CITY OF SACRAMENTO
1231 I Street, Sacramento, CA 95814

Permit No: 0106185
Insp Area: 1

Site Address: 330 16TH ST SAC
Parcel No. 002-0092-012

Sub-Type: REM
Housing (Y/N): N

CONTRACTOR

OWNER
LEVINSKY CHARLES T/JEANNE M
8115 SOUTH LAKE CR
LOOMIS CA 95650

ARCHITECT

Nature of Work: REMOVE & REPLACE METAL AWNING

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name_____ Lender'sAddress_____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class____ License Number____ Date____ Contractor Signature_____

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code: any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars ($500.00).

✓ I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

MAY 15 2001

____ I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law.)

CITY OF SACRAMENTO
DEVELOPMENT SERVICES

____ I am exempt under Sec.____ B & PC for this reason:_____

Date 5-15-01 Owner Signature Daniel Flores DANIEL FLORES

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and herby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date____ Applicant/Agent Signature____

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:
____ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

____ I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier Policy Number Exp Date

✓ (This section need not be completed if the permit is for $100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 5-15-01 Applicant Signature Daniel Flores DANIEL FLORES

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS ($100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

Pacific Consulting Engineers

2150 Bell Ave, Suite 145 ● Sacramento, CA 95838 ● (916) 564-6028 ● Fax: (916) 564-6029

November 21, 2001

Bill Anson
Duralum Products, Inc.
8269 Alpine Ave.
Sacramento, CA 95826

RE: Awning Fix for Awning built at Taqueria Residence, 330 16th Street, Sacramento, CA.
This Letter is our Job# 01-1515.

Dear Mr. Anson:

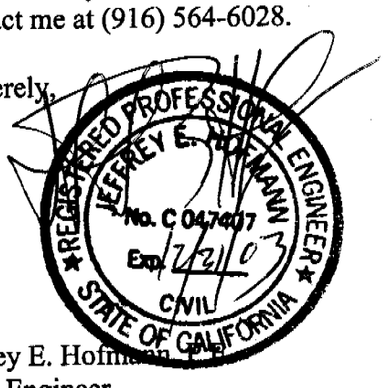
Per your request, I have looked at the configuration of the awnings placed at the aforementioned residence to determine whether an additional beams, posts, and footings are required at the back side of the decking for the awnings on the sides of the home (Awnings 1 and 3 as shown on Attachment 1).

It is my recommendation that the portion of Awnings 1 and 3 that are attached to the building, and Awning 2 are to be built per your ICBO 2640P. For the portions of Awnings 1 and 3 that extend beyond the sides of the buildings, they should be built per the calc's and details attached.

Please find the following attachments:
Attachment 1 of 4: Plan view of awning configuration and Section at location of new beam.
Attachment 2 of 4: Detail of column to slab connection
Attachment 3 and 4 of 4: Calculations regarding Awning not attached to building
Attachment D1 thru D2: Column Information
Attachment G1 thru G11: Beam Information

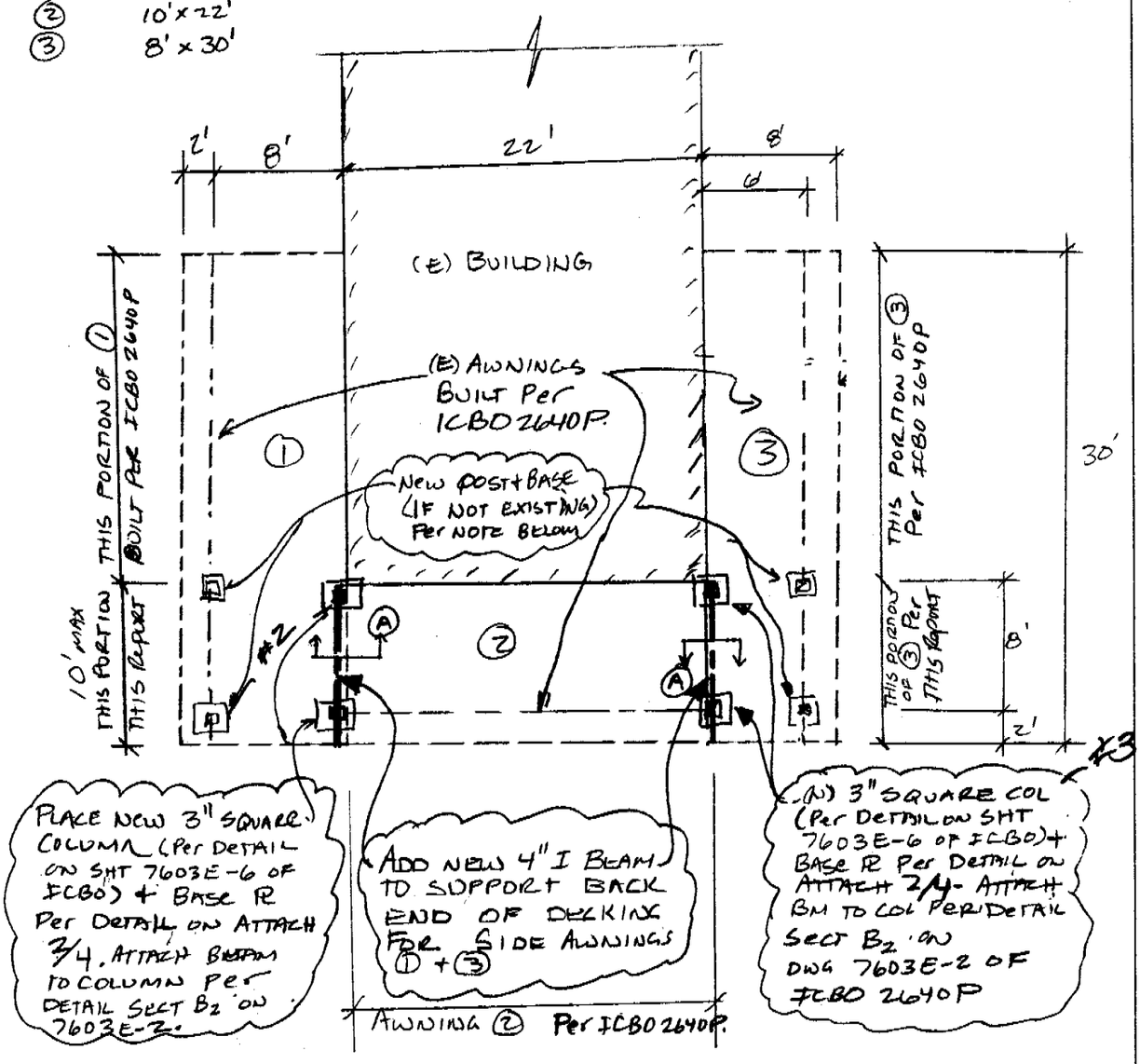
If there is any additional information I can provide regarding this matter please feel free to contact me at (916) 564-6028.

Sincerely,

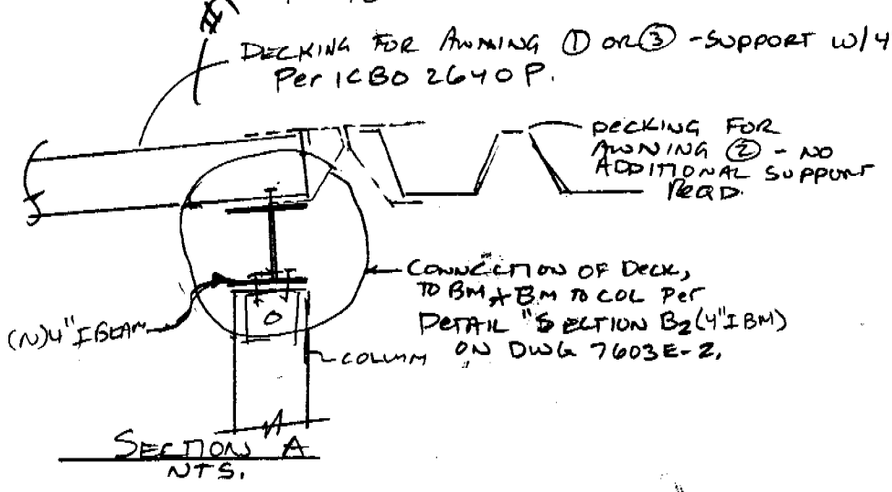


Jeffrey E. Hoffmann
Civil Engineer

AWNING	SIZE
①	10'x30'
②	10'x22'
③	8'x30'



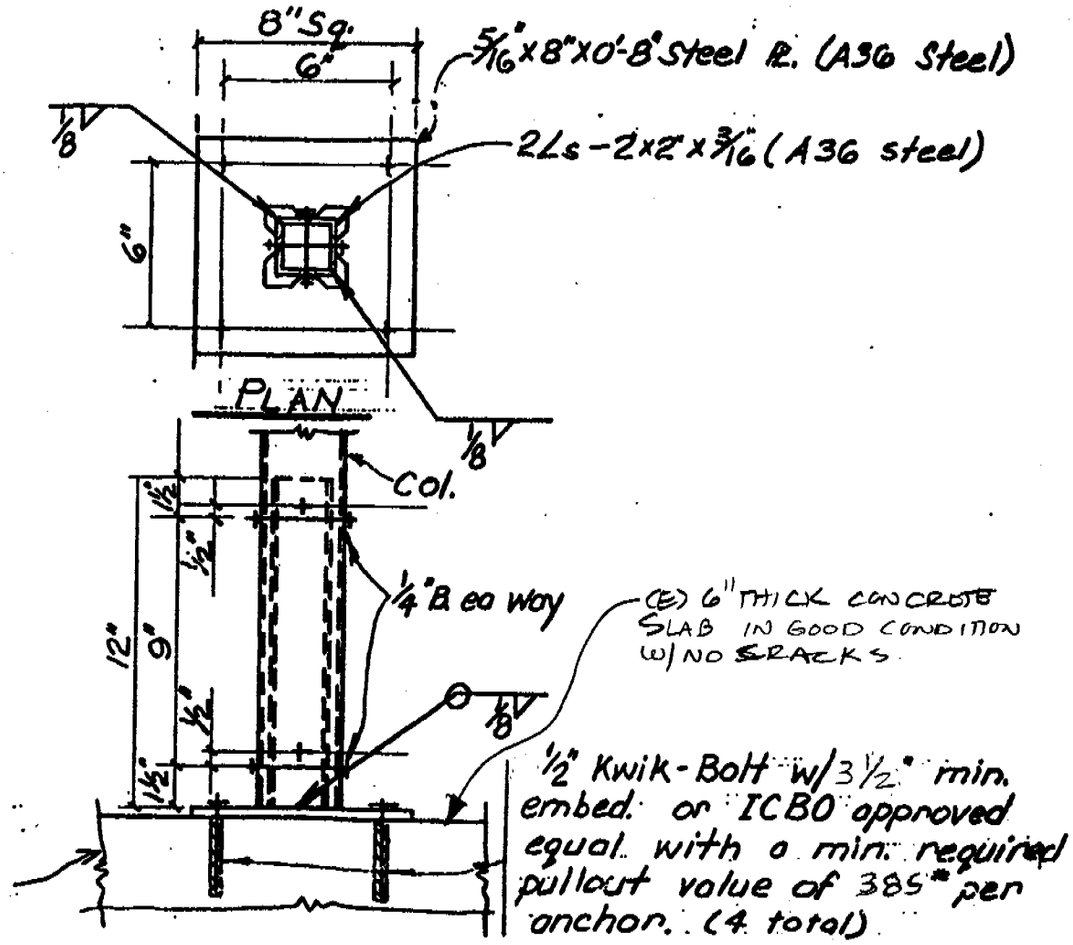
PLAN VIEW
1" = 10' ±



SECTION A
NTS.

REGISTERED PROFESSIONAL ENGINEER
JEFFREY E. [illegible]
No. C 047[illegible]
Exp. 12/31/03
CIVIL
STATE OF CALIFORNIA

PACIFIC CONSULTING ENGINEERS
2150 BELL AVE., SUITE 146
SACRAMENTO, CA 9582[illegible]



COLUMN TO BASE TO SLAB CONNECTION

REGISTERED PROFESSIONAL ENGINEER
JEFFREY E. HOFFMANN
No. C 047677
Exp. 12/31/03
CIVIL
STATE OF CALIFORNIA

PACIFIC CONSULTING ENGINEERS
2150 BELL AVE., SUITE 145
SACRAMENTO CA 95838

SIZE BEAM FOR SUPPORT @ BACK END OF DECKING OF ① & ③ WHERE NOT ATTACHED TO BLDG

SPAN = 8'

$W_{TL} = \frac{8}{2}(10+3) = 52$ PLF

$M_{MAX} = 52(8)^2/8 = 416'\text{-}\# = 4992''\text{-}\#$

FROM 4" I BM INFO ATTACHED (PP G1-67).

$M_{ALL} = 31800''\text{-}\# > 4992''\text{-}\#$

∴ USE OF 4" I BM OK

CONNECTIONS OF I BM TO COLUMN SAME AS SECT B_2 ON DWG # 7603E-2 OF ICBO - SEE PP 69 OF 4" I BM ATTACH.

LOADS TO COLUMNS

$P_{LAT} = \frac{(11.5\ PSF)(1')(10')}{2\ POSTS} = 29\#$

$P_{TL} = \frac{(10+3)(8')(10)^2}{2(8)} = 406\# < 750\#$ ∴ SLAB OK (NO FTG REQ'D)

$P_{DL} = \frac{11.5\ PSF\ (4')(10)^2}{2(8)} = 359\#$

FROM PP D1-D2 ATTACHED FOR 3" □ COLUMN - 18 Ga

$P_{TL\ ALLOW}(9'\text{-}6"\ ht) = .1397\ PST\ (1.626\ IN^2) = 875\#$ (P. D-2 OF ATTACH) $> 406\#$ ∴ OK

$M_{ALL} = 13.3''\text{-}K$ (PP D-18) $\geq 29\#(12)(10')^{ht} = 3.5''\text{-}K = M_{ACT}$ ∴ OK

COMBINED

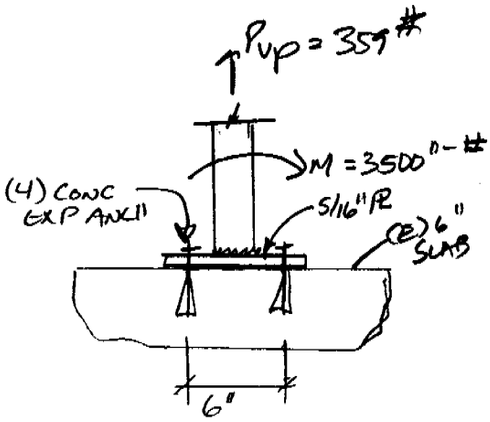
$\frac{406}{875} + \frac{3.5}{13.3} = 0.73 < 1.0$

∴ 3" □ x 18 Ga COLUMN OK

CONNECTION OF COLUMN TO BASE INSERT OK PER PP G-11 OF ATTACHMENT

STAEDTLER® No. 937 811E Engineer's Computation Pad

CONNECTION OF BASE PL TO CONC SLAB



$T_{MAX\ TO\ A,B} = \frac{359^{\#}}{4} + \frac{3500^{"-\#}}{2(6")} = 381^{\#}$

USE (4) 1/2"ø HILTI KWIK BOLTS W/ 3 1/2" EMBEDMENT INTO (E) 6" CONC SLAB.

Per ICBO ER4627

$T_{ALL} = 875^{\#} > 381^{\#}$ OK

$V_{ALL} = 1840 > \frac{29}{4} = 8^{\#}$

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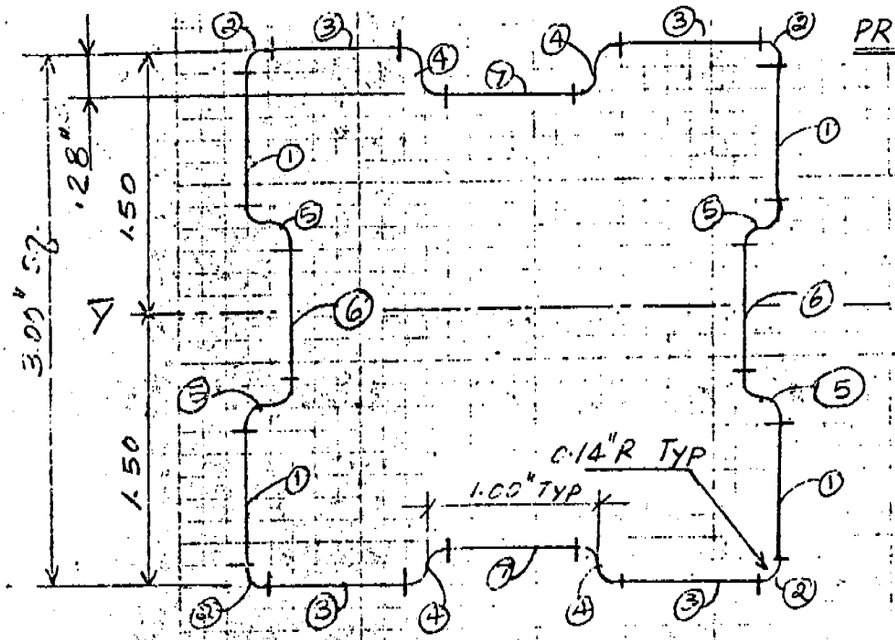
D-1

PROPERTIES OF 3" ⌀ COLUMN



PART	LENGTH	AREA	$\bar{Y}$	$A\bar{Y}$	$A\bar{Y}^2$	I_o
1	4X 0.72	2.880	1.600	2.880	2.880	4X.031 = 0.124
2	4X 0.22	0.880	1.449	1.275	1.848	4X.0004 = 0.002
3	4X 0.72	2.880	1.500	4.320	6.480	–
4	4X 0.44	1.760	1.360	2.394	3.255	4X.0043 = 0.017
5	4X 0.44	1.760	0.500	0.880	0.440	4X.0043 = 0.017
6	2X 0.72	1.440	0.000	–	–	2X.031 = 0.06
7	2X 0.72	1.440	1.220	1.757	2.143	–
		13.040			17.046	0.22
					0.222	
				I_T	17.268	

	THICK.	1.00"t	.040"t	.048"t	.060"t
			18 Ga.	16 Ga.	
A	in.²	13.040	0.522	0.626	0.782
I_T	in.⁴	17.268	0.691	0.829	1.036
r	in.	1.151	1.151	1.151	1.151
$\sqrt{I/A}$		11.512	0.460	0.553	0.691

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D-1a

PROPERTIES FOR 3"ϕ COLUMN STEEL ASTM A 446 YS = 40

REF: 1) AISI - SPEC. FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS 1968
2) AISI - COLD FORMED STEEL DESIGN MANUAL - PART IV

Determination of Q value	.048 (18Ga)	.060 (16Ga)
Area A	0.626	0.782
I_T	0.829	1.036
r	1.151	1.151
Flat width $w = 0.72 \times 3 =$	2.16	2.16
w/t	45.0	36.0
For $f = 0.6\,F_y = 0.6 \times 40 =$	24.0	24.0
Chart 2.3.1.1.(D) Eff. width ratio $\frac{b}{t}$	40.0	36.0
$\therefore$ Eff width $=$	1.92	2.16
$A_{eff} = \left(\frac{.048}{.060}\right)\left[\left(\frac{1.92}{2.16} \times 4\right) + (4 \times 0.22) + (8 \times 0.44)\right]$	0.580	0.783
$Q = \frac{A_{eff}}{A}$	0.927	1.00
$Q\,F_y$	37.08	40.0 ←

This value is used to determine allow load on col.

For bending stress since w/t lim $= \frac{184}{\sqrt{f}} = \frac{184}{\sqrt{24}} = 37.6$

The compression flanges are fully effective

$F_b = 0.6\,F_y = 0.6 \times 40 = 24$ ksi

SECT. 5.3

LATERALLY UNSUPPORTED LENGTH / DISTANCE BETWEEN WEBS $= \frac{l}{a} < \frac{2500}{F_y}$; $l = \frac{2500\,a}{F_y}$

$\frac{l}{3} < \frac{2500(3)}{40} = 187.5"$; $l < 15'6"$ (MAX UNSUPPORTED LENGTH WITH FUL ALLOWABLE STRES

18 Ga 16 Ga

$M_a = 24 \times .553 = 13.3"K$ $24 \times .691 = 16.6"K$

~~CONNECTION - SHEET 3.6.1.1(B) p.31~~

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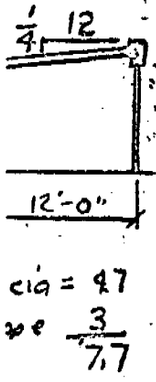
Column Loads

DL + LL on roof, $R = \frac{17.08}{2} \times 9.58 \times 10.7^{0.7} = 875^{\#}$ $\frac{DL}{57}$

DL + Wind up, $R = \frac{17.08}{2} \times 9.58 \times 9.3 = 761$

Horiz Wind, $H = 2\left(\frac{7.1}{12} \times \frac{11.5' \times 10^{\#}}{2 \text{ Colo pbay}}\right) = 74^{\#}$



Col - DL+LL

3"#, 18 Ga.

$\frac{P}{A} = \frac{875}{.626} = 1397$ psi.

$Max\ l = \frac{200 \times 1.15}{12 \times 2} = 9.6'$; $\frac{kl}{r} = 200$;

$QF_y = 37.08$

Chart 3.6.1.1B Pg 31

$F_a = 3700$ psi

Col - DL + Horiz Wind

$M = 74 \times 9.6' \times 12 = 8520^{\#}$

$\frac{P}{Af} = \frac{457}{.626 \times 3700} = .031$

$\frac{M}{Sf} = \frac{8520}{3531 \times 29000} = .69$

.659

OK allow 1.33

Web $h/t = \frac{7.4}{2(3 \times .048)} = 25.6$

.288

OK allow $1.33 \times 13.33P$ - Table 3.4.1

$h/t = \frac{3.0}{.048} = 62$

Par 3.4.2 - Bend in Web

$f_{w\,allow} = \frac{520{,}000{,}000}{62^2} = 135{,}000$

OK $f = 7200$

Par 3.4.3 - Combined Bend & Shear in Web

→ OK by inspection

Par. 3.5 - Web Crip.

$allow P = 2 \times 300 = 600$

Chart 7A

OK $P = 74$

* Light Gage Cold-Formed Steel Des Man. 1962 Ed., AISI

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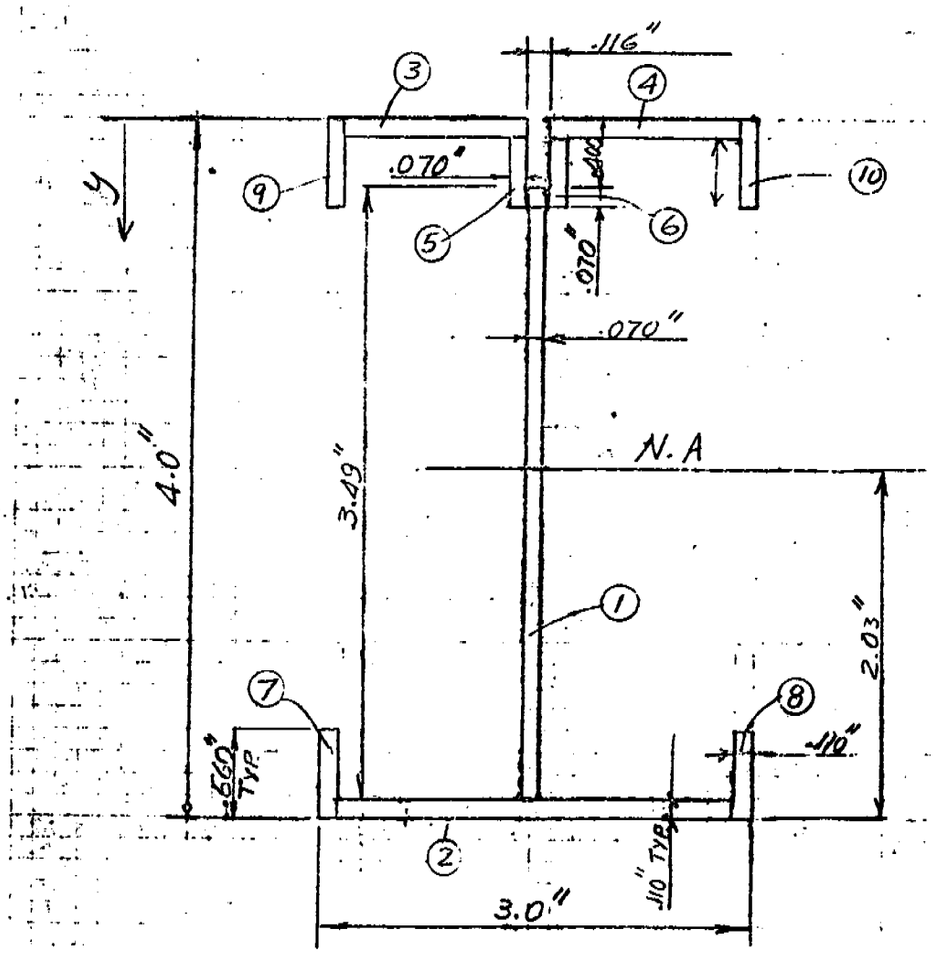
G 1

4" I-Beam (6061 -T6 Alum.)



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G2

4" I Beam Sect Properties (6061-T6 Alum) X-Axis

NO.	Dimensions	A (in^2)	Y (in)	AY (in^3)	AY^2 (in^4)	$I_o = A\frac{d^2}{12}$
1	3.490 x .070	.244	1.86	.454	.844	$\frac{.244(3.41)^2}{12} = .248$
2	2.780 x .110	.306	.055	.017	.001	—
3	1.532 x .110	.147	3.945	.580	2.288	—
4	1.332 x .110	.147	3.945	.580	2.288	—
5	.360 x .070	.025	3.710	.094	.347	—
6	.360 x .070	.025	3.710	.094	.347	—
7	.560 x .110	.062	.280	.017	.005	—
8	.560 x .110	.062	.280	.017	.005	—
9	.560 x .110	.062	3.720	.231	.859	—
10	.560 x .110	.062	3.720	.231	.859	—
		1.14	2.03	2.32	7.86	.248
					−4.71	
					3.15	
					0.25	
					3.40	

I_t =

$S_T = \frac{3.40}{2.03} = 1.67 \ in^3$

$S_B = \frac{3.40}{1.97} = 1.73 \ in^3$

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4" I BEAM PROPERTIES (6061-T6 ALUM) ABOUT 'Y' AXIS

Part	DIMENSIONS	$A(in^2)$	X (in)	$AX\ (in^3)$	$AX^2\ (in^4)$	$I = \frac{db^3}{12}$
1	3.490 x.070	.244	1.50	.366	.549	. —
2	2.780 x.110	.306	1.50	.459	.689	.200
3	1.332 x.110	.147	.776	.114	.089	.022
4	1.332 x.110	.147	2.22	.326	.724	.022
5	.360 x.070	.025	1.377	.034	.047	—
6	.360 x.070	.025	1.613	.040	.065	—
7	.560 x.110	.062	.055	.003	—	—
8	.560 x.110	.062	2.95	.183	.540	—
9	.560 x.110	.062	.055	.003	—	—
10	.560 x.110	.062	2.95	.183	.540	—
		1.14	1.500	1.71	3.243	.244

$$3.243 - 2.565 = .678$$

$$.678 + .244 = .922$$

$$I_t = .922$$

$$S_y = \frac{.922}{1.50} = .615 \ in^3$$

$$r = \sqrt{\frac{I}{A}} = \sqrt{\frac{.922}{1.14}} = .899 \ in$$

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G4

4" I Beam Allowable Stress & Moment (6061-T6 Alum)

	F_a (ksi)	$M_a = F_a \frac{I}{c}$ (K-IN)
Part ① for Spec 18 $\frac{h}{t} = \frac{3.49}{.070} = 49.9$; $40.5 - .27(49.9) =$	27.0	
Part ① For Spec 2	19.0	$19 \frac{3.40}{1.92} = 33.6$
Part ② For Spec 2	19.0	$19 \frac{3.40}{2.03} = 31.8$
Parts ③ & ④ for spec 16 $\frac{b}{t} = \frac{1.33}{.110} = 12.1$	21.0	
Parts ⑤ & ⑥ for Spec 18 $\frac{h}{t} = \frac{.360}{.070} = 5.1$	28.0	
Parts ⑦ & ⑧ For Spec 2	19.0	$19 \frac{3.40}{2.03} = 31.8$
Parts ⑨ & ⑩ For Spec 15 $\frac{h}{t} = \frac{.45}{.110} = 4.1$	21.0	

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G5

WEIGHT AVE. COMP. STRESS 47.2

Part	Area in²	F_a (Comp) psi	$F_a \times A$ (lbs)
①	$\frac{1}{6}(.244) = .041$	27.3	1.12
③	.147	21.0	3.15
④	.147	21.0	3.15
⑤	.025	28.0	.70
⑥	.025	28.0	.70
⑨	.062	21.0	1.30
⑩	.062	21.0	1.30
	.509		11.42

$F_{a_c} = \frac{11.42}{.509} = 22.44$ PSI

$Ma = 22.44 \times \frac{3.40}{1.97} = 38.7$ K-in (COMPRESSION)

$Ma = 31.8$ K-IN (Tension) Governs

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4" I Beam Allowable stress & Moments (~~For wind load~~) INVERTED

	F_a psi	$M_a = F_a \frac{I}{c}$
Part ① Wt For Spec. 18		
$\frac{h}{t} = \frac{3.49}{.070} = 49.9$; $40.5 - .27(49.9 - 40) =$	27.0	
Part ① Wt For Spec 2	19.0	$19 \frac{3.40}{1.46} = 44.2$
Part ② For Spec 16		
$\frac{b}{t} = \frac{2.78}{.110} = 25.3$; $27.3 - .29(25.3) =$	20.0	
Parts ③ & ④ For Spec. 2	19.0	$19 \frac{3.40}{1.97} = 32.8$
Parts ⑤ & ⑥ For Spec 2	19.0	$19 \frac{3.40}{1.86} = 34.7$
Parts ⑦ & ⑧ For Spec. 15		
$\frac{h}{t} = \frac{.56}{.11} = 5.10$	21.0	
Parts ⑨ & ⑩ For Spec 2	19.0	$19 \frac{3.40}{1.97} = 32.8$

1/27/97

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WEIGHT AVERAGE COMP. STRESS 47.2 INVERTED

Part	Area (in²)	Fa (COMP.) PSI	Fa x A (lbs)
①	$\frac{1}{6}(.344) = .041$	27.3	1.09
②	.306	20.0	6.12
⑦	.062	21.0	1.30
⑧	.062	21.0	1.30
	.471		9.81

$F_{ac} = \frac{9.81}{.471} = 20.8$ PSI

$M_a = 20.8 \frac{3.44}{2.03} = 34.8$ K-in (COMPRESSION)

$M_a = 32.8$ K-in (TENSION) GOVERNS.

M_a 32.2 X 1.33 = 42.8 K-in.
Uplift

ALLOWABLE SHEAR STRESS

SPEC. 20:

$\frac{h}{t} = \frac{3.44}{.07} = 49.1$

SHEAR STRESS ALLOW = 15.6 − .099 (15.6) = 14.1 KSI

F_v = Allow. Shear = 14.1 (3.44 x .07) = 3.4 K

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94-1147 G7 G8

4" I BEAM @ INT SPAN - CONNECT.

COL SPCG

EXAMPLE: LL. = 10 PSF
D.L. = 1 PSF
11 #/LF

FOR DECK SPAN = 17'-1"
t = .033"

w = 17.08' x 11 #/LF = 188

BENDING

Ma = 31.8 K-IN Pg.

$$\ell = \sqrt{\frac{Ma}{1.5w}}$$

$$\ell = \sqrt{\frac{31800}{1.5 \times 188}} = 10.6'$$

AXIAL - COL

Fa = 3700 PSI Pg. D2
A = .626 SI Pg. D1
Pa = 3700 x .626 = 2320#

$$\ell = \frac{Pa}{w}$$

$$\ell = \frac{2320\#}{188\#/1} = 12.3'$$

CHECK LATERAL BUCKLING

r = .864 IN Pg. G3
ℓ_{max} = 11'-5"

$$\ell/r = \frac{11.42 \times 12}{.864} = 152$$

SPEC 11

$$Fa = \frac{87000}{(\ell/r)^2} = \frac{57000}{152^2} = 3.7$$

S = 1.73

$Ma = Sf = 1.73 \times 3.7 = 6.52$

$$M = \frac{11.42^2 \times 129}{8} \times 12 = 25.2$$ N.G

BRACE @ MIDSPAN

$$M = \frac{5.71^2 \times 129}{8} = 6.3$$ OK

D.L + HORIZ. WIND

P = 1 #/LF x 11.42' x 11.75' = 134#

$$\frac{P}{AF} = \frac{134}{.626 \times 3700} = .06$$

$$M/Sf = \frac{.64}{.70} < 1.33$$ OK → D2

MATCH FASCIA BM COL. SPCG @ OUTSIDE SPANS

SUMMARY

PANEL #	DECK THICK	DECK SPAN 'P'	W = P x 11	BEAM SPAN COL.	BEAM SPAN BENDING	FASCIA BM SPAN Pg. C3	R ↓ = W x ℓ x 1.25 @ COL	R ← = 3/4 R → @ COL	
1	.018	11'-9"	129	18.0'	12.8'	11'-5"	1840#	1505#	18GA
1	.027	16'-0"	176	13.2'	11.0'	9'-11"	2180#	1785#	18W
1	.033	17'-1"	188	12.3'	10.6'	9'-7"	2250#	1845#	
2	.018	11'-9"	129	18.0'	12.8'	11'-5"	1840#	1505#	

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G 9

4" I BEAM @ INT. SPAN - CONNECTIONS

CONN. DECK TO BM

$W = 188 \times 9/11 = 154\#/1$ UPLIFT

FOR #14 SMS w/ 5/8" WASHER $P_t = 140t\ F_{ty}$

FOR #8 SMS w/ 1/2" " $P_t = \frac{1/2}{3/8} \times 140t\ F_{ty} = 112t\ F_{ty}$

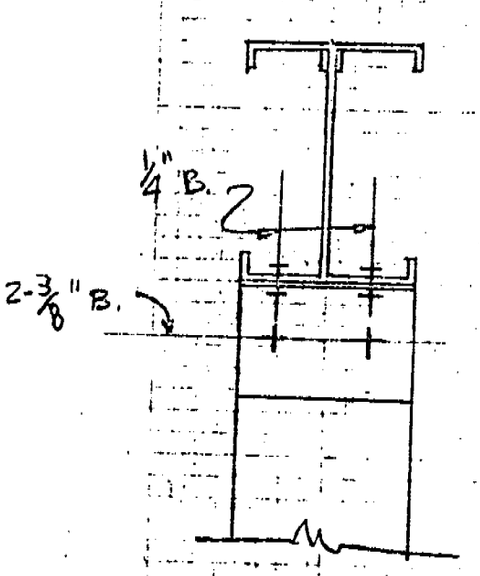
ALUM 3004 H36 $t = .033"$

$P_L = 112 \times .033 \times 28 = 103\#$

SPLG. $= \frac{103\#}{154\#/1} = .67'$ ..... SAY #8 SMS @ 6" C.

CONN. BM TO COL

$R\downarrow = 2250\#$ $R\uparrow = 1840\#$



BM TO COL. CAP

1/4" B. TENSION AREA $= \frac{.25^2 \times \pi}{4} = .049\ \square"$

$f_a = 20$ KSI

$P_L = .049 \times 20 = 980\#$

$2 \times 980 = 1960\# > 1840\#$ ✓ (OK)

COL CAP TO COL

3/8" B. BEAR ON .048" STEEL $y_s = 33$ KSI $\times 2.1 = 69$

$P_b = .048" \times .375" \times 69 = 1240\#$

BEAR ON .125" ALUM 6063 T6

$P_b = .125" \times .375" \times 24 = 1125\#$

SHEAR

$A = \frac{.375^2 \times \pi}{4} = .110\ \square"$

$f_a = 10$ KSI

$P_s = .110 \times 10 = 1100\#$ ⇐ GOVERNS

$2 \times 1100 = 2200\#$

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94-147 G 10.

4" I BEAM @ INT. SPAN - CONNECTIONS

CONNECTION COL. TO SLAB

R ↑ = 1840# Pg. GB

ANCHOR BOLTS

REACTION DUE TO MOMENT = 703# (Pg. P2)

NOTE: THE HORIZONTAL WIND DISTRIBUTED EQUALLY

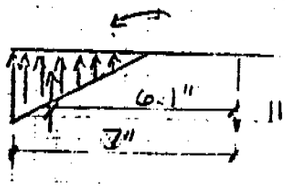
UPLIFT 1840/4 bolts = 460

+ 703

1163 / 1.33 = 875# ✓ OK

½" A.B. = 950# ✓ OK

BASE PLATE



J = 7/8

Jd = 7/8 × 7 = 49/8 = 6.1"

1163#/bolt × 2 bolts = 2326#

BENDING @ EDGE OF COL.

ANGLE 6 − 2.2" = 3.8"/2 = 1.9"

3" − 1.5" = 1.5"

M = 2326 × 1.9 = 4.42 "-K

$t_{REQD} = \sqrt{\frac{6 \times 4.42}{8 \times 27 \times 1.33}} = .30"$

5/16" = .31 ✓ OK

COMPRESSION IN CONCRETE

$f_c = \frac{2M}{KJbd^2} = \frac{2 \times 8440}{.4 \times .9 \times 8 \times 8^2}$ (Pg. P2) = 91.6 psi

F_c = .25 × 2000 = 500 ✓ OK

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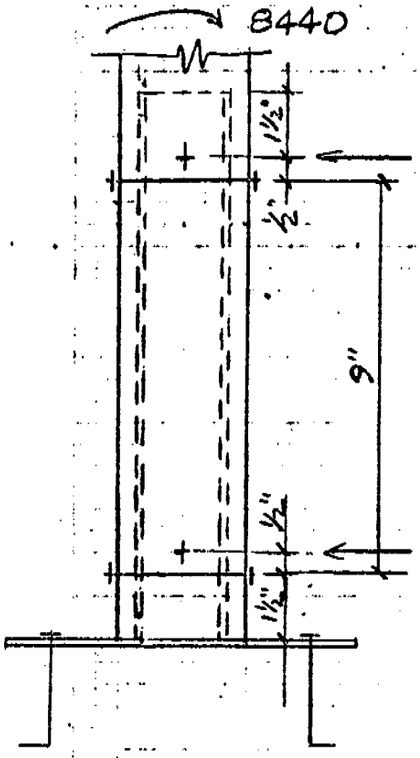
G 11

9" I BEAM @ INT. SPAN - CONNECTIONS

CONN. COL. TO INSERT



$F = \frac{8440''\#}{9''} = 940\#$

¼" B. BEAR .048" STEEL x.133
$P_b = .048 \times .25 \times 69 = 828$ 1100#

¼" B. SS
$P_s = 650\#$ Pg. B12

← GOVERNS

¼ B. D.S. = $2 \times 650 = 1300\# > 940\#$ ✓ OK

CITY OF SACRAMENTO
CALIFORNIA

PLANNING AND
BUILDING DEPARTMENT

PHONE 916-264-5381

1231 I STREET, ROOM 200
SACRAMENTO, CA 95814-2998

FAX 916-264-5543

STAFF LEVEL PROJECT REVIEW

DR Number:	DR01-082	Applicant/Owner:	Daniel Flores
Address:	330 116th Street	Date Filed:	May 2, 2001
Description:	New awning	Date Approved:	May 11th, 2001
Staff Contact:	Ellen A. Schmidt, 264-5962	APN:	002-0092-012

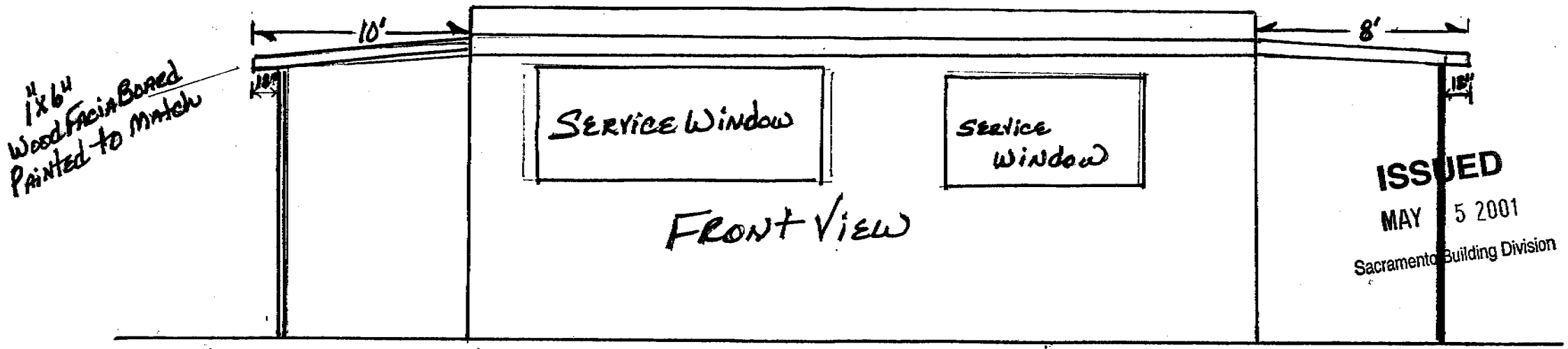
STAFF ACTION AND CONDITIONS OF APPROVAL:

Staff has reviewed the proposed project, and approves it with the following conditions of approval:

1. Provide new metal canopy and columns as indicated on drawings.
2. Provide new wood fascia with smooth finish. Rough-sawn is not allowed. Paint to match trim color.
3. Signage is not approved under this application.
4. No roof-mounted mechanical equipment is allowed.
5. The scope of work is limited to the above listed items. Any changes are subject to Design Review staff approval.

Ellen A. Schmidt

Ellen A. Schmidt
Assistant Architect
Design Review



ISSUED
MAY 5 2001
Sacramento Building Division

PARKING

CITY OF SACRAMENTO
DESIGN REVIEW
PROJECT NO: DR01-082
APPROVED BY: PHIL REED for Ellen Schmidt
APPROVAL DATE: 5/15/01

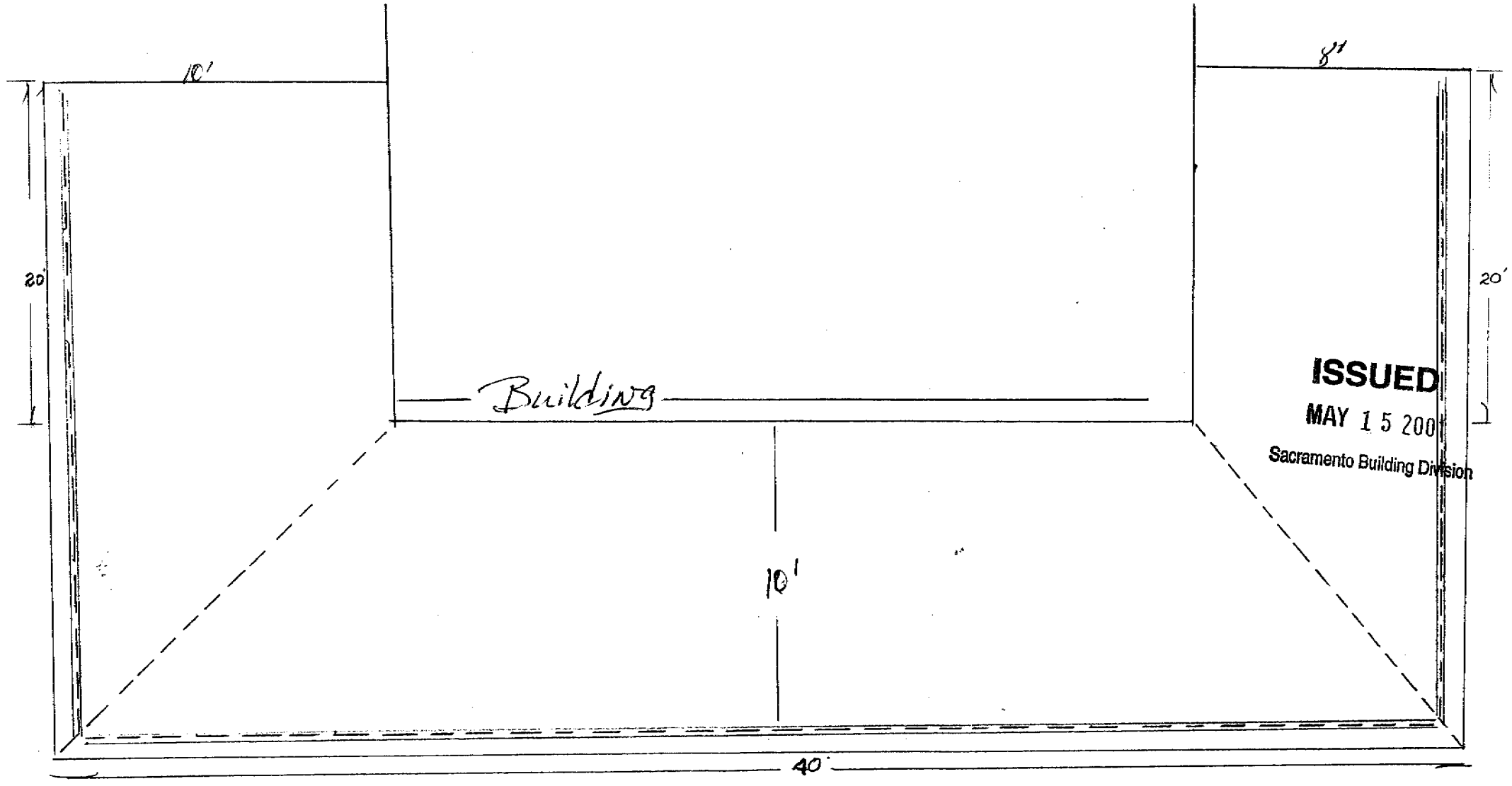
TAQUERIA JALISCO
SEE ATTACHED DESIGN REVIEW
330 CONDITIONS OF APPROVAL
SACRAMENTO, CA. APN 002-009-017

DEPT. OF PLANNING & DEVELOPMENT
APPROVED
UNDER TITLE 15 CITY CODE City of SACRAMENTO, CALIF.
BUILDING INSPECTION DIVISION

This set of plans and specifications must be kept on the job at all times and it is unlawful to make any changes or alterations from the same without written permission from the Building Inspection Division.

The approval of this plan and specification SHALL NOT be held to permit or approve the violation of any City Ordinance or State Law.

MAY 8 '01
DR01 082



ISSUED
MAY 15 200
Sacramento Building Division

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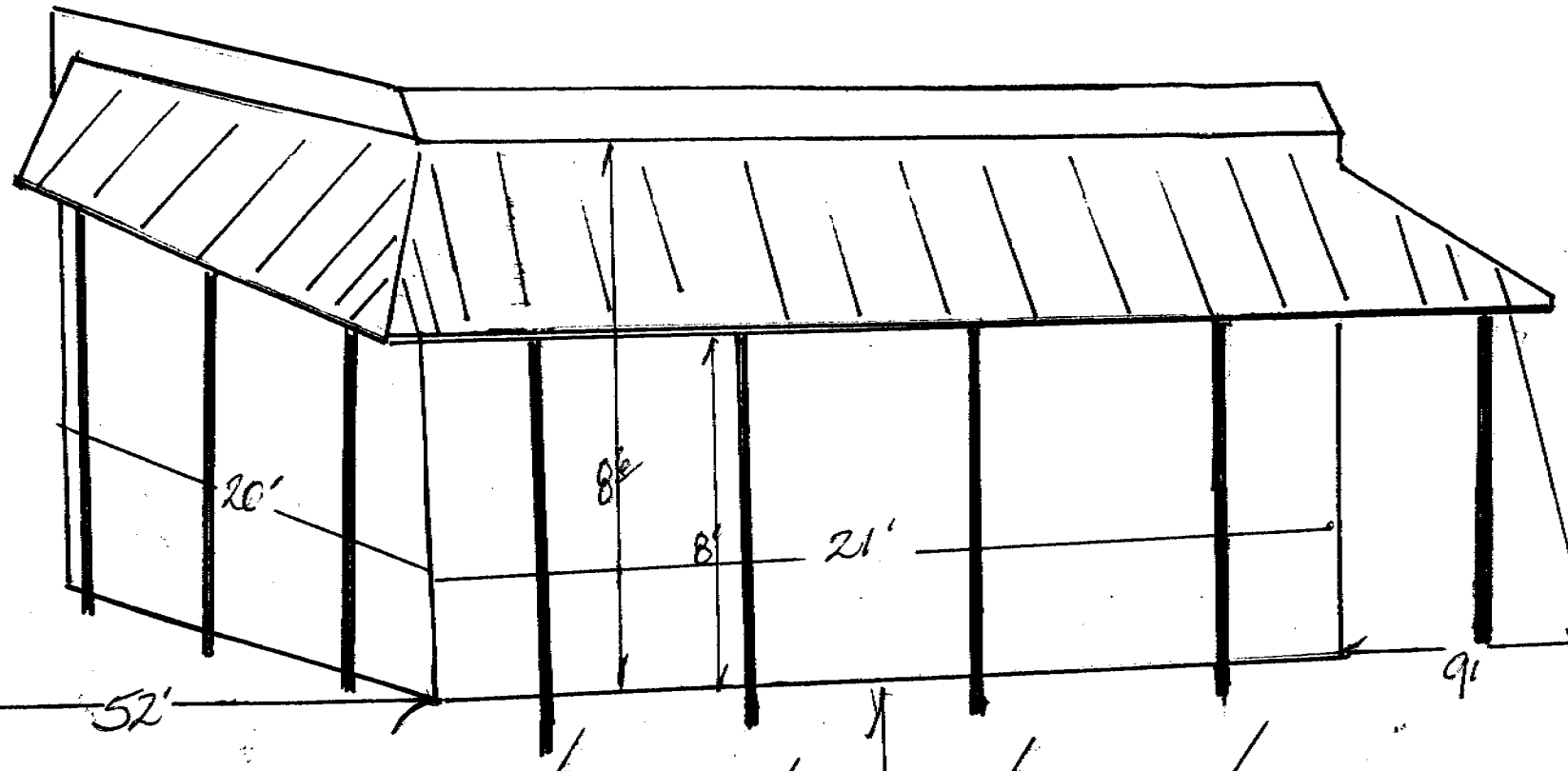
TAQUERIA JALISCO
330 16th St
SACRAMENTO, CA. APN#002009201 2

D R U T 082

7'



ISSUED
MAY 15 2001
Sacramento Building Division

16th St.

PARKING

FENCE

56'

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Taqueria Jalisco
330 16th St.
Sacramento, Ca. APN-002009207

SIDEWALK

STREET

MAY 2 '01

DR01 082