

**CITY OF SACRAMENTO**  
1231 I Street, Sacramento, CA 95814

**Permit No: 0107009**  
**Insp Area: 4**

**Site Address: 2980 SEMINOLE WY SAC**  
Parcel No: 262-0300-048

Sub-Type: RES  
Housing (Y/N): N

CONTRACTOR

OWNER

ARCHITECT

PIREZ LIBRADO I. & JOSEFINA  
2980 SEMINOLE WY  
SACRAMENTO CA 95833

**Nature of Work: REROOF T/O 23 SQ INSTALL LT WT TILE**

**CONSTRUCTION LENDING AGENCY:** I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C.)

Lender's Name \_\_\_\_\_ Lender's Address \_\_\_\_\_

**LICENSED CONTRACTORS DECLARATION:** I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class \_\_\_\_\_ License Number \_\_\_\_\_ Date \_\_\_\_\_ Contractor Signature \_\_\_\_\_

**OWNER-BUILDER DECLARATION:** I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code, any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00).

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code. The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

\_\_\_\_ I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code. The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law)

\_\_\_\_ I am exempt under Sec \_\_\_\_\_ B & PC for this reason: \_\_\_\_\_

Date 6-4-01  Owner Signature Librado Pirez

**IN ISSUING THIS BUILDING PERMIT,** the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 6-4-01  Applicant/Agent Signature Librado Pirez

**WORKER'S COMPENSATION DECLARATION:** I hereby affirm under penalty of perjury one of the following declarations:

\_\_\_\_ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued

\_\_\_\_ I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier \_\_\_\_\_ Policy Number \_\_\_\_\_ Exp Date \_\_\_\_\_

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to be subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 6-4-01  Applicant Signature Librado Pirez

**WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.**

**THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.**

OWNER-BUILDER VERIFICATION

ATTENTION PROPERTY OWNERS

An owner-builder building permit has been applied for in your name and bearing your signature.

Please complete and return this information in the envelope provided at your earliest opportunity to avoid unnecessary delay in processing and issuing your building permit. No building permit will be issued until this verification is received.

1. I personally plan to provide the major labor and materials for construction of the proposed Improvement (yes) or no) \_\_\_\_\_
2. I (have) have not) \_\_\_\_\_ signed an application for A building permit for the proposed work.

3. I have contracted with the following person (firm) to provide the proposed construction:

Name \_\_\_\_\_ Address \_\_\_\_\_

City \_\_\_\_\_ Telephone \_\_\_\_\_

Contractors License No \_\_\_\_\_

4. I plan to provide portions of the work, but I have hired the following person to coordinate, Supervise, and provide the major work.

Name \_\_\_\_\_ Address \_\_\_\_\_

City \_\_\_\_\_ Telephone \_\_\_\_\_

Contractors License No \_\_\_\_\_

5. I will provide some of the work but I have contracted (hired) the following to provide the Work indicated:

Name	Address	Phone	Type of work
------	---------	-------	--------------

_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____

Signed Librado King

Job Address 2980 SEMINOLE WY

Permit No: C 107009

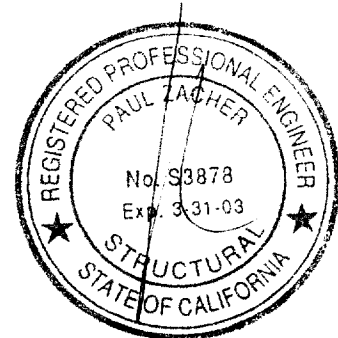
Perez

Paul Zacher - Structural Engineers  
4701 Lakeside Way  
Fair Oaks, CA 95628

TEL: 916.961.3960  
FAX: 916.961.6552

May 31, 2001

Perez  
2980 Seminole Way  
Sacramento, CA 95833  
TEL: (916) 925-2128



Attn: Mr. Perez.

re: Job 2001\_009 PEREZ

Subject: Structural Investigation Report of the Roof for the Residence located at 2980 Seminole Way, Sacramento, CA 95833

As requested by Mr. Perez, this is a report to determine what needs should be addressed to correct any structural deficiencies of the roof. Paul Zacher visited the site May 31, 2001. The investigation was made to determine the existing condition of the structure. All information, data and analysis contained within this report are based on the 1997 Uniform Building Code.

The following is based on visual observations with no subsurface investigation being made.

**DESCRIPTION:**

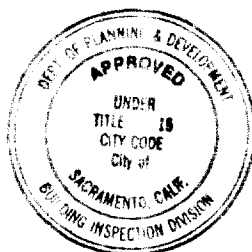
Type of Facility	Residence
Year Built:	Estimated 1970's vintage
Occupancy	Residential
No. of Stories	Two
Dimensions	Approximately 2500 square feet with a first story plate height of 8 feet.

**CONSTRUCTION:**

Roof:  
The roof covering will consist of a Light Weight Concrete Tile over 1/2" solid sheathing. The living area is framed with pre-engineered wood trusses spaced at 24" on center.

**CONCLUSIONS:**

Roof:  
The living area has sufficient structural capacity for the applied live and dead loads.



**This set of plans and specifications must be kept on the job at all times and it is unlawful to make any changes or alterations from the same without written permission from the Building Inspection Division.**  
The approval of this plan and specification SHALL NOT be held to permit or approve the violation of any City Ordinance or State Law.

**ISSUED**

JUN 01 2001

Sacramento Building Division

Percz.

Paul Zacher - Structural Engineers  
4701 Lakeside Way  
Fair Oaks, CA 95628

TEL: 916.961.3960  
FAX: 916.961.6552

**RECOMMENDATIONS:**

None.

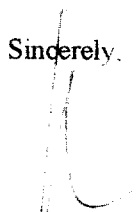
It shall be noted that small hairline cracking may occur at exterior stucco and interior gypboard finished walls that are load bearing or distributing roof strut loads. These cracks are a natural occurrence as the existing structure re-distributes the new roof weight. They are cosmetic in nature and are not an indication of a structural hazard or failure

It shall be noted that some deflection of the rafters may be evident after installation of the tile. The existing roof framing has deflected but this may not be readily evident due to the uneven nature of the existing roofing material. Concrete tile is a very consistent and uniform product and when installed in an even plane, even small deflections can become apparent. This is only a cosmetic issue and not a structural concern

The inspection consisted of visual observation only, made solely to determine the structural capacity of the existing roof. Analysis does not determine any effects on the overall structure under lateral forces or effects on the foundation unless specifically noted in the calculations and in this document. No warranties, expressed or implied, are made or intended in conjunction with this report. The inspection was made only to the portions that were accessible. The specific items noted were those that were observable and there may be defects that are not observable, or are hidden by architectural and structural materials.

If you have any questions on the above, do not hesitate to call.

Sincerely,

  
Paul Zacher, P.E., S.E.  
file

**DESIGN LOADING:**

Roof Pitch	4	in 12
Pitch Adjustment Factor	1.05	

**LOCATION: TOP CHORD**

<u>MATERIAL</u>	<u>WEIGHT</u>	
Light Weight Tile	7.00	psf
Roofing felt	0.30	psf
1/2" OSB/ plywood	1.50	psf
1x4 skip sht'g	1.09	psf
2x4 truss @ 24" oc	0.64	psf
	Load	10.5 psf
	Roof Pitch Adjustment	0.57 psf
	Total Load	11.1 psf

**LOCATION: BOTTOM CHORD**

<u>MATERIAL</u>	<u>WEIGHT</u>	
Batt/blown insul	0.50	psf
2x4 truss @ 24" oc	1.28	psf
1/2" Gypboard	2.50	psf
	Load	4.3 psf

Paul Zacher - Structural Engineers  
 4701 Lakeside Way  
 Fair Oaks  
 TEL: (916) 961-3960  
 FAX: (916) 961-6552

Title :  
 Dsgnr:  
 Description :  
 Scope :

Job #  
 Date: 8:31AM, 31 MAY 01

Rev: 510304  
 User: RW\_0602844\_Ver 5.1.5.00 Jun 1999 Wa:22  
 (c) 1983-99 ENERCALC

### Timber Beam & Joist

c:\enercalc\test.ecw\Calculations

#### Description BEAMS

#### Timber Member Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

		B1
Timber Section		4x14
Beam Width	in.	3.500
Beam Depth	in.	13.250
Le: Unbraced Length	ft.	2.00
Timber Grade		Douglas Fir - Larch
Fb - Basic Allow	psi	875.0
Fv - Basic Allow	psi	95.0
Elastic Modulus	ksi	1,600.0
Load Duration Factor		1.250
Member Type		Sawn
Repetitive Status		No

#### Center Span Data

Span	ft.	16.00
Dead Load	#/ft.	182.00
Live Load	#/ft.	64.00

#### Results Ratio = 0.8472

Mmax @ Center	in-k	94.46
@ X =	ft.	8.00
fb: Actual	psi	922.4
Fb: Allowable	psi	1,088.8
		Bending OK
fv: Actual	psi	55.0
Fv: Allowable	psi	118.8
		Shear OK

#### Reactions

@ Left End	DL	lbs	1,456.00
	LL	lbs	512.00
	Max. DL+LL	lbs	1,968.00
@ Right End	DL	lbs	1,456.00
	LL	lbs	512.00
	Max. DL+LL	lbs	1,968.00

#### Deflections

		Ratio On
Center DL Defl	in.	-0.247
L/Defl Ratio		776.7
Center LL Defl	in.	-0.087
L/Defl Ratio		2,208.6
Center Total Defl	in.	-0.334
Location	ft.	8.000
L/Defl Ratio		574.6

□ K Zacher S E

4701 Lakeside Way  
Fair Oaks, CA 95628  
TEL: (916) 961-3960  
FAX: (916) 961-6552

Job #:           

Date:           

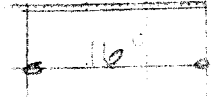
          

182/04

81

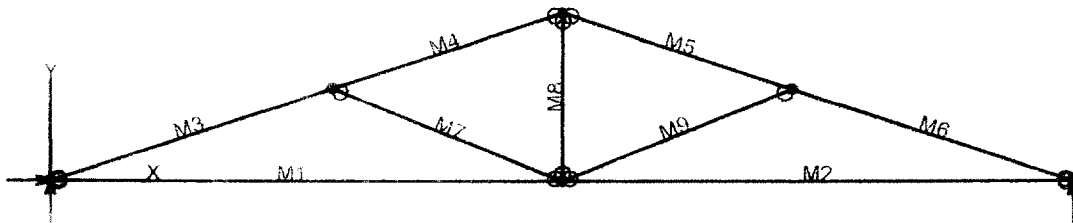
15.4 4° 11.4

9x14<sup>2</sup>



16.5 3.64

15.0 8° 11.0





# VisualAnalysis 3.50.c Report

07/31/01 08:19:25

Project: Truss 1

File: C:\Program Files\IES\VA35\truss 1.vap

Company: PK Associates Engineers

Engineer: Paul Zacher

Default Units: Feet, Pounds, Degrees, °Fahrenheit, Seconds.

## Nodes

Node	X ft	Y ft	Fix	DX	DY	RZ
N1	0.00	0.00	Yes	Yes	No	No
N2	10.00	0.00	No	No	No	No
N3	20.00	0.00	"	Yes	"	"
N4	5.50	1.83	"	No	"	"
N5	14.50	1.83	"	"	"	"
N6	10.00	3.33	"	"	"	"

## Member Elements

Member	Section	Material	Length ft
M1	SS2x4	Wood	10.00
M2	"	"	10.00
M3	"	"	5.83
M4	"	"	4.14
M5	"	"	4.14
M6	"	"	5.80
M7	"	"	4.86
M8	"	"	3.33
M9	"	"	4.86

## Section Properties

Category	Section	Ax in <sup>2</sup>	Iz in <sup>4</sup>	Sy+ in <sup>3</sup>	Sy- in <sup>3</sup>
Wood Sha	SS2x4	5.25	5.36	3.06	3.06

## Material Properties

Material	Strength psi	Elasticity psi	Poisson	Density lb/ft <sup>3</sup>
Wood	-NA-	1700000.00	0.34	40.47

## Load Combination Summary

Equation Case: Equation Case 1

Combination: +1D+1L+1Lr

Contributing Cases & Source

Service Case 1 (Dead loads)

Service Case 2 (Roof Live loads)

## Member Uniform Loads

This item is empty. Check the selection state, or report properties.

## Nodal Reactions

Node	Load Case	FX lbs	FY lbs	MZ lb-ft
N1	Equation Case 1	0.00	657.26	-NA-
N3	"	-NA-	657.26	-NA-

## Member Results

Member	Axial lbs	Vy lbs	Mz lb-ft	Dy in
M1	1484.50	51.81	-88.15	-0.1224
"	1484.50	-23.15	36.5526	-0.1628
"	1484.50	5.5187	65.9346	-0.1425
"	<b>1484.50</b>	34.1854	0.0000	-0.0000
M2	1484.50	34.19	-0.0000	-0.0000
"	1484.50	5.5187	65.9346	-0.1425
"	1484.50	23.1479	36.5526	-0.1628
"	1484.50	51.8146	-88.15	-0.1224
M3	<b>-1605.28</b>	122.54	0.0000	-0.0000
"	-1572.22	23.1716	<b>140.29</b>	-0.1156
"	-1539.16	-76.20	89.0621	-0.1431
"	-1506.10	<b>-175.56</b>	<b>-153.67</b>	-0.1187
M4	-1119.65	154.35	-153.67	-0.1187
"	-1092.55	73.0474	25.7758	-0.1346
"	-1065.45	-8.2526	77.0005	-0.1432
"	-1038.35	-89.55	0.0000	-0.1203
M5	-1119.65	-154.35	-153.67	-0.1060
"	-1092.55	-73.05	25.7758	-0.1220
"	-1065.45	8.2526	77.0005	-0.1305
"	-1038.35	89.5526	0.0000	-0.1077
M6	-1605.28	122.54	-0.0000	<b>0.0126</b>
"	-1572.22	-23.17	140.29	-0.1030
"	-1539.16	76.1951	89.0621	-0.1305
"	-1506.10	<b>175.56</b>	-153.67	-0.1061
M7	-508.57	0.0000	-0.0000	-0.1058
"	-508.57	0.0000	0.0000	-0.1031
"	-508.57	0.0000	-0.0000	-0.1007
"	-508.57	0.0000	0.0000	-0.0981
M8	486.80	0.0000	0.0000	-0.0200
"	486.80	0.0000	0.0000	-0.0200
"	486.80	0.0000	0.0000	0.0200
"	486.80	0.0000	0.0000	-0.0200
M9	-508.57	0.0000	0.0000	0.1209
"	-508.57	0.0000	0.0000	-0.1183
"	-508.57	0.0000	0.0000	-0.1157
"	-508.57	0.0000	0.0000	-0.1131

### **BENDING & COMP: TRUSS 1 - MEMBER 3**

Design based on 1997 UBC 2321 Division V and ANSI/TPI 1-1995

#### Grading:

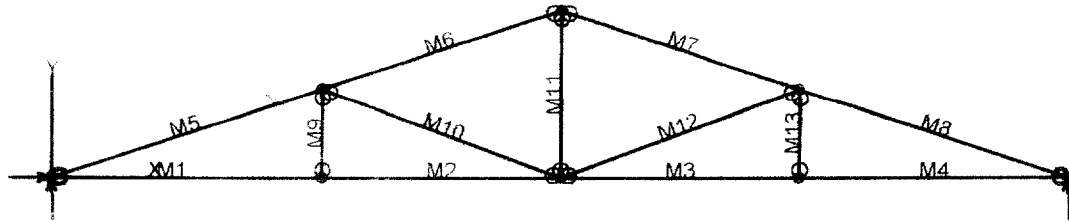
2x or 4x

Doug-fir larch No. 2

#### Assumptions:

Solid sheathing on top chord of truss. Therefore,  
continuous lateral support is provided along compression face  
Maximum center-center spacing = 24"

Width, b	1.5 inches
Depth, d	3.5 inches
Length	5.8 feet
Max Axial Comp. C	1506 lbs
Max Reaction, R	175 lbs
Max Moment, M	153 ft-lbs
Max LL Deflection	0.05 inches
Max TL Deflection	0.11 inches
LL Defl Criteria = L/	240
TL Defl Criteria = L/	180
Duration factor, Cd	1.25
Repetitive Factor, Cr	1.15
Size Factor, Cf bending	1.5 1.5 for 2x4, 1.3 for 2x6
Size Factor, Cf comp	1.15 1.15 for 2x4, 1.1 for 2x6
Buckling Factor, CT =	1.16
fc =	287 psi
Fce =	1496 psi
Fc* =	2084 psi
F'c =	1184 psi
fb =	600 psi
F'b = Fb* =	2156 psi
Shear D/C ratio	0.42 < 1.0, Member OK
Interaction equation, (fc/F'c)^2 +	
fb / (F'b(1-fc/Fce)) =	0.40 < 1.0, Member OK
Live Load defl ratio	0.17 < 1.0, Member OK
Total Load defl ratio	0.28 < 1.0, Member OK



# VisualAnalysis 3.50.c Report

09/31/01 08:22:44

Project: Truss 2

File: C:\Program Files\IES\VA35\truss 2.vap

Company: PK Associates Engineers

Engineer: Paul Zacher

Default Units: Feet, Pounds, Degrees, °Fahrenheit, Seconds.

## Nodes

Node	X ft	Y ft	Fix DX	Fix DY	Fix RZ
N1	0.00	0.00	Yes	Yes	No
N2	6.75	0.00	No	No	"
N3	12.75	0.00	"	"	"
N4	18.75	0.00	"	"	"
N5	25.50	0.00	"	Yes	"
N6	6.75	2.25	"	No	"
N7	18.75	2.25	"	"	"
N8	12.75	4.25	"	"	"

## Member Elements

Member	Section	Material	Length ft
M1	SS2x4	Wood	6.75
M2	"	"	6.00
M3	"	"	6.00
M4	"	"	6.75
M5	"	"	7.10
M6	"	"	6.30
M7	"	"	6.30
M8	"	"	7.10
M9	"	"	2.25
M10	"	"	6.41
M11	"	"	4.25
M12	"	"	6.41
M13	"	"	2.25

## Section Properties

Category	Section	Ax in <sup>2</sup>	Iz in <sup>4</sup>	Sy+ in <sup>3</sup>	Sy- in <sup>3</sup>
Wood	Sha SS2x4	5.25	5.36	3.06	3.06

## Material Properties

Material	Strength psi	Elasticity psi	Poisson	Density lb/ft <sup>3</sup>
Wood	-NA	1700000.00	0.38	40.47

## Load Combination Summary

Equation Case: Equation Case 1  
 Combination: +1D+1L+1Lr  
 Contributing Cases & Source

Service Case 1 Dead loads  
 Service Case 2 Roof Live loads

## Member Uniform Loads

This item is empty. Check the selection state, or report properties.

## Nodal Reactions

Node	Load Case	FX lbs	FY lbs	MZ lb-ft
01	Equation Case 1	0.00	838.08	-NA-
05	"	-NA	838.08	-NA-

## Member Results

Member	Axial lbs	Vy lbs	Mz lb-ft	Dy in
M1	1965.69	-19.37	2.3299	-0.1942
"	1965.69	-10.02	11.8754	-0.1665
"	1965.69	9.3298	42.6520	-0.1020
"	<b>1965.69</b>	28.6798	0.0000	-0.0000
M2	1965.69	-31.03	33.69	-0.2030
"	1965.69	-13.83	11.0788	-0.2090
"	1965.69	3.3737	21.5315	-0.2087
"	1965.69	20.5737	2.3299	-0.1942
M3	1965.69	-20.57	2.3299	-0.1942
"	1965.69	-3.3737	21.5315	-0.2087
"	1965.69	13.8263	11.0788	-0.2090
"	1965.69	31.0263	33.69	-0.2030
M4	1965.69	-28.68	0.0000	-0.0000
"	1965.69	-9.3298	42.6520	-0.1020
"	1965.69	10.0202	41.8754	-0.1665
"	1965.69	29.3702	2.3299	-0.1942
M5	<b>-2120.77</b>	146.26	0.0000	-0.0000
"	-2080.12	24.3104	<b>201.55</b>	-0.2246
"	-2039.47	97.64	114.59	-0.2600
"	-1998.82	<b>-219.59</b>	<b>-260.87</b>	-0.1980
M6	-1401.43	203.85	-260.87	-0.1980
"	-1365.29	95.4477	54.0404	-0.2530
"	-1329.16	-12.95	141.00	<b>-0.2780</b>
"	-1293.03	-121.35	0.0000	-0.2000
M	-1401.43	-203.85	-260.87	-0.1767
"	-1365.29	-95.45	54.0404	-0.2317
"	-1329.16	12.9523	141.00	-0.2566
"	-1293.03	121.35	0.0000	-0.1787
M8	-2120.77	-146.26	0.0000	0.0213
"	-2080.12	-24.31	201.55	0.2034
"	-2039.47	97.6396	114.59	-0.2387
"	-1998.82	<b>219.59</b>	-260.87	-0.1767
M9	49.9438	0.0000	0.0000	0.0178
"	49.9438	0.0000	0.0000	0.0265
"	49.9438	0.0000	0.0000	0.0352
"	49.9438	0.0000	0.0000	0.0439
M10	-748.28	-0.0000	-0.0000	-0.1782
"	-748.28	-0.0000	-0.0000	-0.1743
"	-748.28	-0.0000	-0.0000	-0.1703
"	-748.28	-0.0000	0.0000	-0.1663
M11	587.53	0.0000	0.0000	-0.0337
"	587.53	0.0000	0.0000	-0.0337
"	587.53	0.0000	0.0000	-0.0337
"	587.53	0.0000	0.0000	-0.0337

M 1	-748.28	0.0000	0.0000	0.0019
	-748.28	0.0000	0.0000	-0.1979
	748.28	0.0000	0.0000	-0.1939
	-748.28	0.0000	0.0000	0.1900
M 2	49.9438	0.0000	0.0000	0.0235
	49.9438	0.0000	0.0000	0.0322
	49.9438	0.0000	0.0000	0.0409
	49.9438	0.0000	0.0000	<b>0.0496</b>

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## **BENDING & COMP: TRUSS 2 - MEMBER 5**

Design based on 1997 UBC 2321 Division V and ANSI/TPI 1-1995

### Grading:

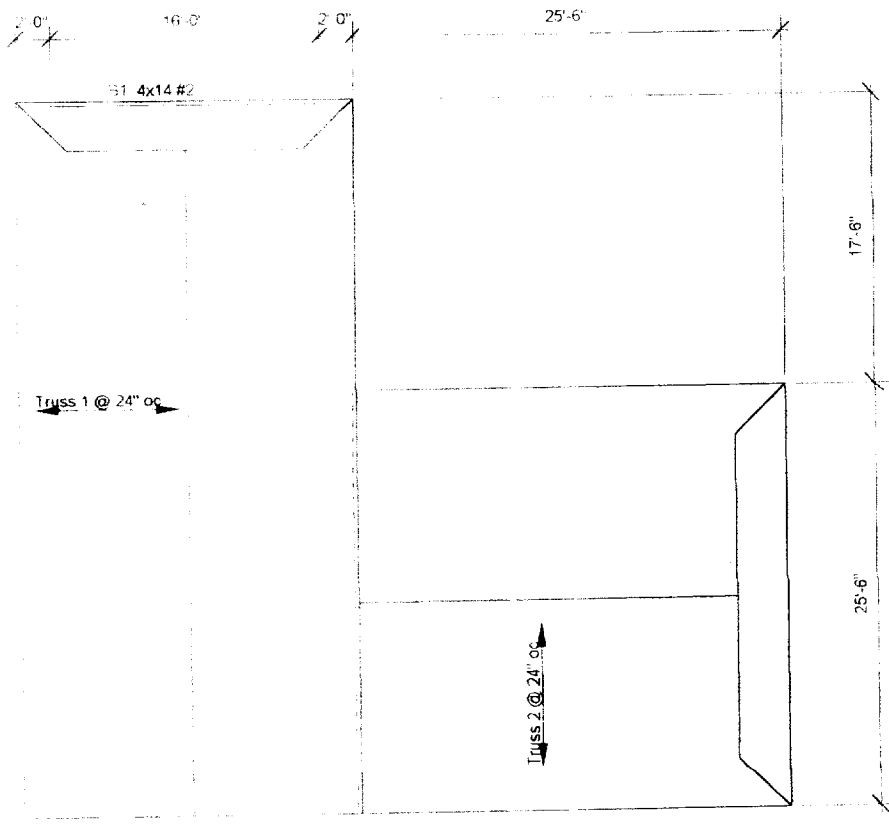
2x or 4x                                  Doug-fir larch No. 2

### Assumptions:

Solid sheathing on top chord of truss. Therefore,  
continuous lateral support is provided along compression face  
Maximum center-center spacing = 24"

Width, b	1.5 inches
Depth, d	3.5 inches
Length	7.12 feet
Max Axial Comp. C	1998 lbs
Max Reaction, R	219 lbs
Max Moment, M	260 ft-lbs
Max LL Deflection	0.1 inches
Max TL Deflection	0.19 inches
LL Defl Criteria = L/	240
TL Defl Criteria = L/	180
Duration factor, Cd	1.25
Repetitive Factor, Cr	1.15
Size Factor, Cf bending	1.5    1.5 for 2x4, 1.3 for 2x6
Size Factor, Cf comp	1.15    1.15 for 2x4, 1.1 for 2x6
Buckling Factor, CT =	1.20
fc =	381 psi
Fce=	1023 psi
Fc*=	2084 psi
F'c=	891 psi
fb=	1019 psi
F'b=Fb*=	2156 psi
Shear D/C ratio	0.53 < 1.0. Member OK
Interaction equation:	
(fc/F'c)^2 +	
fb/ (F'b(1-fc/Fce)) =	0.93 < 1.0. Member OK
Live Load defl ratio	0.28 < 1.0. Member OK
Total Load defl ratio	0.40 < 1.0. Member OK





Notes.

- 1 This is a reroof project. The new roofing material shall be a Light Weight Concrete Tile. The tile shall weigh less than or equal to 7.0 psf.
- 2 All structural wood members that were observed appear to be in sound condition and without structural defect.



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**ROOF PLAN - PEREZ**

Not to Scale

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