

CITY OF SACRAMENTO

1231 I Street, Sacramento, CA 95814

Permit No: 0406598

Insp Area: 1

Thos Bros: 298A4

Site Address: 4831 JERRY WY SAC

Parcel No: 004-0092-022

Sub-Type: REM

Housing (Y/N):

CONTRACTOR

OWNER

SHAWN KILLAM
4800 D ST
SACRAMENTO CA 95819

ARCHITECT

Nature of Work: ADD 296 SF FRONT PORCH TO EXISTING DWELLING.

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class _____ License Number _____ Date _____ Contractor Signature _____

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. _____ B & PC for this reason: _____

Date 4/29/04 Owner Signature [Signature]

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 4/29/04 Applicant/Agent Signature [Signature]

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier _____ Policy Number _____ Exp Date _____

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

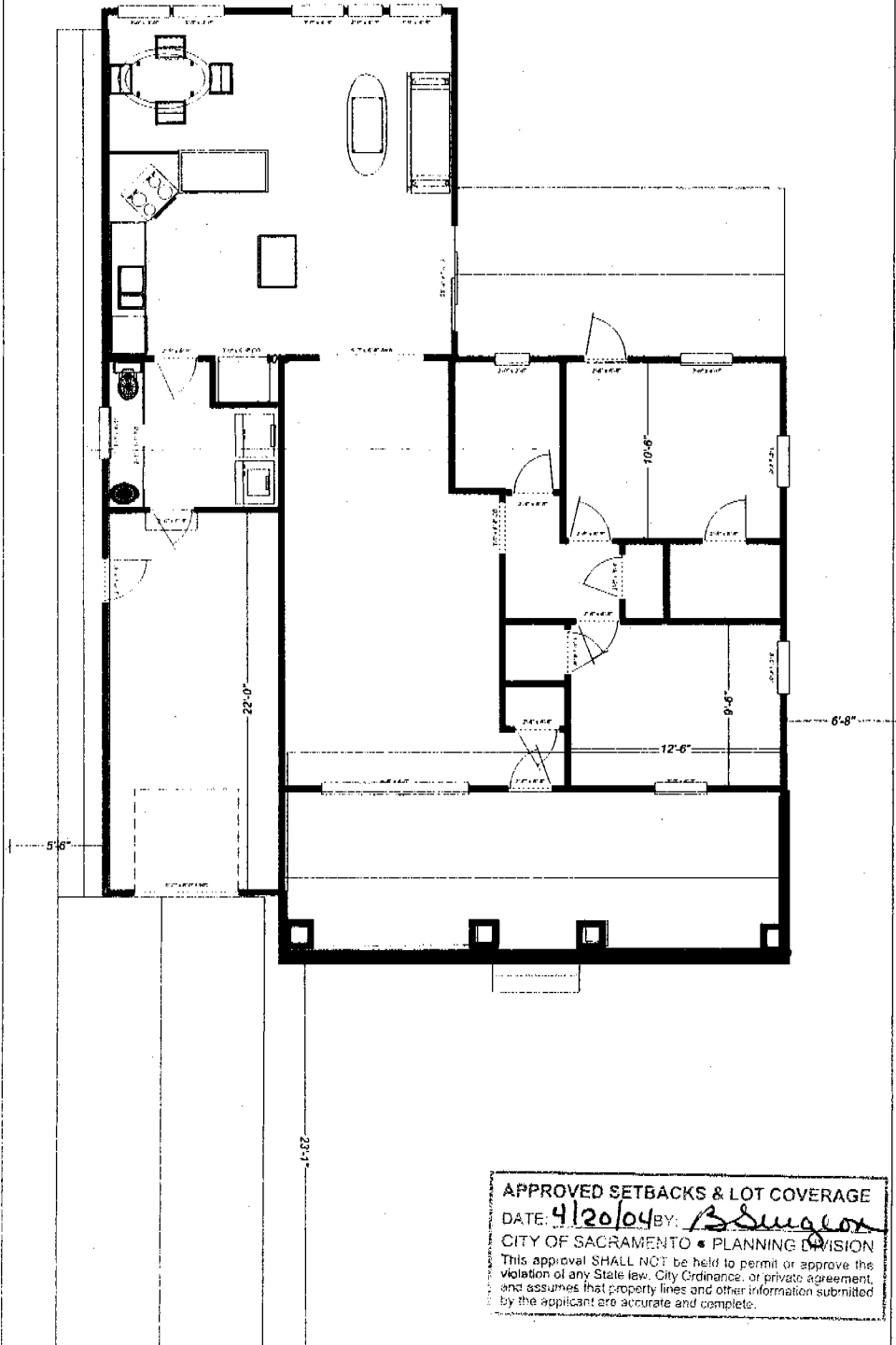
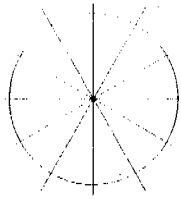
Date 4/29/04 Applicant Signature [Signature]

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

City of Sacramento Planning Division
PLANNING REVIEW FOR BUILDING PERMIT SUBMITTAL

ADDRESS: 4831 Jerry Way	APN: 004-0092-022
DRPB AREA / PUD / SPD: None	ZONING: R-1
EXISTING LAND USE: RSF	
PROPOSED USE: Remove existing concrete handicapped ramp and install new front porch (29.6 x 10)	
PLANNING STAFF WILL CHECK ONE OR MORE OF THE ITEMS BELOW:	
<input type="checkbox"/>	Planning review is NOT required.
<input type="checkbox"/>	Use is NOT allowed; applicant CANNOT submit for plan check.
<input type="checkbox"/>	Requires APPLICATION(s): PC ZA IR ER DR PB Required Planning application must be submitted <i>before</i> project can be submitted for plan check.
<input type="checkbox"/>	Application(s) IN PROGRESS: Applicant may submit for concurrent building permit plan check, at applicant's risk. Building Division must check with Planning staff and/or SITE before issuing building permit.
<input type="checkbox"/>	Application(s) COMPLETED: Building permit must conform to approved plans and comply with all conditions of approval. Do NOT issue building permit prior to end of 10 day appeal period.
<input type="checkbox"/>	Plans may be submitted for plan check. Plan checker(s) shall confirm compliance with Zoning Ordinance requirements and all applicable development standards <i>prior to issuance</i> of building permit.
<input checked="" type="checkbox"/>	Meets setback & lot coverage requirements as shown on site plan provided.
<input checked="" type="checkbox"/>	Plans to be submitted have been stamped/signed by Planning counter staff.
<input type="checkbox"/>	Route to SITE for plan check and inspection.
<input type="checkbox"/>	Preliminary review ONLY; the information on this form must be reviewed again and confirmed at the time of building permit submittal.
COMMENTS: Lot area = 5150 (Metroscan). Existing lot coverage = 1450 + 296 (proposed) = 1746 $1746 / 5150 = 34\%$ total lot coverage. Front setback at 23' due to averaging of adjacent parcels. (24.6 & 21.2) Front setback is at average 23'. Meets all lot coverage and setback requirements. No additional Planning entitlements required.	
DATE: 04/20/04	BY: Bonnie Surgeon



proposed

APPROVED SETBACKS & LOT COVERAGE
DATE: 4/20/04 BY: B. Suggs
CITY OF SACRAMENTO • PLANNING DIVISION
This approval SHALL NOT be held to permit or approve the violation of any State law, City Ordinance, or private agreement, and assumes that property lines and other information submitted by the applicant are accurate and complete.

SAL KADDORAH, P.E.

211 Yacht Club Way # 339
Redondo beach, Ca 90277
Phone (310)937-8725
Fax (310)937-8495

Page T1 of 24

STRUCTURAL CALCULATIONS

FOR

“KILLAM” porch extension

4831 Jerry way

Sacramento, Ca

ISSUED
City of Sacramento
APR 29 2004
PERMIT CENTER

Date : April 22, 2004

Job # 04-011



This set of plans and specifications must be kept on the job at all times and it is unlawful to make any changes or alterations from the same without written permission from the Building Inspection Division.

The approval of this plan and specification SHALL NOT be held to permit or approve the violation of any City Ordinance or State Law.

TMO 4/29/04

CITY COPY



4/23/04

1.0 Design Criteria:

Code: 2001 California Building Code

Timber: Douglas Fir-Larch (DF-L), WWSA or WCLIB
2x Wall Framing: DF-L #2 (unless noted otherwise)
2x Rafters & Joists: DF-L #2 "
Posts & Beams: DF-L #1 "

Glue-Lam Beams: Simple Span: Grade 24F-V4 (DF/DF)
Cantilevers: Grade 24F-V8 (DF/DF)

Sheathing: Min. APA-Rated Sheathing, Exposure 1, Plywood or OSB (U.N.O.)

Engineered Framing: Wood I-Joists: TJI/Pro 150,250,350,550 - NER-200
LVL, PSL 1.9E Microllam, 2.0E Parallam - NER-481

Concrete: Compressive Strength @ 28 days per ASTM C39-96:
Footings: $f'_c = 2500$ psi
Grade Beams: $f'_c = 3000$ psi

Concrete Block: Grade N-I per ASTM C90-95, $f'_m = 1500$ psi per ASTM E447-92

Mortar: Type S Mortar Cement per ASTM C270-95, Min. $f'_m = 1800$ psi @ 28 days.

Grout: Coarse Grout w/ 3/8" Max. Aggregate per ASTM C476-91,
Min. $f'_m = 2000$ psi @ 28 days.

Reinforcing Steel: #4 & Larger: ASTM A615-60 ($F_y = 60$ ksi)
#3 & Smaller: ASTM A615-40 ($F_y = 40$ ksi)

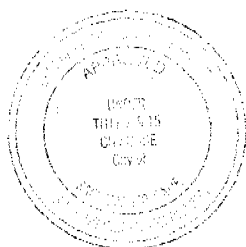
Structural Steel: 'W' Shapes: ASTM A992, $F_y = 50-65$ ksi
Plates, Angles, Channels: ASTM A36, $F_y = 36$ ksi
Tube Shapes: ASTM A500, Grade B, $F_y = 46$ ksi
Pipe Shapes: ASTM A53, Grade B, $F_y = 35$ ksi

Welding Electrodes: Structural Steel: E70-T6
A615-60 Rebar: E90 Series

Bolts: A307 Quality Minimum.

Soils: 1000 psf Bearing Pressure

References:



This set of plans and specifications shall be kept on the job at all times and shall be subject to inspection and approval by the Building Department. The approval of the Building Department is required for the construction of any part of the building. The approval of the Building Department is required for the construction of any part of the building. The approval of the Building Department is required for the construction of any part of the building.

SAL KADDORAH, P.E.

211 yacht club way # 339
redondo beach, ca 90277
(310)937-8725

3

LOADS

<u>ROOF:</u>	4.0
COMPOSITION	1.5
PLYWOOD	2.2
FRAMING	1.5
INSULATION	2.8
5/8" GYPBOARD	2.0
MISCELLANEOUS	2.0
DEAD LOAD=====	14.0
LIVE LOAD=====	16.0
TOTAL LOAD=====	30.0

<u>FLOOR:</u>	0.0
LIGHT WT. CONC	3.4
PLYWOOD	3.3
FRAMING	0.5
INSULATION	2.8
5/8" GYPBOARD	2.0
MISCELLANEOUS	2.0
DEAD LOAD=====	12.0
LIVE LOAD=====	40.0
TOTAL LOAD=====	52.0

<u>EXTERIOR WALLS:</u>	10.0
STUCCO	1.1
PLYWOOD	1.1
FRAMING	0.5
INSULATION	2.8
5/8" GYPBOARD	0.5
MISCELLANEOUS	0.5
DEAD LOAD=====	16.0

<u>EXIT BALCONIES/COURTS:</u>	100
LIVE LOAD=====	

CEILING:

PLYWOOD	0.0
FRAMING	2.2
INSULATION	0.5
5/8" GYPBOARD	2.8
MISCELLANEOUS	0.5
DEAD LOAD=====	6.0
LIVE LOAD=====	10.0
TOTAL LOAD=====	16.0

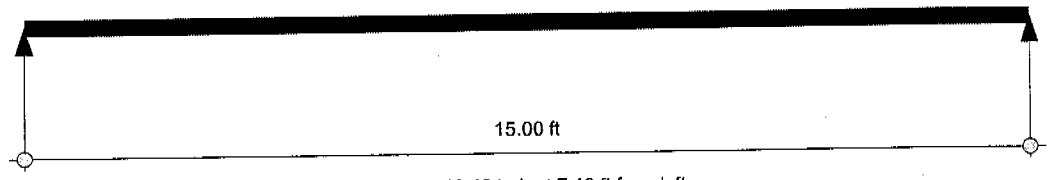
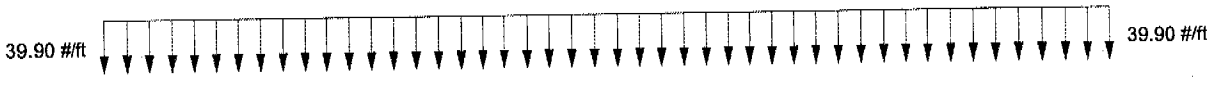
<u>DECKS:</u>	10.0
LIGHT WT. CONC	3.2
PLYWOOD	1.5
FRAMING	0.5
INSULATION	2.8
5/8" GYPBOARD	2.0
MISCELLANEOUS	2.0
DEAD LOAD=====	20.0
LIVE LOAD=====	60.0
TOTAL LOAD=====	80.0

INTERIOR WALLS:

FRAMING	1.4
INSULATION	0.5
5/8" GYPBOARD	5.6
MISCELLANEOUS	0.5
DEAD LOAD=====	8.0

<u>MOVEABLE PARTITIONS:</u>	20.0
LIVE LOAD=====	

RAFTER DESIGN



Mmax = 13.46 in-k at 7.49 ft from left

Dmax = -0.5963 in at 7.49 ft from left

Rl:max = 299.249 lbs

Rr:max = 299.249 lbs

Title :
 Dsgnr:
 Description :

Job # 5
 Date: 2:53PM, 22 APR 04

Scope :

Rev: 560100
 User: KW-0605313, Ver 5.6.1, 25-Oct-2002
 (c)1983-2002 ENERCALC Engineering Software

Timber Beam & Joist

f:\ec55\killam.porch.ecw:Calculations

Description roof rafters & beams

Timber Member Information Calculations are designed to 1997 NDS and 1997 UBC Requirements

Timber Section		porch rafters	beam # 1	beam # 2
		2x8	4x10	4x10
Beam Width	in	1.500	3.500	3.500
Beam Depth	in	7.250	9.250	9.250
Le: Unbraced Length	ft	2.00	2.00	2.00
Timber Grade		Douglas Fir - Larch, Douglas Fir - Larch, Douglas Fir - Larch,		
Fb - Basic Allow	psi	875.0	875.0	875.0
Fv - Basic Allow	psi	95.0	95.0	95.0
Elastic Modulus	ksi	1,600.0	1,600.0	1,600.0
Load Duration Factor		1.250	1.250	1.250
Member Type		Sawn	Sawn	Sawn
Repetitive Status		Repetitive	No	No

Center Span Data

	ft	15.00	10.00	12.00
Span	ft	15.00	10.00	12.00
Dead Load	#/ft	18.60	168.00	56.00
Live Load	#/ft	21.30	192.00	64.00

Results Ratio = 0.6960 0.8274 0.3972

Mmax @ Center	in-k	13.47	54.00	25.92
@ X =	ft	7.50	5.00	6.00
fb : Actual	psi	1,024.8	1,081.9	519.3
Fb : Allowable	psi	1,472.3	1,307.6	1,307.6
		Bending OK	Bending OK	Bending OK
fv : Actual	psi	38.0	70.7	29.1
Fv : Allowable	psi	118.8	118.8	118.8
		Shear OK	Shear OK	Shear OK

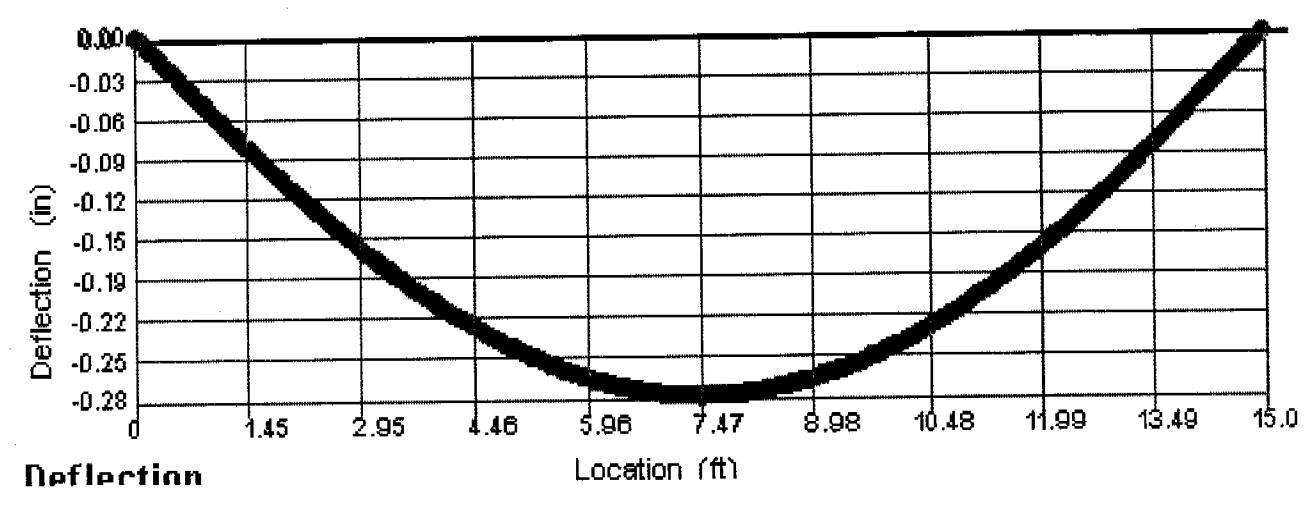
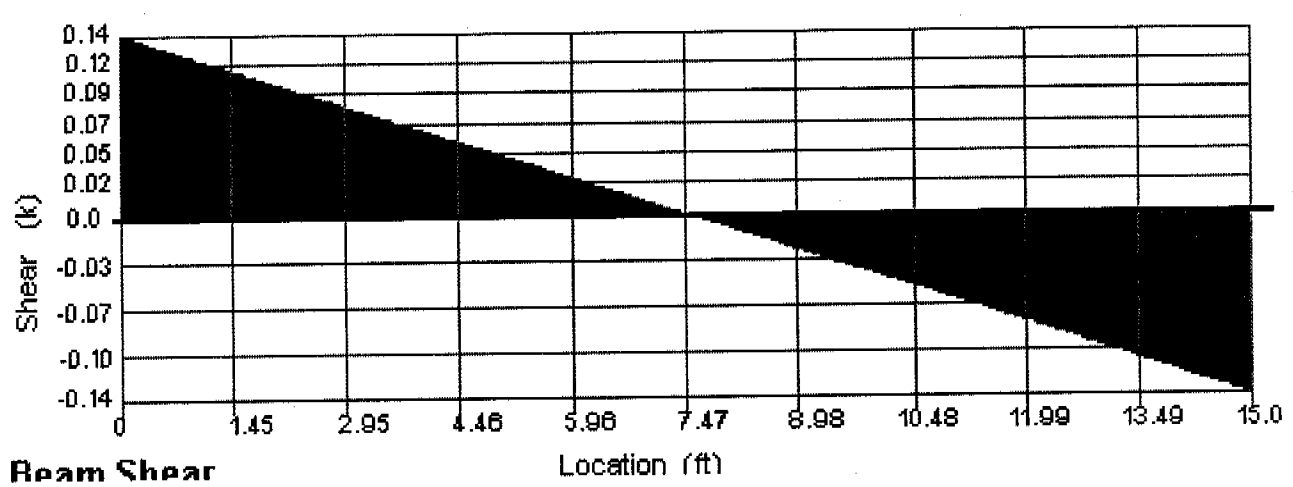
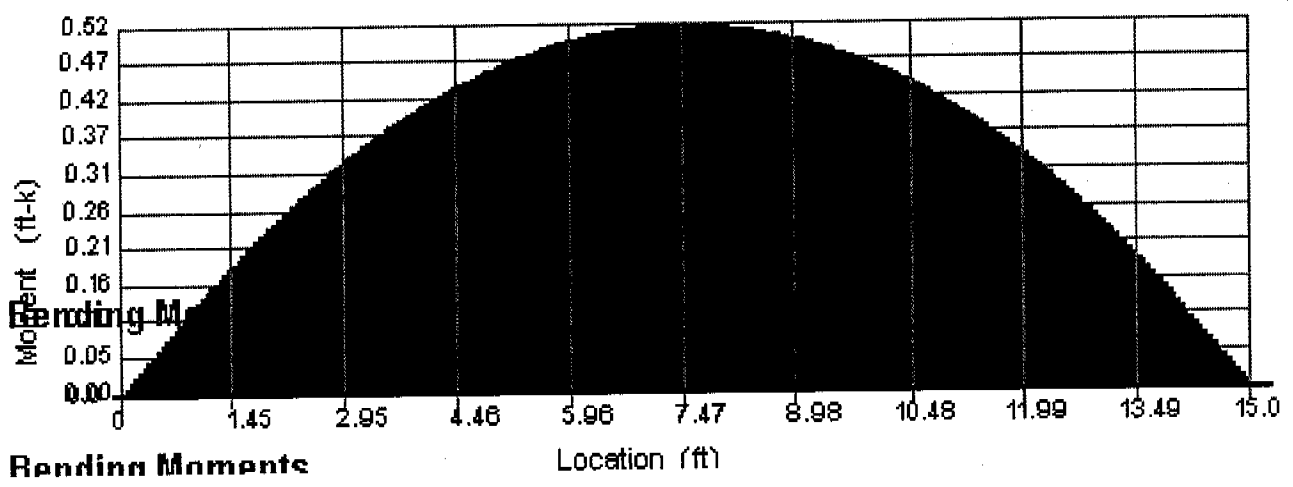
Reactions

@ Left End	DL	lbs	139.50	840.00	336.00
	LL	lbs	159.75	960.00	384.00
	Max. DL+LL	lbs	299.25	1,800.00	720.00
@ Right End	DL	lbs	139.50	840.00	336.00
	LL	lbs	159.75	960.00	384.00
	Max. DL+LL	lbs	299.25	1,800.00	720.00

Deflections Ratio OK Deflection OK Deflection OK

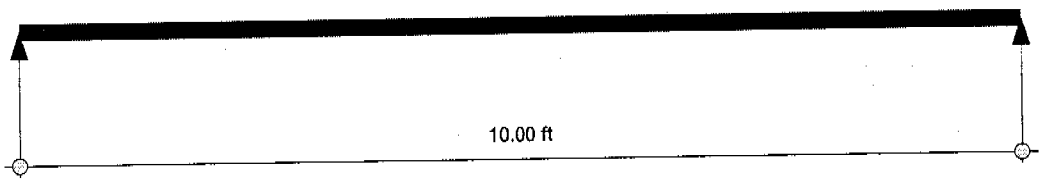
Center DL Defl	in	-0.278	-0.102	-0.071
L/Defl Ratio		647.5	1,172.6	2,035.7
Center LL Defl	in	-0.318	-0.117	-0.081
L/Defl Ratio		565.5	1,026.0	1,781.2
Center Total Defl	in	-0.596	-0.219	-0.152
Location	ft	7.500	5.000	6.000
L/Defl Ratio		301.9	547.2	950.0





BEAM #1

360.00 #/ft  360.00 #/ft



$M_{max} = 53.99 \text{ in-k at } 4.99 \text{ ft from left}$
 $D_{max} = -0.2193 \text{ in at } 4.99 \text{ ft from left}$

$R_l: max = 1799.999 \text{ lbs}$

$R_r: max = 1799.999 \text{ lbs}$

Title :
 Dsgnr:
 Description :

Job #
 Date: 2:53PM, 22 APR 04

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Scope :

Rev: 560100
 User: KW-0605313, Ver 5.6.1, 25-Oct-2002
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Timber Beam & Joist

Page 1
 f:\ec55\killam.porch.ecw:Calculations

Description roof rafters & beams

Timber Member Information Calculations are designed to 1997 NDS and 1997 UBC Requirements

	porch rafters	beam # 1	beam # 2
Timber Section	2x8	4x10	4x10
Beam Width	in 1.500	3.500	3.500
Beam Depth	in 7.250	9.250	9.250
Le: Unbraced Length	ft 2.00	2.00	2.00
Timber Grade	Douglas Fir - Larch, Douglas Fir - Larch, Douglas Fir - Larch,		
Fb - Basic Allow	psi 875.0	875.0	875.0
Fv - Basic Allow	psi 95.0	95.0	95.0
Elastic Modulus	ksi 1,600.0	1,600.0	1,600.0
Load Duration Factor	1.250	1.250	1.250
Member Type	Sawn	Sawn	Sawn
Repetitive Status	Repetitive	No	No

Center Span Data

		15.00	10.00	12.00
Span	ft			
Dead Load	#/ft	18.60	168.00	56.00
Live Load	#/ft	21.30	192.00	64.00

Results Ratio = 0.6960 0.8274 0.3972

Mmax @ Center	in-k	13.47	54.00	25.92
@ X =	ft	7.50	5.00	6.00
Fb : Actual	psi	1,024.8	1,081.9	519.3
Fb : Allowable	psi	1,472.3	1,307.6	1,307.6
		Bending OK	Bending OK	Bending OK
Fv : Actual	psi	38.0	70.7	29.1
Fv : Allowable	psi	118.8	118.8	118.8
		Shear OK	Shear OK	Shear OK

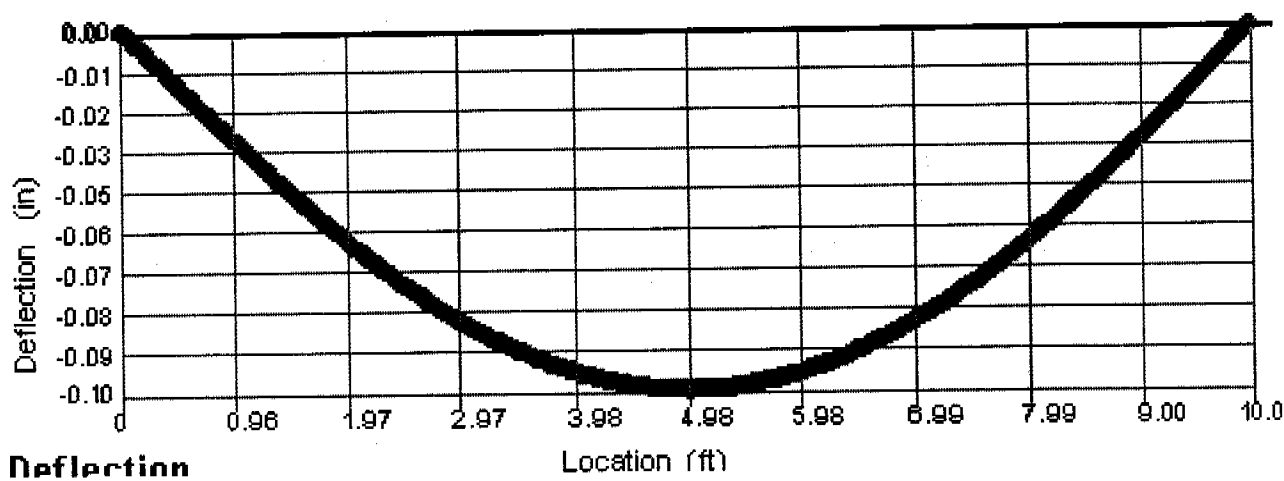
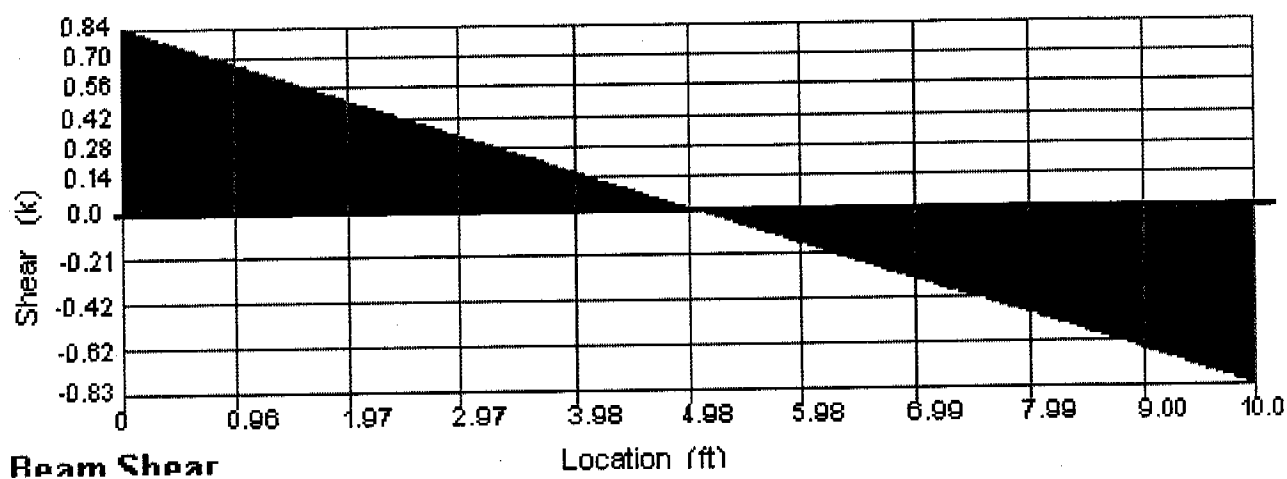
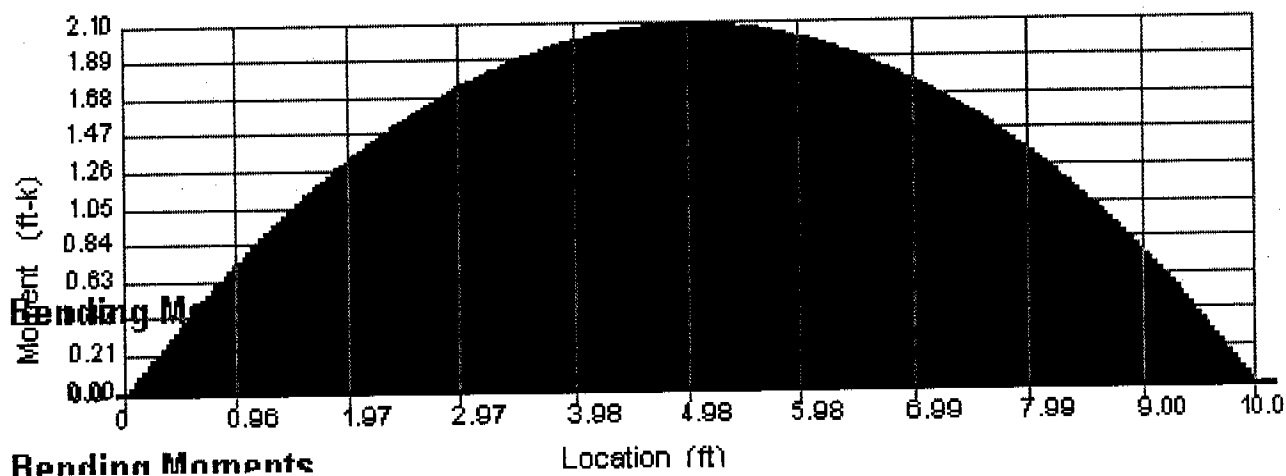
Reactions

@ Left End	DL	lbs	139.50	840.00	336.00
	LL	lbs	159.75	960.00	384.00
	Max. DL+LL	lbs	299.25	1,800.00	720.00
@ Right End	DL	lbs	139.50	840.00	336.00
	LL	lbs	159.75	960.00	384.00
	Max. DL+LL	lbs	299.25	1,800.00	720.00

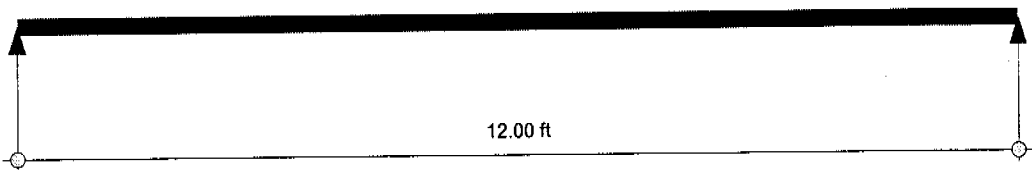
Deflections Ratio OK Deflection OK Deflection OK

Center DL Defl	in	-0.278	-0.102	-0.071
L/Defl Ratio		647.5	1,172.6	2,035.7
Center LL Defl	in	-0.318	-0.117	-0.081
L/Defl Ratio		565.5	1,026.0	1,781.2
Center Total Defl	in	-0.596	-0.219	-0.152
Location	ft	7.500	5.000	6.000
L/Defl Ratio		301.9	547.2	950.0





BEAM # 2



Mmax = 25.91 in-k at 5.99 ft from left
Dmax = -0.1515 in at 5.99 ft from left

Rl:max = 719.999 lbs

Rr:max = 719.999 lbs

Title :
 Dsgnr:
 Description :

Job #
 Date: 2:53PM, 22 APR 04

//

Scope :

Timber Beam & Joist

Description roof rafters & beams

Timber Member Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Timber Section		porch rafters 2x8	beam # 1 4x10	beam # 2 4x10
Beam Width	in	1.500	3.500	3.500
Beam Depth	in	7.250	9.250	9.250
Le: Unbraced Length	ft	2.00	2.00	2.00
Timber Grade		Douglas Fir - Larch, Douglas Fir - Larch, Douglas Fir - Larch,		
Fb - Basic Allow	psi	875.0	875.0	875.0
Fv - Basic Allow	psi	95.0	95.0	95.0
Elastic Modulus	ksi	1,600.0	1,600.0	1,600.0
Load Duration Factor		1.250	1.250	1.250
Member Type		Sawn	Sawn	Sawn
Repetitive Status		Repetitive	No	No

Center Span Data

	ft	15.00	10.00	12.00
Span	ft	15.00	10.00	12.00
Dead Load	#/ft	18.60	168.00	56.00
Live Load	#/ft	21.30	192.00	64.00

Results

Ratio =		0.6960	0.8274	0.3972
Mmax @ Center	in-k	13.47	54.00	25.92
@ X =	ft	7.50	5.00	6.00
fb : Actual	psi	1,024.8	1,081.9	519.3
fb : Allowable	psi	1,472.3	1,307.6	1,307.6
		Bending OK	Bending OK	Bending OK
fv : Actual	psi	38.0	70.7	29.1
Fv : Allowable	psi	118.8	118.8	118.8
		Shear OK	Shear OK	Shear OK

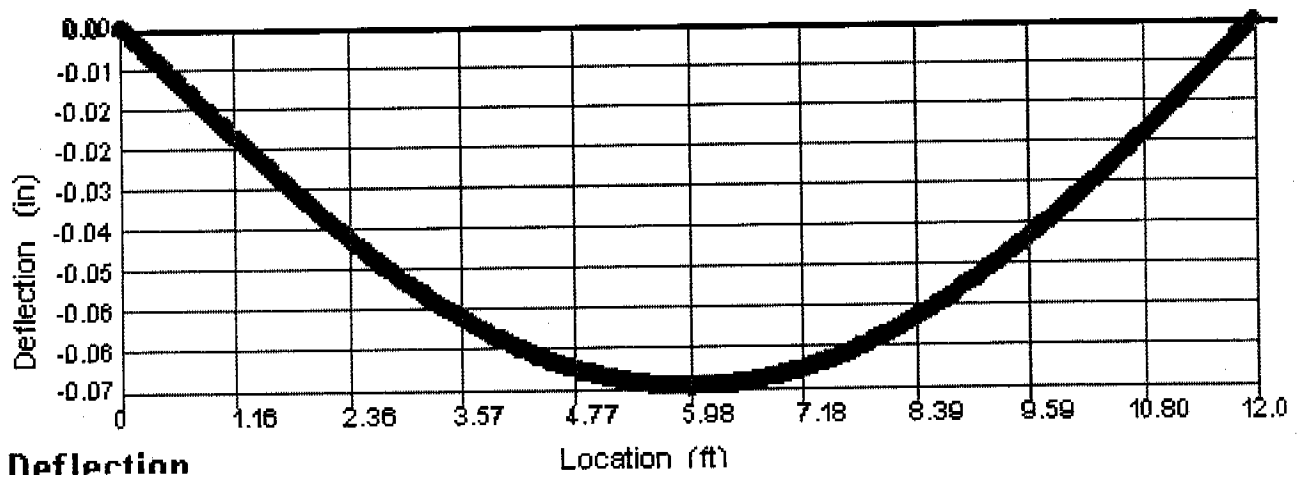
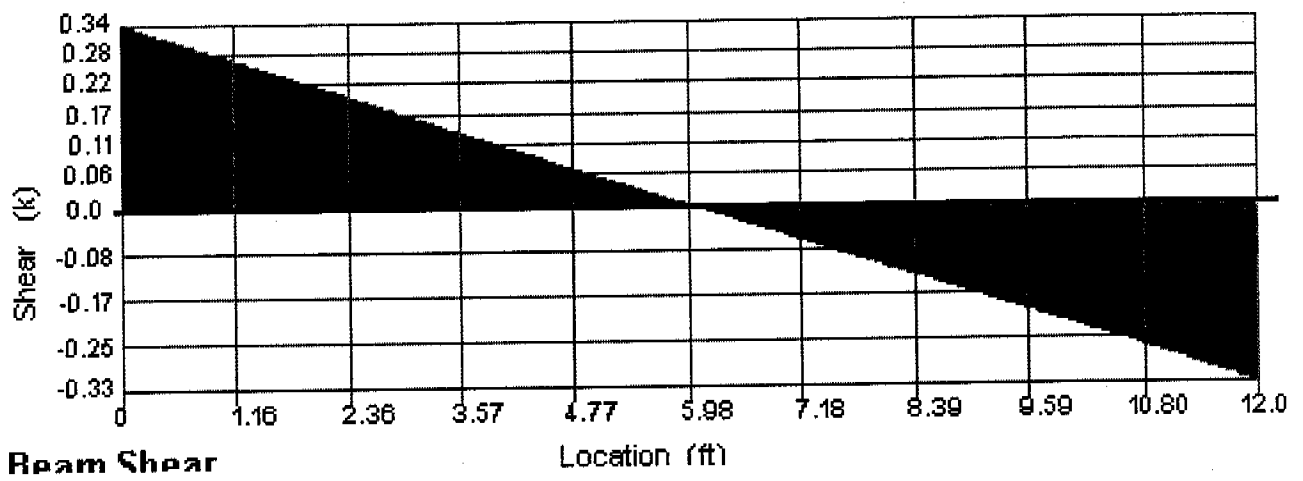
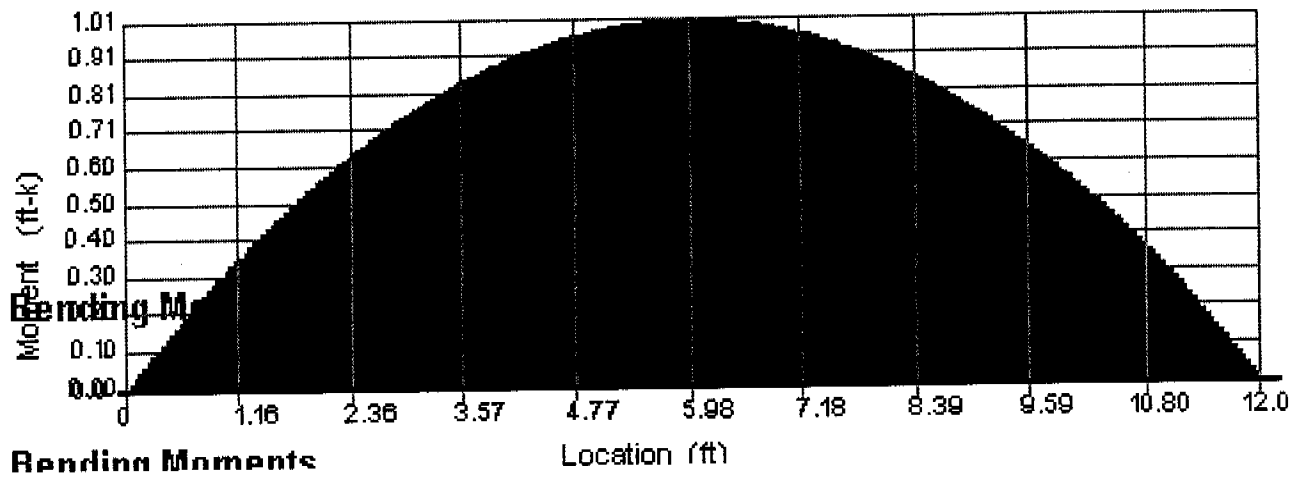
Reactions

@ Left End	DL	lbs	139.50	840.00	336.00
	LL	lbs	159.75	960.00	384.00
	Max. DL+LL	lbs	299.25	1,800.00	720.00
@ Right End	DL	lbs	139.50	840.00	336.00
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	Max. DL+LL	lbs	299.25	1,800.00	720.00

Deflections

		Ratio OK	Deflection OK	Deflection OK
Center DL Defl	in	-0.278	-0.102	-0.071
L/Defl Ratio		647.5	1,172.6	2,035.7
Center LL Defl	in	-0.318	-0.117	-0.081
L/Defl Ratio		565.5	1,026.0	1,781.2
Center Total Defl	in	-0.596	-0.219	-0.152
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Title :
 Dsgnr:
 Description :

Job #
 Date: 2:54PM, 22 APR 04

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Scope :

Rev: 560100
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Timber Column Design

Page 1
 f:\ec55\willam.porch.ecw:Calculations

Description post design

General Information Calculations are designed to 1997 NDS and 1997 UBC Requirements

Wood Section	4x4	Total Column Height	9.00 ft	Le XX for Axial	9.00 ft
Rectangular Column		Load Duration Factor	1.00	Le YY for Axial	9.00 ft
Column Depth	3.50 in	Fc	1,500.00 psi	Lu XX for Bending	9.00 ft
Width	3.50 in	Fb	1,000.00 psi		
Sawn		E - Elastic Modulus	1,700 ksi		
		Douglas Fir - Larch, No.1			

Loads

	Dead Load	Live Load	Short Term Load
Axial Load	1,050.00 lbs	1,200.00 lbs	156.00 lbs
Eccentricity	0.000 in		

Summary Column OK

Using : 4x4, Width= 3.50in, Depth= 3.50in, Total Column Ht= 9.00ft

	DL + LL	DL + LL + ST	DL + ST
fc : Compression	183.67 psi	196.41 psi	98.45 psi
Fc : Allowable	495.65 psi	495.65 psi	495.65 psi
fbx : Flexural	0.00 psi	0.00 psi	0.00 psi
F'bx : Allowable	1,500.00 psi	1,500.00 psi	1,500.00 psi
Interaction Value	0.3706	0.3963	0.1986

Stress Details

Fc : X-X	495.65 psi	For Bending Stress Calcs...	
Fc : Y-Y	495.65 psi	Max k*Lu / d	50.00
F'c : Allowable	495.65 psi	Actual k*Lu/d	22.59
F'c:Allow * Load Dur Factor	495.65 psi	Min. Allow k*Lu / d	11.00
F'bx	1,500.00 psi	Cf:Bending	1.500
F'bx * Load Duration Factor	1,500.00 psi	Rb : (Le d / b^2) ^.5	7.535
		For Axial Stress Calcs...	
		Cf : Axial	1.150
		Axial X-X k Lu / d	30.86
		Axial Y-Y k Lu / d	30.86

Notes

Calculations are designed to 1997 NDS and 1997 UBC Guidelines
 Section databases have been updated as of 2-Apr-1999
 Allowable stress databases have been updated to 1997 NDS & 1997 UBS values on 2-Apr-1999
 To determine Cf values for sawn sections, the program looks for the identifying words in the "Stress" entry.
 "Select", "No.1", "Standard" and similar typical words are used to determine Cf category
 "Unbraced length" is multiplied by the following values to calculate "Le"
 When beam depth <= 7", Le = 2.06 * Lu
 When 7" < beam depth <= 14.3", Le = 1.62 * Lu + 3d
 When beam depth > 14.3", Le = 1.84 * Lu

General Footing Analysis & Design

Description foundation design

General Information Calculations are designed to ACI 318-95 and 1997 UBC Requirements

<table border="0" style="width: 100%;"> <tr> <td>Allowable Soil Bearing</td> <td style="text-align: right;">1,000.0 psf</td> </tr> <tr> <td>Short Term Increase</td> <td style="text-align: right;">1.330</td> </tr> <tr> <td>Seismic Zone</td> <td style="text-align: right;">3</td> </tr> <tr> <td colspan="2">Biaxial Applied Loads</td> </tr> <tr> <td colspan="2">Live & Short Term Combined</td> </tr> <tr> <td> fc</td> <td style="text-align: right;"> 2,000.0 psi</td> </tr> <tr> <td>Fy</td> <td style="text-align: right;">40,000.0 psi</td> </tr> <tr> <td>Concrete Weight</td> <td style="text-align: right;">145.00 pcf</td> </tr> <tr> <td>Overburden Weight</td> <td style="text-align: right;">0.00 psf</td> </tr> </table>	Allowable Soil Bearing	1,000.0 psf	Short Term Increase	1.330	Seismic Zone	3	Biaxial Applied Loads		Live & Short Term Combined		 fc	 2,000.0 psi	Fy	40,000.0 psi	Concrete Weight	145.00 pcf	Overburden Weight	0.00 psf	<table border="0" style="width: 100%;"> <tr> <td colspan="2">Dimensions...</td> </tr> <tr> <td>Width along X-X Axis</td> <td style="text-align: right;">2.000 ft</td> </tr> <tr> <td>Length along Y-Y Axis</td> <td style="text-align: right;">2.000 ft</td> </tr> <tr> <td>Footing Thickness</td> <td style="text-align: right;">12.00 in</td> </tr> <tr> <td>Col Dim. Along X-X Axis</td> <td style="text-align: right;">3.50 in</td> </tr> <tr> <td>Col Dim. Along Y-Y Axis</td> <td style="text-align: right;">3.50 in</td> </tr> <tr> <td>Base Pedestal Height</td> <td style="text-align: right;">0.000 in</td> </tr> <tr> <td> Min Steel %</td> <td style="text-align: right;"> 0.0014</td> </tr> <tr> <td>Rebar Center To Edge Distance</td> <td style="text-align: right;">3.50 in</td> </tr> </table>	Dimensions...		Width along X-X Axis	2.000 ft	Length along Y-Y Axis	2.000 ft	Footing Thickness	12.00 in	Col Dim. Along X-X Axis	3.50 in	Col Dim. Along Y-Y Axis	3.50 in	Base Pedestal Height	0.000 in	 Min Steel %	 0.0014	Rebar Center To Edge Distance	3.50 in
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Rebar Center To Edge Distance	3.50 in																																				

Loads

Applied Vertical Load...			
Dead Load	1.050 k	...ecc along X-X Axis	0.000 in
Live Load	1.200 k	...ecc along Y-Y Axis	0.000 in
Short Term Load	0.156 k		
<u>Creates Rotation about Y-Y Axis</u> (pressures @ left & right)		<u>Creates Rotation about X-X Axis</u> (pressures @ top & bot)	
Applied Moments...			
Dead Load	k-ft		k-ft
Live Load	k-ft		k-ft
Short Term	k-ft		k-ft
<u>Creates Rotation about Y-Y Axis</u> (pressures @ left & right)		<u>Creates Rotation about X-X Axis</u> (pressures @ top & bot)	
Applied Shears...			
Dead Load	k		k
Live Load	k		k
Short Term	k		k

Summary

Footing Design OK

2.00ft x 2.00ft Footing, 12.0in Thick, w/ Column Support 3.50 x 3.50in x 0.0in high			
2.00ft x 2.00ft Footing, 12.0in Thick, w/ Column Suppo			
	<u>DL+LL</u>	<u>DL+LL+ST</u>	
Max Soil Pressure	707.5	746.5 psf	Max Mu
Allowable	1,000.0	1,330.0 psf	Required Steel Area
"X" Ecc, of Resultant	0.000 in	0.000 in	0.320 k-ft per ft
"Y" Ecc, of Resultant	0.000 in	0.000 in	0.143 in ² per ft
X-X Min. Stability Ratio	No Overturning	1.500 :1	Shear Stresses....
Y-Y Min. Stability Ratio	No Overturning		<u>Vu</u>
			1.223
			6.192
			76.026 psi
			152.053 psi

Footing Design

Shear Forces	<u>ACI 9-1</u>	<u>ACI 9-2</u>	<u>ACI 9-3</u>	<u>Vn * Phi</u>	
Two-Way Shear	4.30 psi	6.19 psi	2.14 psi	152.05 psi	
One-Way Shears...					
Vu @ Left	1.22 psi	1.17 psi	0.40 psi	76.03 psi	
Vu @ Right	1.22 psi	1.17 psi	0.40 psi	76.03 psi	
Vu @ Top	1.22 psi	1.17 psi	0.40 psi	76.03 psi	
Vu @ Bottom	1.22 psi	1.17 psi	0.40 psi	76.03 psi	
Moments					
	<u>ACI 9-1</u>	<u>ACI 9-2</u>	<u>ACI 9-3</u>	<u>Ru / Phi</u>	<u>As Req'd</u>
Mu @ Left	0.32 k-ft	0.31 k-ft	0.11 k-ft	4.9 psi	0.14 in ² per ft
Mu @ Right	0.32 k-ft	0.31 k-ft	0.11 k-ft	4.9 psi	0.14 in ² per ft
Mu @ Top	0.32 k-ft	0.31 k-ft	0.11 k-ft	4.9 psi	0.14 in ² per ft
Mu @ Bottom	0.32 k-ft	0.31 k-ft	0.11 k-ft	4.9 psi	0.14 in ² per ft

Title :
Dsgnr:
Description :

Job #
Date: 2:54PM, 22 APR 04

15

Scope :

Rev: 560100
User: KW-0805313, Ver 5.6.1, 25-Oct-2002
(c)1983-2002 ENERCALC Engineering Software

General Footing Analysis & Design

Page 2

f:\ec55\killam.porch.ecw\Calculations

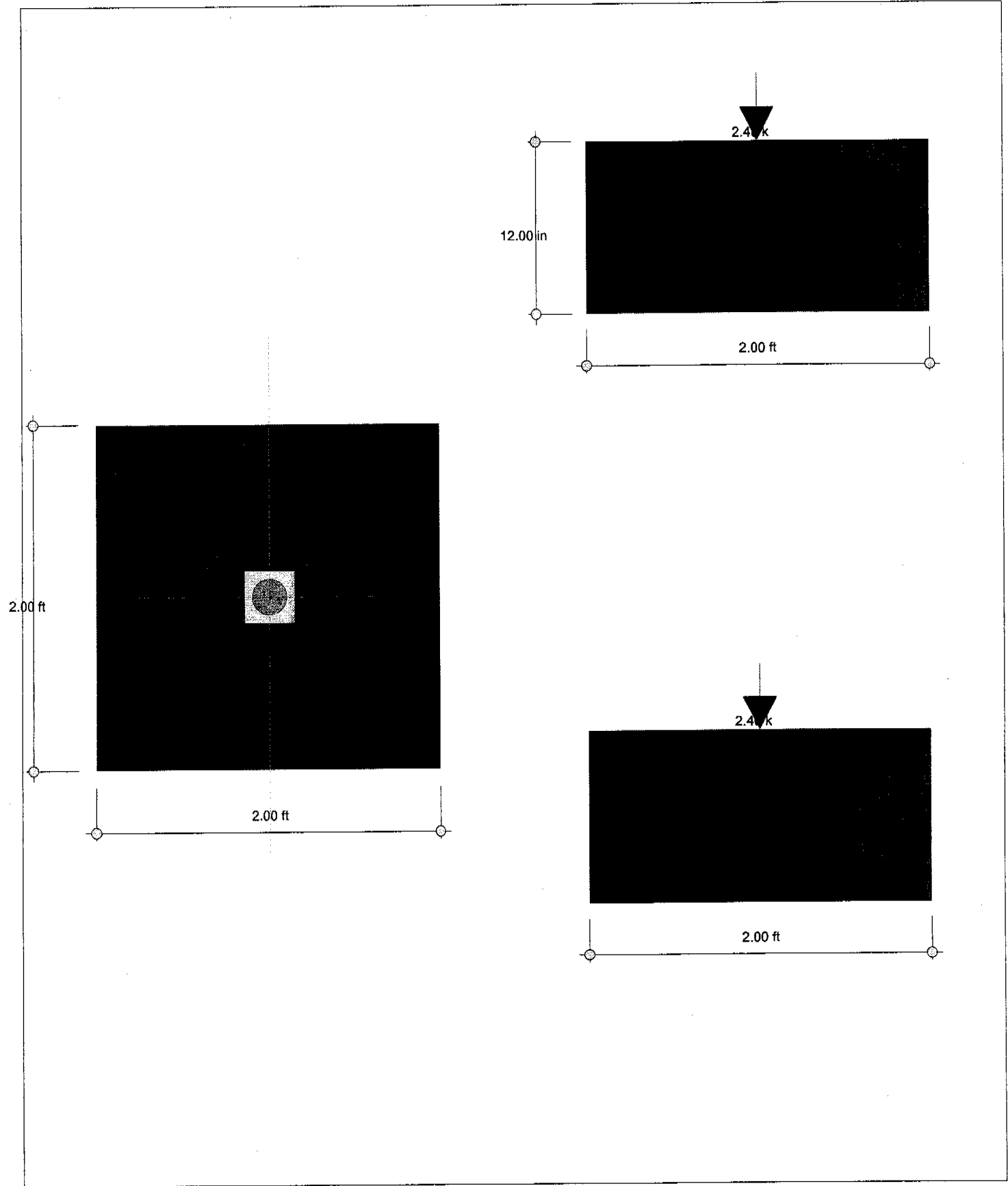
Description foundation design

Soil Pressure Summary

Service Load Soil Pressures	Bottom-	Top-Rig	Bottom-	Top-Lef
DL + LL	707.50	707.50	707.50	707.50 psf
DL + LL + ST	746.50	746.50	746.50	746.50 psf
Factored Load Soil Pressures				
ACI Eq. 9-1	1,080.50	1,080.50	1,080.50	1,080.50 psf
ACI Eq. 9-2	1,045.10	1,045.10	1,045.10	1,045.10 psf
ACI Eq. 9-3	421.35	421.35	421.35	421.35 psf

ACI Factors (per ACI, applied internally to entered loads)

ACI 9-1 & 9-2 DL	1.400	ACI 9-2 Group Factor	0.750	UBC 1921.2.7 "1.4" Factor	1.400
ACI 9-1 & 9-2 LL	1.700	ACI 9-3 Dead Load Factor	0.900	UBC 1921.2.7 "0.9" Factor	0.900
ACI 9-1 & 9-2 ST	1.700	ACI 9-3 Short Term Factor	1.300		
....seismic = ST * :	1.100				



SAL KADDORAH, P.E.

211 yacht club way # 339
redondo beach, ca 90277
(310)937-8725

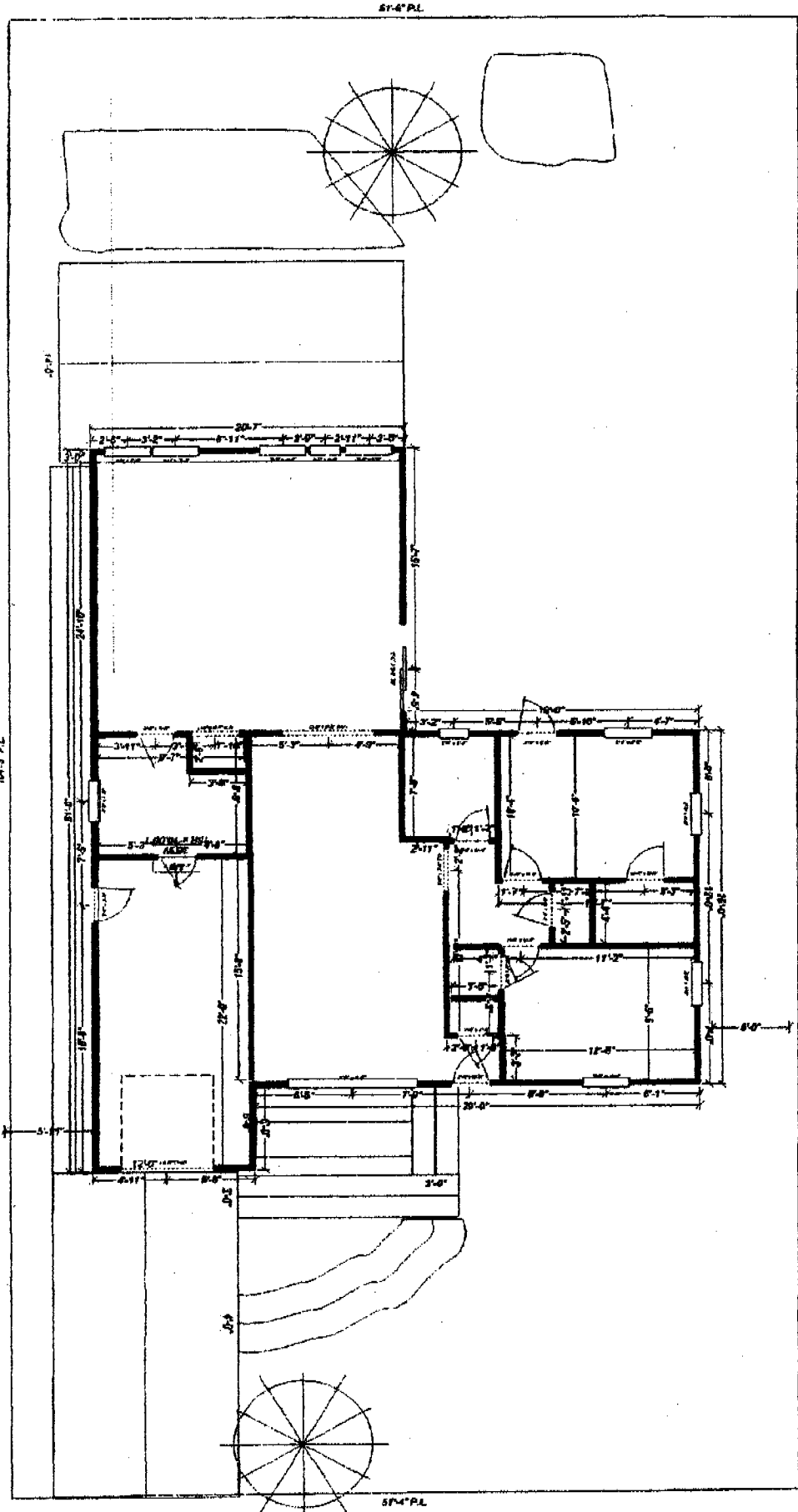
FOUNDATION DESIGN

USING MAXIMUM BEARING CAPACITY OF 1000 PSF WITH
APPLICABLE INCREASES.

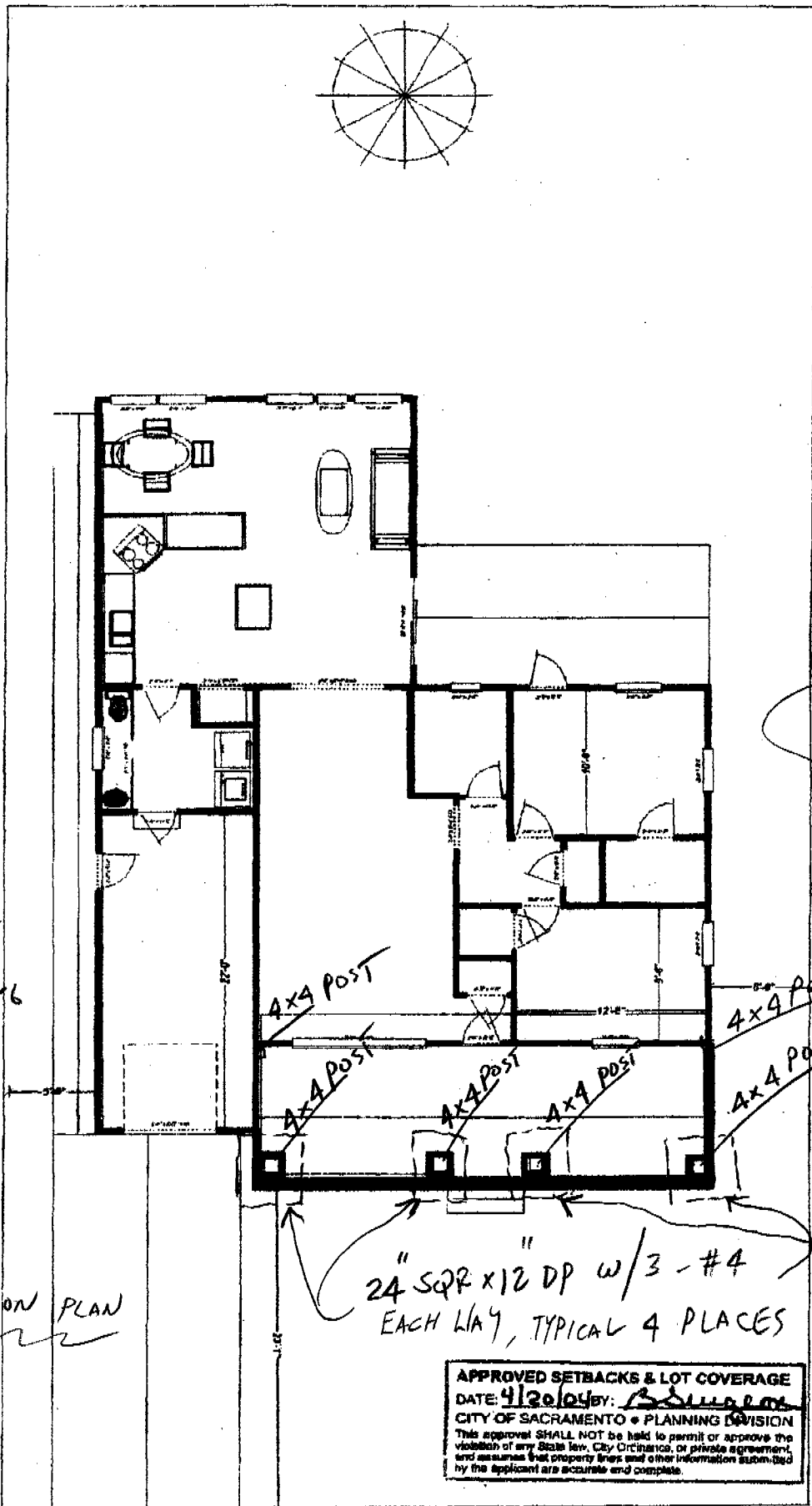
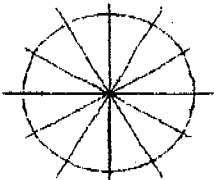
LOAD	REQUIRED FOUNDATION
2828 #	12" WIDE STRIP FTG X 12" DP
3535 #	15" WIDE STRIP FTG X 12" DP
4000 #	24" SQR. CONC. PAD
6250 #	30" SQR CONC. PAD
9000 #	36" SQR CONC. PAD
1225 #	42" SQR CONC. PAD
16000 #	48" SQR CONC. PAD

12" DEEP FOR 1 STORY
18" DEEP FOR 2 STORY

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Existing



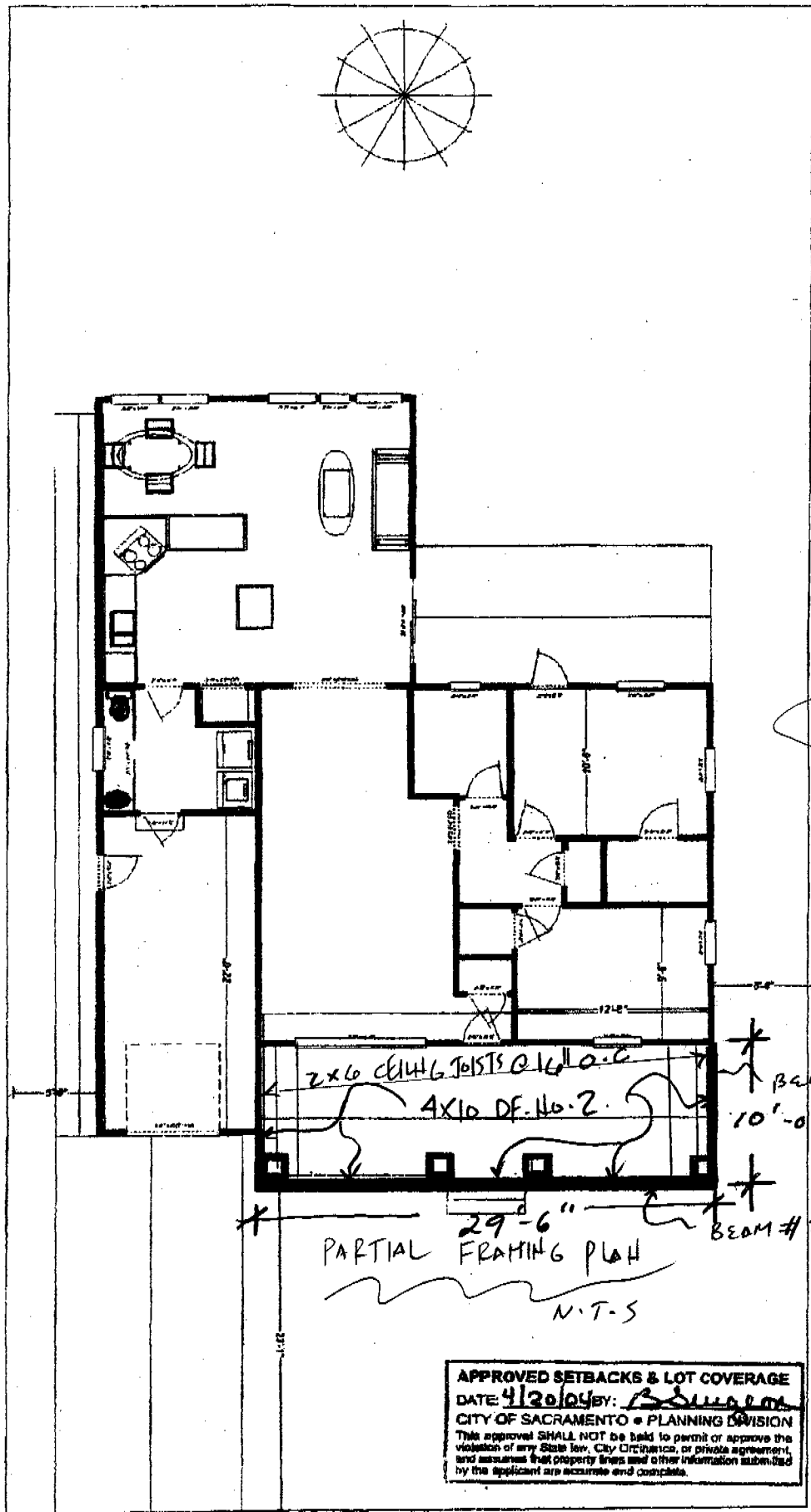
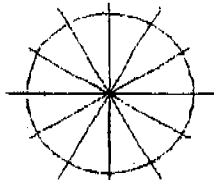
proposed

USE SIMPSON
'PB' POST BASE
TO ATTACH POST
TO NEW FOOTING
TYPICAL.

PARTIAL FOUNDATION PLAN
N.T.S.

24" SQR x 12" DP w/3 - #4
EACH LAY, TYPICAL 4 PLACES

APPROVED SETBACKS & LOT COVERAGE
DATE: 4/20/04 BY: [Signature]
CITY OF SACRAMENTO - PLANNING DIVISION
This approval SHALL NOT be held to permit or approve the
violation of any State law, City Ordinance, or private agreement,
and assumes that property lines and other information submitted
by the applicant are accurate and complete.



proposed

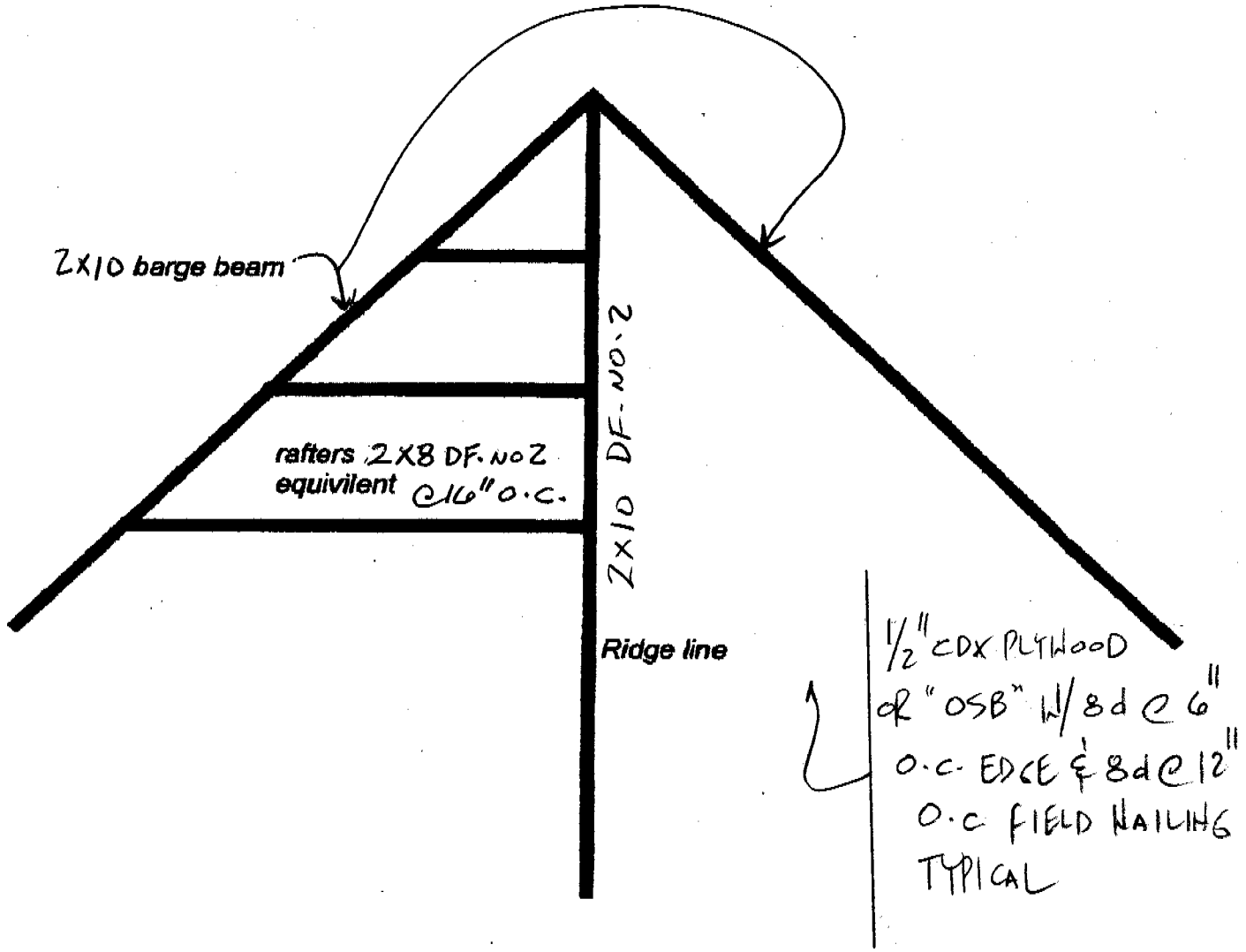
USE SIMPSON
PC POST CAP
TO ATTACH
BEAMS TO POSTS
TYPICAL

2x6 CEILING JOISTS @ 16" O.C.
4x10 DF. No. 2
10'-0"
BEAM #1
29'-6"
BEAM #2
PARTIAL FRAMING PLAN
N.T.S

APPROVED SETBACKS & LOT COVERAGE
DATE: 4/20/04 BY: [Signature]
CITY OF SACRAMENTO - PLANNING DIVISION
This approval SHALL NOT be held to permit or approve the violation of any State law, City Ordinance, or private agreement, and assumes that property lines and other information submitted by the applicant are accurate and complete.

Jerry Way 1831

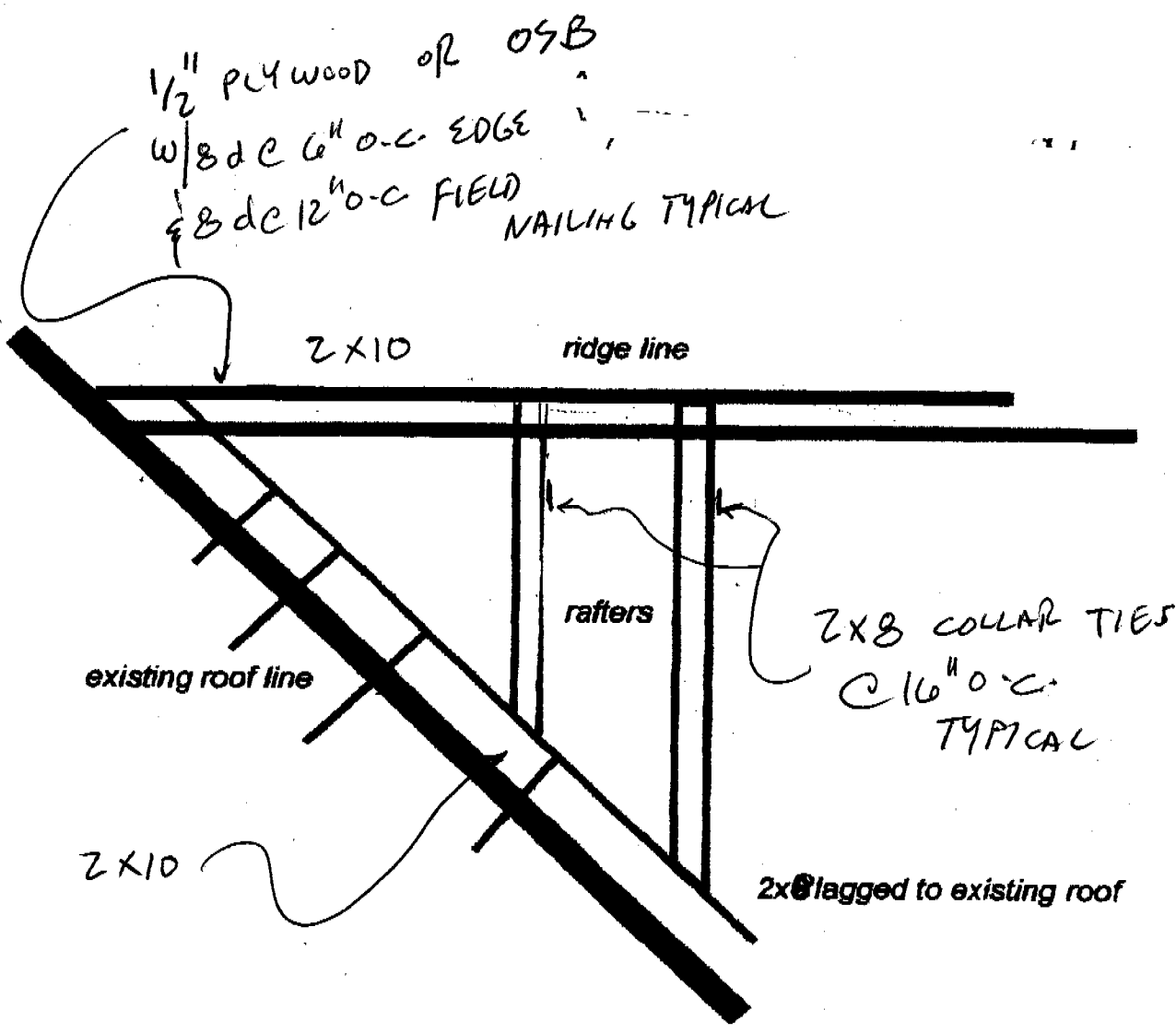
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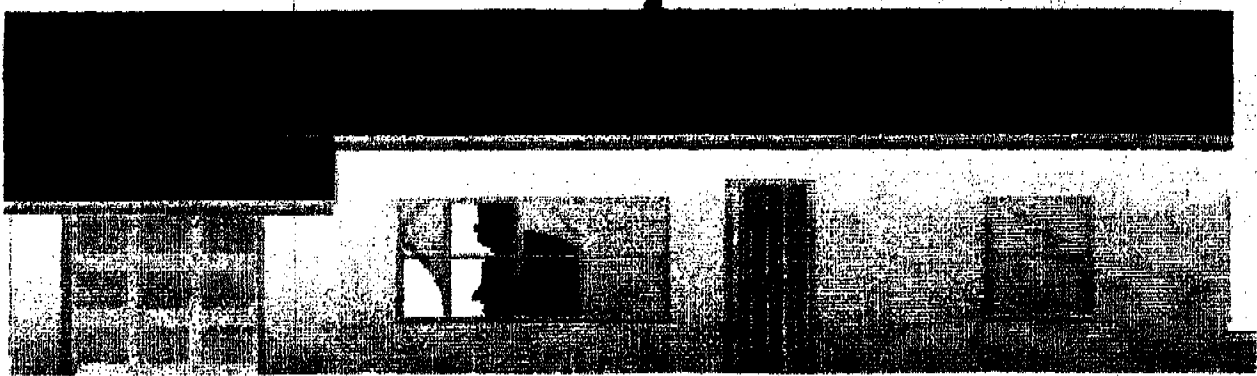


PARTIAL FRAMING OVER EXTENDED PORCH

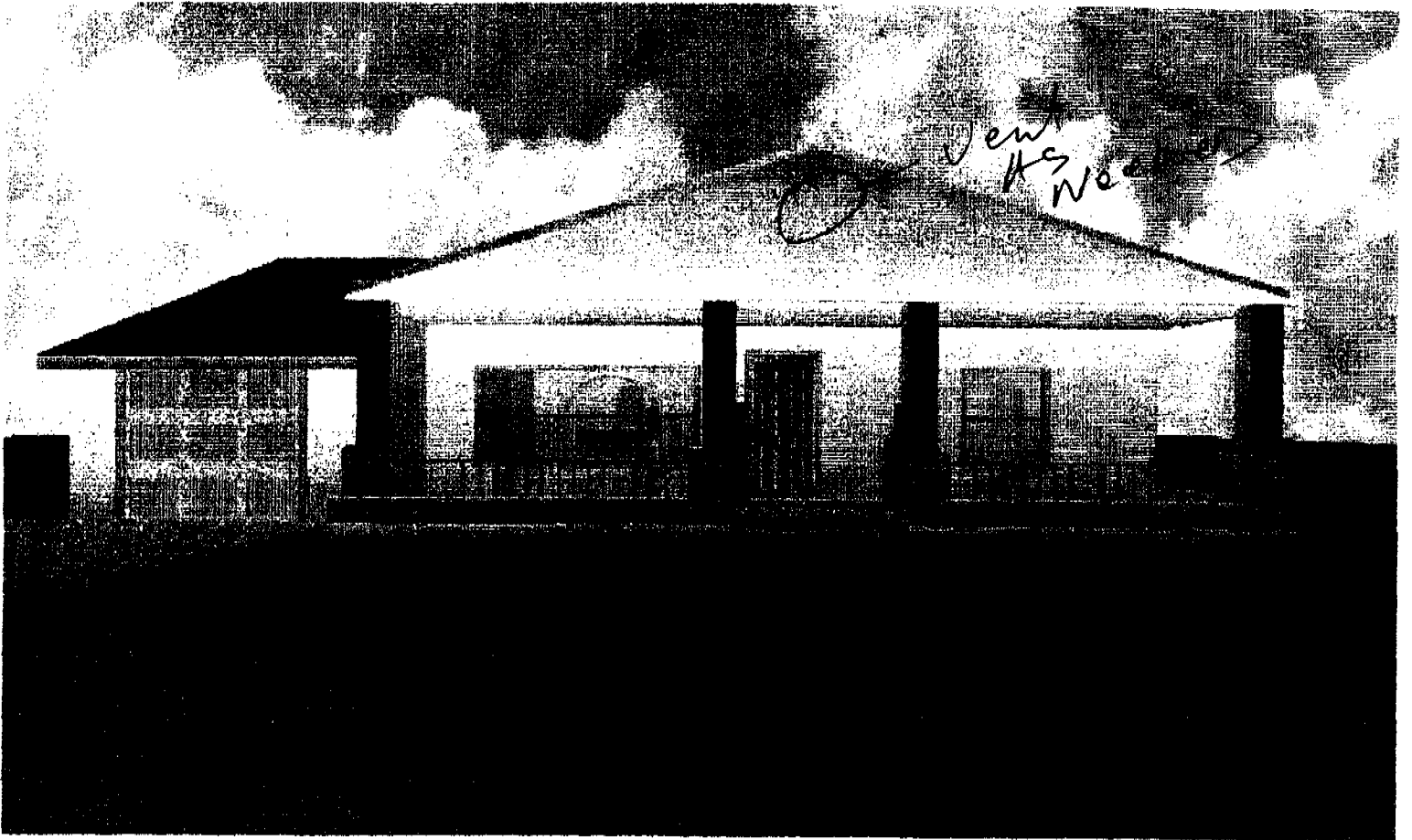
N.T.S

22





existing



proposed