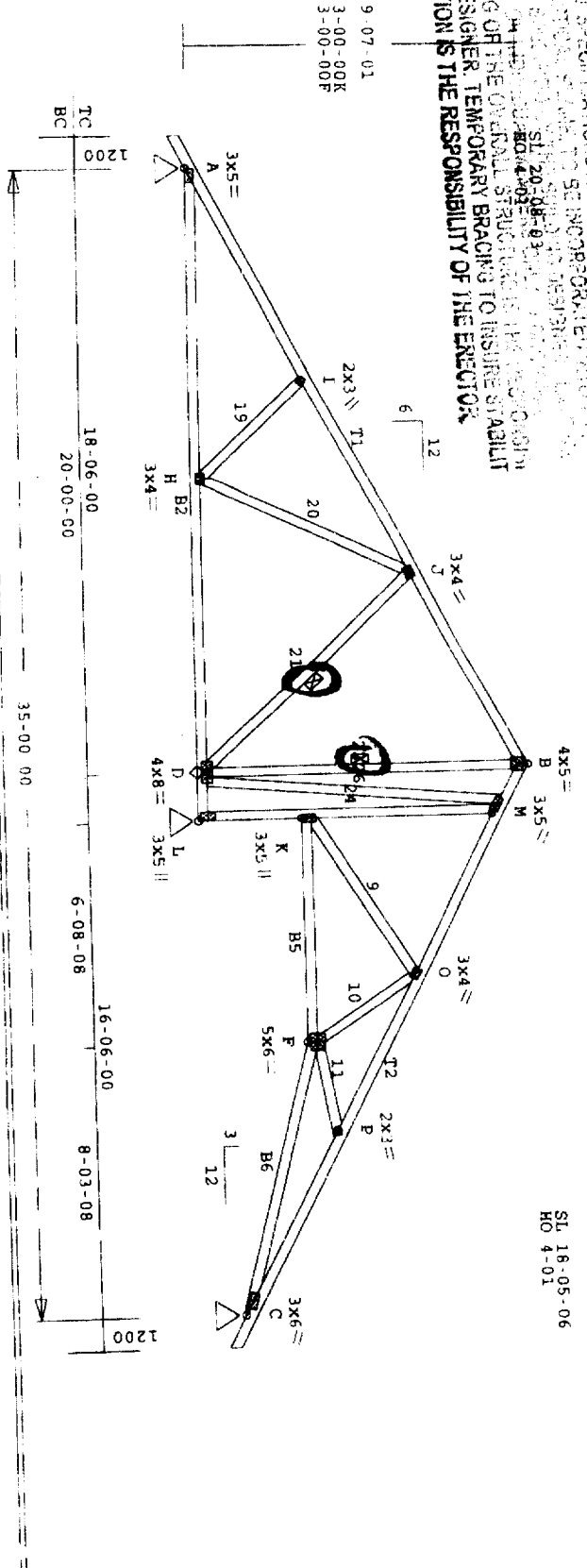
CHECKED NGV - 9 1999

51

THIS DESIGN SPECIFICATION IS AN INDIVIDUAL BUILDING PERMIT DESIGN. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE PERMANENT BRACING OF THE OVERALL STRUCTURE, THE PROVISION OF THE BUILDING DESIGNER TEMPORARY BRACING TO INSURE STABILITY DURING CONSTRUCTION IS THE RESPONSIBILITY OF THE ERECTOR.

SL 16-05-06  
HO 4-01

9-07-01  
3-00-00K  
3-00-00K



UnitStar -- Version 40.0.110  
RUN DATE: 9-8-99  
CSI SIZE LUMBER 1.15FB  
TOP 0.40 2X 4 DFL-#1B 1985  
BTM 0.60 2X 4 DFL-#1B 1985  
WBS 0.62 2X 4 HF-STAN 635  
REPETITIVE MEMBER STRESS USED.

LATERAL BRACING:  
TOP CHORD - CONTINUOUS  
BTM CHORD - CONTINUOUS  
ONE BRACE - J D D-B  
TRUSS SPACING - 24.0 IN.

LUMBER CASE #1  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD 0.0 7.0  
TOTAL 16.0 21.0 37.0  
SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
C 520 3-8 A 708 3-8

LUMBER CASE #2  
LUMBER STRESS INCREASE: 25.0%  
PLATE STRESS INCREASE: 25.0%  
LOADING LIVE DEAD (PSF)  
TOP CHD 0.0 14.0  
BTM CHD 10.0 7.0  
TOTAL 10.0 21.0 31.0  
SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
C 1097 3-8 A 596 3-8

LUMBER CASE #3 WIND FROM LEFT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD 10.0 7.0  
TOTAL 26.0 21.0 47.0  
EXCEPTIONS:  
A-B -11.3N 14.0  
B-C -17.4N 14.0  
SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
C 1318 3-8 A 708 3-8

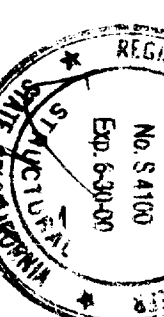
LUMBER CASE #4 WIND FROM RIGHT  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD 10.0 7.0  
TOTAL 26.0 21.0 47.0  
EXCEPTIONS:  
A-B -17.4N 14.0  
B-C -11.3N 14.0  
SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
C 147 3-8 A 243 3-8

LUMBER CASE #5 WIND // RIDGE  
LUMBER STRESS INCREASE: 33.3%  
PLATE STRESS INCREASE: 33.3%  
LOADING LIVE DEAD (PSF)  
TOP CHD 16.0 14.0  
BTM CHD 10.0 7.0  
TOTAL 26.0 21.0 47.0  
EXCEPTIONS:  
A-B -17.4N 14.0  
B-C -11.3N 14.0  
SUPPORT CRITERIA  
JT REACT WIDTH JT REACT WIDTH  
LBS IN-SX LBS IN-SX  
C 1097 3-8 A 596 3-8

MEMBER CST P(LBS) M@1ST M@2ND  
A-1 0.40 1121 C 0 2594 -2528  
I-J 0.39 852 C 2594 -2528  
J-B 0.34 180 T -629 0  
B-M 0.22 221 T -33 354  
M-O 0.23 261 T -33 354  
O-P 0.24 994 C 1142 -1562  
P-C 0.29 1465 C 1562 0  
BOT TOM CHORDS  
A-H 0.60 1018 T 0 -1501  
H-D 0.55 558 T 1501 -497  
D-L 0.22 15 C 497 0  
K-F 0.36 530 T 0 0  
F-C 0.56 1361 T 0 0  
WEBS  
L-K 0.32 1336 C 0 -513  
K-M 0.62 929 C 513 544 T  
I-H = 376 C H-J = 544 T  
J-D = 675 C D-B = 178 C  
D-M = 625 T K-O = 659 C  
O-F = 654 T F-P = 450 C

PLATING CONFORMS TO TP1-95  
PLATE VALUES MAY BE VERIFIED  
WITH ROBINS MANUFACTURING.  
GRIP BASED ON DFL AND HF  
LUMBER USING GROSS AREA TEST.  
GRIP REDUCED 20% FOR M.C-194  
IN LUMBER.  
PLATES - 20 GAUGE LOCK  
GRIPPING 486-201 PSI PER PAIR  
INCLUDES 25.0% INCREASE  
TENSION 1339 465 PLI PER PAIR  
SHEAR 784 506 PLI PER PAIR

JT	TYPE	PLATE	SIZE	X	Y
A	2001	3.00	X 5.00	6.5	3.5
B	3001	4.00	X 5.00	2.2	2.2
C	2101	3.00	X 6.00	8.7	2.8
D	1074	4.00	X 8.00	CTR	CTR
E	7010	5.00	X 6.00	CTR	CTR
F	1010	3.00	X 4.00	CTR	CTR
G	1001	2.00	X 3.00	CTR	CTR
H	1019	3.00	X 4.00	CTR	CTR
I	8010	3.00	X 5.00	CTR	CTR
J	8010	3.00	X 5.00	CTR	CTR
K	4000	3.00	X 5.00	CTR	CTR
L	1011	3.00	X 5.00	CTR	CTR
M	1010	3.00	X 5.00	CTR	CTR
N	1010	3.00	X 5.00	CTR	CTR
O	1000	3.00	X 5.00	CTR	CTR



CHECKED 9-9-99

1. TRUSSES MANUFACTURED BY  
WALKER LUMBER CO.
2. ANALYSIS CONFORMS TO  
TPI (ANSI/TPI 1-1995).
3. WIND LOADS - ANSI/ASCE 7-93  
TRUSS IS DESIGNED AS A  
MAIN WIND-FORCE RES SYSTEM  
WIND SPEED - 80 MPH  
MEAN ROOF HEIGHT - 25'  
EXPOSURE CATEGORY - C  
OCCUPANCY CATEGORY - 1  
OCEANLINE DIST - 100 MILES

CHECKED SEP - 8 1993

DL