

CITY OF SACRAMENTO

1231 I Street, Sacramento, CA 95814

Permit No: 0111607

Insp Area: 4

Thos Bros: 277C5

Site Address: 3025 STONECREEK DR SAC

Parcel No: 225-0464-011

Sub-Type: RES

Housing (Y/N): N

CONTRACTOR

OWNER

ARCHITECT

ESPINOZA SAMUEL G/HILDA
3025 STONECREEK DR
SACRAMENTO CA 95833

Nature of Work: TEAR OFF & REROOF 27 SQ'S W/CHESTNUT BROWN CONCRETE TILE.

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

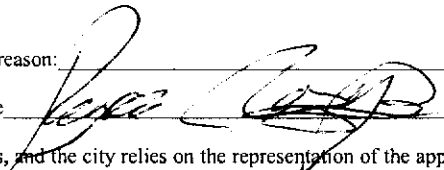
License Class _____ License Number _____ Date _____ Contractor Signature _____

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

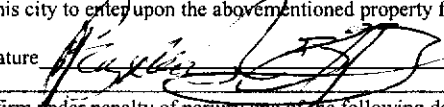
____ I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

____ I am exempt under Sec. _____ B & PC for this reason: _____

X Date 9-10-01 Owner Signature 

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

X Date 9-10-01 Applicant/Agent Signature 

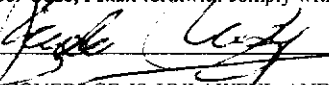
WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

____ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

____ I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier _____ Policy Number _____ Exp Date _____

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

X Date 9-10-01 Applicant Signature 

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.



DEPARTMENT OF
NEIGHBORHOODS, PLANNING
AND DEVELOPMENT SERVICES

CITY OF SACRAMENTO
CALIFORNIA

1231 I STREET
ROOM 200
SACRAMENTO, CA
95814-2904

DEVELOPMENT SERVICES
DIVISION

916-264-7619
FAX 916-264-7046

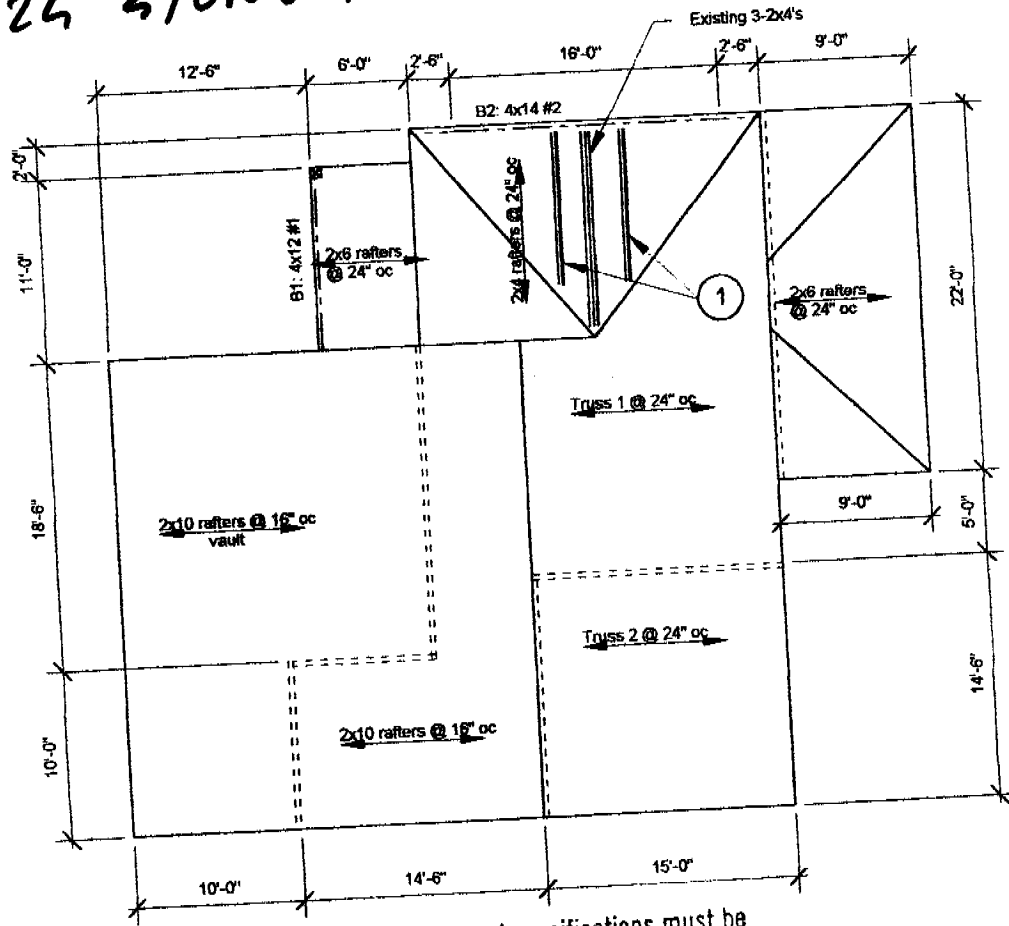
EXHIBIT 1

I have read and am familiar with the contents of the City's Standard
Owner-Builder Notification and Owner-Builder Verification, as required by
California Health and Safety Code Section 19830 and 19831. I authorize my
agent(s) Rogelio Varquez
to sign the Owner-Builder Verification on my behalf.

Signature Hilda Espinoza
Print Name Hilda Espinoza
Address 3025 Stonecreek Dr.
Sacramento, Ca. 95833
Telephone (916) 929-8876

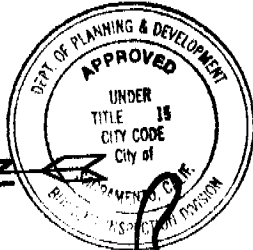
City of Sacramento

3025 STONECREEK DR.

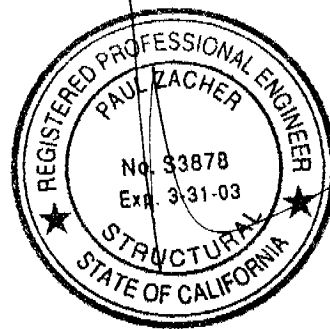


This set of plans and specifications must be kept on the job at all times and it is unlawful to make any changes or alterations from the same without written permission from the Building Inspection Division.

The approval of this plan and specification SHALL NOT be held to permit or approve the violation of any City Ordinance or State Law.



J. Zacher 9/10/01



FRAMING NOTES:

1. Scab a 2x4 to existing 2x4 rafters where the span is greater than 7'-9" (total 2).

Notes:

- A. This is a reroof project. The new roofing material shall be a Light Weight Concrete Tile. The tile shall weigh less than or equal to 7.0 psf.
- B. All rafters are 2x6 DF#2 and hips and valleys are 2x8 DF#2 unless otherwise noted.
- C. All existing rafter, hips, valleys, rafter ties, and purlins are braced per UBC Section 2320.1 "Roof and Ceiling Framing" unless otherwise shown.
- D. All structural wood members that were observed appear to be in sound condition and without structural defect.



ROOF PLAN - ESPINOZA

Not to Scale

15

Espinoza

Paul Zacher - Structural Engineers
4701 Lakeside Way
Fair Oaks, CA 95628

TEL: 916.961.3960
FAX: 916.961.6552

2001-09-10 10:00 AM

September 10, 2001

Rojelio Roofing
8263 Bedford Cove Way
Sacramento, CA 95828
TEL: (916) 508-2171
FAX:



Attn.: Mr. Rojelio,

re: Job 2001_276: ESPINOZA

Subject: Structural Investigation Report of the Roof for the Residence located at 3025 Stonecreek Drive
Sacramento, CA 95833.

As requested by Mr. Rojelio, this is a report to determine what needs should be addressed to correct any structural deficiencies of the roof. Paul Zacher visited the site September 10, 2001. The investigation was made to determine the existing condition of the structure. All information, data and analysis contained within this report are based on the 1997 Uniform Building Code.

The following is based on visual observations with no subsurface investigation being made.

DESCRIPTION:

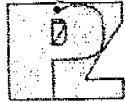
Type of Facility: Residence.
Year Built: Estimated 1980's vintage.
Occupancy: Residential.
No. of Stories: Two.
Dimensions: Approximately 2000 square feet with a first story plate height of 8 feet.

CONSTRUCTION:

Roof:
The roof covering will consist of a Light Weight Concrete Tile over 1/2" solid sheathing. The living area is framed with pre-engineered wood trusses spaced at 24" on center except for the vaulted ceiling areas. The vaulted ceiling is constructed of 2x10 rafters spaced at 16" on center.

CONCLUSIONS:

Roof:
The roof structure currently lacks sufficient structural capacity for the applied live and dead loads. See "Recommendations" for location and repair to bring the roof structure up to the required capacity.



Espinoza

Paul Zacher - Structural Engineers
4701 Lakeside Way
Fair Oaks, CA 95628

TEL: 916.961.3960
FAX: 916.961.6552

RECOMMENDATIONS:

If any of the following recommendations do not correspond to actual field conditions, the engineer of record shall be notified for further investigation and evaluation before continuing work.

Roof Structure:

1. Scab a 2x4 rafter to the existing 2x4 rafters with 16d's @ 12" on center where the span is greater than 7'-9". See detail 1.

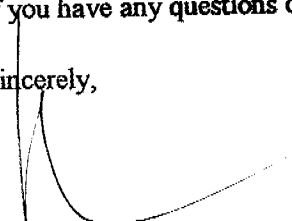
It shall be noted that small hairline cracking may occur at exterior stucco and interior gypboard finished walls that are load bearing or distributing roof strut loads. These cracks are a natural occurrence as the existing structure re-distributes the new roof weight. They are cosmetic in nature and are not an indication of a structural hazard or failure.

It shall be noted that some deflection of the rafters may be evident after installation of the tile. The existing roof framing has deflected but this may not be readily evident due to the uneven nature of the existing roofing material. Concrete tile is a very consistent and uniform product and when installed in an even plane, even small deflections can become apparent. This is only a cosmetic issue and not a structural concern.

The inspection consisted of visual observation only, made solely to determine the structural capacity of the existing roof. Analysis does not determine any effects on the overall structure under lateral forces or effects on the foundation unless specifically noted in the calculations and in this document. No warranties, expressed or implied, are made or intended in conjunction with this report. The inspection was made only to the portions that were accessible. The specific items noted were those that were observable and there may be defects that are not observable, or are hidden by architectural and structural materials.

If you have any questions on the above, do not hesitate to call.

Sincerely,


Paul Zacher, P.E., S.E.
file

DESIGN LOADING:

Roof Pitch 4 in 12
Pitch Adjustment Factor 1.05

LOCATION: ROOF

<u>MATERIAL</u>	<u>WEIGHT</u>	
Light Weight Tile	7.00	psf
Roofing felt	0.30	psf
1x4 skip sh't'g	1.09	psf
1/2" OSB/ plywood	1.50	psf
2x4 rafters @ 24" oc	<u>0.64</u>	psf
Load	10.5	psf
Roof Pitch Adjustment	<u>0.57</u>	psf
Total Load	11.1	psf

LOCATION: VAULT

<u>MATERIAL</u>	<u>WEIGHT</u>	
Light Weight Tile	7.00	psf
Roofing felt	0.30	psf
1/2" OSB/ plywood	1.50	psf
1x4 skip sh't'g	1.09	psf
2x10 rafters @ 16" oc	2.54	psf
Batt/blown insul	0.50	psf
1/2" Gypboard	<u>2.50</u>	psf
Load	15.4	psf
Roof Pitch Adjustment	<u>0.83</u>	psf
Total Load	16.3	psf

LOCATION: TOP CHORD

<u>MATERIAL</u>	<u>WEIGHT</u>	
Light Weight Tile	7.00	psf
Roofing felt	0.30	psf
1/2" OSB/ plywood	1.50	psf
1x4 skip sh't'g	1.09	psf
2x4 truss @ 24" oc	<u>0.64</u>	psf
Load	10.5	psf
Roof Pitch Adjustment	<u>0.57</u>	psf
Total Load	11.1	psf

LOCATION: BOTTOM CHORD

<u>MATERIAL</u>	<u>WEIGHT</u>	
Batt/blown insul	0.50	psf
2x4 truss @ 24" oc	1.28	psf
1/2" Gypboard	<u>2.50</u>	psf
Load	4.3	psf

Job #: 01-276

Date: 9/10/01

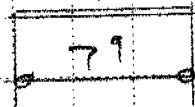
LOADING

RAFTER

DR = 11.1 psf x 2' = 22.2 psf 2 x 4 #2

LP = 16.0' = 32'

22.2 / 32

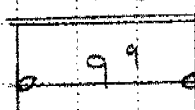


RAFTER

DR = 11.1 psf x 2' = 22.2 psf 2 x 4 #2

LP = 16.0' = 32'

22.2 / 32

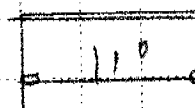


RAFTER

DR = 11.1 psf x 2' = 22.2 psf 3 x 2 x 4 #2

LP = 16.0' = 32' psf

22.2 / 32

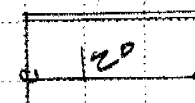


RAFTER

DR = 11.1 psf x 2' = 22.2 psf 2 x 6 #2

LP = 16.0' = 32'

22.2 / 32

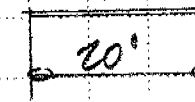


VAULT

DR = 16.3 psf x 4/3 = 21.7 psf 2 x 10 #2

LP = 16.0' = 21.7'

21.7 / 21.7

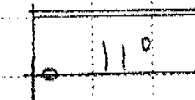


B1

DR = 11.1 psf x 3' = 33 psf 4 x 12 #1

LP = 16.0' = 48'

33 / 48

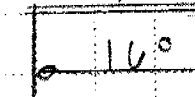


B2

DR = 11.1 psf x 6' = 67 psf 4 x 14 #2

LP = 16.0' = 96'

67 / 96



Paul Zacher - Structural Engineers
 4701 Lakeside Way
 Fair Oaks
 TEL: (916) 961-3960
 FAX: (916) 961-6552

Title :
 Dsgnr:
 Description :
 Scope :

Job #
 Date: 10:45AM, 10 SEP 01

Rev: 510304
 User: KW-0602844, Ver 5.1.3, 22-Jun-1999, Win32
 (c) 1983-99 ENERCALC

Timber Beam & Joist

c:\enercalc\test.ecw:Calculations

Description RAFTERS AND BEAMS

Timber Member Information Calculations are designed to 1997 NDS and 1997 UBC Requirements

	rafter	rafter	rafter	rafter	vault	B1	B2
Timber Section	2x4	2-2x4	3-2x4	2x6	2x10	4x12	4x14
Beam Width	1.500	3.000	4.500	1.500	1.500	3.500	3.500
Beam Depth	3.500	3.500	3.500	5.500	9.250	11.250	13.250
Le: Unbraced Length	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Timber Grade	Douglas Fir - Larch	Douglas Fir - Larch	Douglas Fir - Larch	Douglas Fir - Larch	Douglas Fir - Larch	Douglas Fir - Larch	Douglas Fir - Larch
Fb - Basic Allow	875.0	875.0	875.0	875.0	875.0	1,000.0	875.0
Fv - Basic Allow	95.0	95.0	95.0	95.0	95.0	95.0	95.0
Elastic Modulus	1,600.0	1,600.0	1,600.0	1,600.0	1,600.0	1,700.0	1,600.0
Load Duration Factor	1.250	1.250	1.250	1.250	1.250	1.250	1.250
Member Type	Sawn	Sawn	Sawn	Sawn	Sawn	Sawn	Sawn
Repetitive Status	Repetitive	Repetitive	Repetitive	Repetitive	Repetitive	No	No

Center Span Data

		7.75	9.75	11.00	12.00	20.00	11.00	16.00
Span	ft							
Dead Load	#/ft	22.20	22.20	22.20	22.20	21.70	33.00	67.00
Live Load	#/ft	32.00	32.00	32.00	32.00	21.30	48.00	96.00

Results Ratio = 0.8461 0.6688 0.5675 0.9467 0.8717 0.1448 0.5588

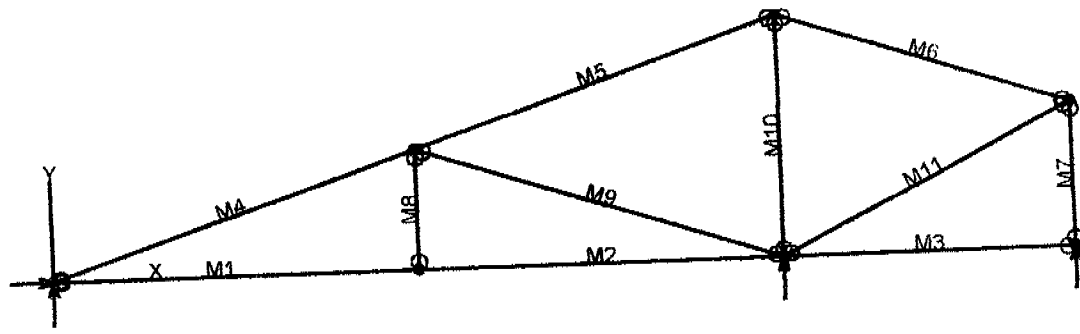
		4.88	7.73	9.84	11.71	25.80	14.70	62.59
Mmax @ Center	in-k							
@ X =	ft	3.87	4.87	5.50	6.00	10.00	5.50	8.00
fb : Actual	psi	1,594.5	1,261.8	1,070.7	1,548.1	1,206.1	199.1	611.2
Fb : Allowable	psi	1,886.7	1,886.7	1,886.7	1,635.2	1,383.6	1,375.0	1,093.8
		Bending OK	Bending OK	Bending OK	Bending OK	Bending OK	Bending OK	Bending OK
fv : Actual	psi	55.7	35.6	27.0	54.9	43.1	14.1	36.4
Fv : Allowable	psi	118.8	118.8	118.8	118.8	118.8	118.8	118.8
		Shear OK	Shear OK	Shear OK	Shear OK	Shear OK	Shear OK	Shear OK

Reactions

		86.02	108.22	122.10	133.20	217.00	181.50	536.00
@ Left End	DL	lbs						
	LL	lbs	124.00	156.00	176.00	192.00	213.00	264.00
	Max. DL+LL	lbs	210.02	264.22	298.10	325.20	430.00	445.50
@ Right End	DL	lbs	86.02	108.22	122.10	133.20	217.00	181.50
	LL	lbs	124.00	156.00	176.00	192.00	213.00	264.00
	Max. DL+LL	lbs	210.02	264.22	298.10	325.20	430.00	445.50

Deflections Ratio OK Deflection OK Deflection OK Deflection OK Deflection OK Deflection OK Deflection OK

		-0.210	-0.263	-0.284	-0.311	-0.494	-0.015	-0.091
Center DL Defl	in							
L/Defl Ratio		442.6	444.5	464.3	462.6	486.3	8,572.6	2,109.7
Center LL Defl	in	-0.303	-0.379	-0.410	-0.449	-0.484	-0.022	-0.130
L/Defl Ratio		307.0	308.4	322.1	320.9	495.4	5,893.6	1,472.4
Center Total Defl	in	-0.513	-0.643	-0.694	-0.760	-0.978	-0.038	-0.221
Location	ft	3.875	4.875	5.500	6.000	10.000	5.500	8.000
L/Defl Ratio		181.3	182.1	190.2	189.5	245.4	3,492.5	867.2



6

VisualAnalysis 3.50.c Report

09/10/01 10:57:01

Project: Truss 1

File: C:\Program Files\IES\VA35\truss 1.vap

Company: PK Associates Engineers

Engineer: Paul Zacher

Default Units: Feet, Pounds, Degrees, °Fahrenheit, Seconds.

Nodes

Node	X ft	Y ft	Fix	DX	Fix	DY	Fix	RZ
N1	0.00	0.00	Yes		Yes			No
N2	7.50	0.00	No		No			"
N3	15.00	0.00	"		Yes			"
N4	21.00	0.00	"		"			"
N5	7.50	2.50	"		No			"
N6	15.00	5.00	"		"			"
N7	21.00	3.00	"		"			"

Member Elements

Member	Section	Material	Length ft
M1	SS2x4	Wood	7.50
M2	"	"	7.50
M3	"	"	6.00
M4	"	"	7.91
M5	"	"	7.91
M6	"	"	6.32
M7	"	"	3.00
M8	"	"	2.50
M9	"	"	7.91
M10	"	"	5.00
M11	"	"	6.71

Section Properties

Category	Section	Ax in ²	Iz in ⁴	Sy+ in ³	Sy- in ³
Wood Sha	SS2x4	5.25	5.36	3.06	3.06

Material Properties

Material	Strength psi	Elasticity psi	Poisson	Density lb/ft ³
Wood	-NA-	1700000.00	0.36	40.47

Load Combination Summary

Equation Case: Equation Case 1

Combination: +1D+1L+1Lr

Contributing Cases & Source

Service Case 1 (Dead loads)

Service Case 2 (Roof Live loads)

Member Uniform Loads

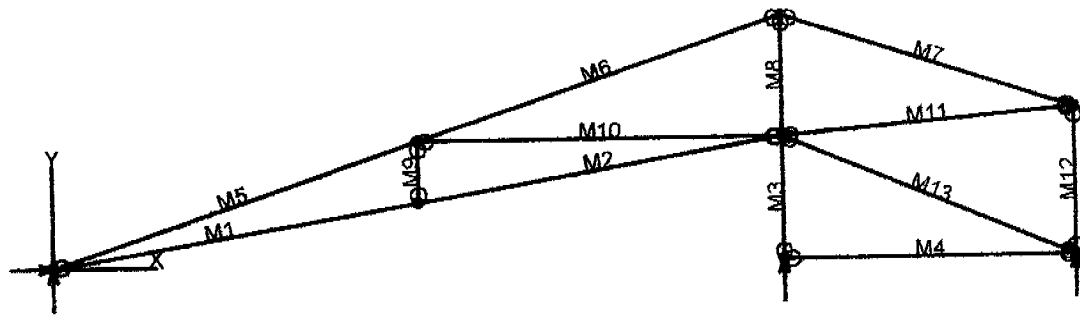
This item is empty. Check the selection state, or report properties.

Nodal Reactions

Node	Load Case	FX lbs	FY lbs	MZ lb-ft
N1	Equation Case 1	0.00	357.47	-NA-
N3	"	-NA-	1164.50	-NA-
N4	"	-NA-	-141.60	-NA-

Member Results

Member	Axial lbs	Vy lbs	Mz lb-ft	Dy in
M1	504.74	-37.25	-37.52	-0.0401
"	504.74	-15.75	28.6016	-0.0604
"	504.74	5.7472	41.1086	-0.0519
"	504.74	27.2472	0.0000	-0.0000
M2	504.74	-33.47	-46.64	-0.0000
"	504.74	-11.97	10.0140	-0.0213
"	504.74	9.5339	13.0543	-0.0358
"	504.74	31.0339	-37.52	-0.0401
M3	0.0000	-18.03	0.0000	0.0000
"	0.0000	-0.8263	18.7667	-0.0082
"	0.0000	16.3737	3.2194	-0.0043
"	0.0000	33.5737	-46.64	-0.0000
M4	-583.26	153.66	0.0000	-0.0000
"	-538.09	18.1643	225.51	-0.2143
"	-492.93	-117.34	94.8418	-0.1709
"	-447.76	-252.84	-392.01	-0.0402
M5	334.44	252.84	-392.01	-0.0402
"	379.61	117.34	94.8418	-0.1721
"	424.77	-18.16	225.51	-0.2170
"	469.94	-153.66	-0.0000	-0.0038
M6	364.52	-162.60	0.0000	0.0023
"	400.65	-54.20	227.96	-0.1759
"	436.78	54.2000	227.96	-0.1779
"	472.92	162.60	0.0000	-0.0038
M7	159.63	0.0000	0.0000	0.0054
"	159.63	0.0000	0.0000	0.0070
"	159.63	0.0000	0.0000	0.0086
"	159.63	0.0000	0.0000	0.0102
M8	68.2867	0.0000	0.0000	0.0051
"	68.2867	0.0000	0.0000	0.0059
"	68.2867	0.0000	0.0000	0.0067
"	68.2867	0.0000	0.0000	0.0075
M9	-950.76	0.0000	0.0000	-0.0355
"	-950.76	0.0000	0.0000	-0.0226
"	-950.76	0.0000	0.0000	-0.0097
"	-950.76	0.0000	0.0000	0.0032
M10	-598.19	0.0000	0.0000	-0.0102
"	-598.19	0.0000	0.0000	-0.0068
"	-598.19	0.0000	0.0000	-0.0034
"	-598.19	0.0000	0.0000	-0.0001
M11	-444.12	-0.0000	0.0000	-0.0046
"	-444.12	-0.0000	-0.0000	-0.0036
"	-444.12	-0.0000	-0.0000	-0.0027
"	-444.12	-0.0000	-0.0000	-0.0018



VisualAnalysis 3.50.c Report

09/10/01 10:55:23

Project: Truss 2

File: Untitled.Vap

Company: PK Associates Engineers

Engineer: Paul Zacher

Default Units: Feet, Pounds, Degrees, °Fahrenheit, Seconds.

Nodes

Node	X ft	Y ft	Fix	DX	Fix	DY	Fix	RZ
N1	0.00	0.00	Yes		Yes		No	
N2	7.50	1.25	No		No			
N3	15.00	2.50	"		"			
N4	15.00	0.00	"		Yes			
N5	21.00	0.00	"		"			
N6	7.50	2.50	"		No			
N7	15.00	5.00	"		"			
N8	21.00	3.00	"		"			

Member Elements

Member	Section	Material	Length ft
M1	SS2x4	Wood	7.60
M2	"	"	7.60
M3	"	"	2.50
M4	"	"	6.00
M5	"	"	7.91
M6	"	"	7.91
M7	"	"	6.32
M8	"	"	2.50
M9	"	"	1.25
M10	"	"	7.50
M11	"	"	6.02
M12	"	"	3.00
M13	"	"	6.50

Section Properties

Category	Section	Ax in ²	Iz in ⁴	Sy+ in ³	Sy- in ³
Wood Sha	SS2x4	5.25	5.36	3.06	3.06

Material Properties

Material	Strength psi	Elasticity psi	Poisson	Density lb/ft ³
Wood	-NA-	1700000.00	0.36	40.47

Load Combination Summary

Equation Case: Equation Case 1
Combination: +1D+1L+1Lr
Contributing Cases & Source

Service Case 1 (Dead loads)
 Service Case 2 (Roof Live loads)

Member Uniform Loads

This item is empty. Check the selection state, or report properties.

Nodal Reactions

Node	Load Case	FX lbs	FY lbs	MZ lb-ft
N1	Equation Case 1	0.00	348.62	-NA-
N4	"	-NA-	1199.49	-NA-
N5	"	-NA-	-165.96	-NA-

Member Results

Member	Axial lbs	Vy lbs	Mz lb-ft	Dy in
M1	926.47	29.4488	0.0000	-0.0000
"	930.06	7.9488	47.2555	-0.0991
"	933.64	-13.55	40.1558	-0.1462
"	937.22	-35.05	-21.30	-0.1498
M2	925.54	35.0512	-21.30	-0.1498
"	929.12	13.5512	40.1558	-0.1486
"	932.70	-7.9488	47.2555	-0.1039
"	936.29	-29.45	0.0000	-0.0072
M3	-1173.69	8.2923	-20.73	0.0200
"	-1173.69	8.2923	-13.82	0.0221
"	-1173.69	8.2923	-6.9102	0.0224
"	-1173.69	8.2923	0.0000	0.0218
M4	-8.2923	-25.80	0.0000	0.0000
"	-8.2923	-8.6000	34.3140	-0.0239
"	-8.2923	8.6000	34.3140	-0.0239
"	-8.2923	25.8000	0.0000	-0.0000
M5	-1020.78	157.14	0.0000	-0.0000
"	-975.62	21.6429	234.68	-0.2672
"	-930.45	-113.86	113.18	-0.2645
"	-885.28	-249.36	-364.51	-0.1506
M6	835.60	249.36	-364.51	-0.1506
"	880.77	113.86	113.18	-0.2669
"	925.93	-21.64	234.68	-0.2721
"	971.10	-157.14	-0.0000	-0.0072
M7	855.78	-162.60	0.0000	0.0047
"	891.91	-54.20	227.96	-0.1751
"	928.04	54.2000	227.96	-0.1786
"	964.18	162.60	0.0000	-0.0061
M8	-915.32	-8.2923	-0.0000	-0.0018
"	-915.32	-8.2923	6.9102	-0.0090
"	-915.32	-8.2923	13.8204	-0.0154
"	-915.32	-8.2923	20.7306	-0.0200
M9	71.0694	0.0000	0.0000	0.0340
"	71.0694	0.0000	0.0000	0.0354
"	71.0694	0.0000	0.0000	0.0367
"	71.0694	0.0000	0.0000	0.0380
M10	-1790.28	-0.0000	0.0000	-0.1461
"	-1790.28	-0.0000	-0.0000	-0.0987
"	-1790.28	-0.0000	-0.0000	-0.0513
"	-1790.28	-0.0000	-0.0000	-0.0039
M11	-866.27	-0.0000	0.0000	-0.0056
"	-866.27	-0.0000	-0.0000	-0.0038
"	-866.27	-0.0000	-0.0000	-0.0021

"	-866.27	-0.0000	-0.0000	-0.0003
M2	188.30	0.0000	0.0000	0.0125
"	188.30	0.0000	0.0000	0.0156
"	188.30	0.0000	0.0000	0.0186
"	188.30	0.0000	0.0000	0.0217
M13	8.9833	-0.0000	0.0000	0.0040
"	8.9833	-0.0000	-0.0000	0.0055
"	8.9833	-0.0000	-0.0000	0.0069
"	8.9833	-0.0000	-0.0000	0.0083

BENDING & COMP: TRUSS 2 - MEMBER 5

Design based on 1997 UBC 2321 Division V and ANSI/TPI 1-1995

Grading:

2x or 4x

Doug-fir larch: No. 2

Assumptions:

Solid sheathing on top chord of truss. Therefore,
continuous lateral support is provided along compression face
Maximum center-center spacing = 24"

Width, b	1.5 inches
Depth, d	3.5 inches
Length	7.91 feet
Max Axial Comp, C	885 feet
Max Reaction, R	249 feet
Max Moment, M	364 feet
Max LL Deflection	0.07 feet
Max TL Deflection	0.15 feet
LL Defl Criteria = L/	240
TL Defl Criteria = L/	180
Duration factor, Cd	1.25
Repetitive Factor, Cr	1.15
Size Factor, Cf bending	1.5 1.5 for 2x4, 1.3 for 2x6
Size Factor, Cf comp	1.15 1.15 for 2x4, 1.1 for 2x6
Buckling Factor, CT =	1.22
fc =	169 psi
Fce=	844 psi
Fc*=	2084 psi
F'c=	758 psi
fb=	1426 psi
F'b=Fb*=	2156 psi
Shear D/C ratio	0.60 < 1.0, Member OK
Interaction equation:	
(fc/F'c)^2 +	
fb/ (F'b(1-fc/Fce)) =	0.88 < 1.0, Member OK
Live Load defl ratio	0.18 < 1.0, Member OK
Total Load defl ratio	0.28 < 1.0, Member OK

BENDING & COMP: TRUSS 1 - MEMBER 4

Design based on 1997 UBC 2321 Division V and ANSI/TPI 1-1995

Grading:

2x or 4x

Doug-fir larch: No. 2

Assumptions:

Solid sheathing on top chord of truss. Therefore,
continuous lateral support is provided along compression face
Maximum center-center spacing = 24"

Width, b	1.5 inches
Depth, d	3.5 inches
Length	7.91 feet
Max Axial Comp, C	447 feet
Max Reaction, R	252 feet
Max Moment, M	392 feet
Max LL Deflection	0.02 feet
Max TL Deflection	0.04 feet
LL Defl Criteria = L/	240
TL Defl Criteria = L/	180
Duration factor, Cd	1.25
Repetitive Factor, Cr	1.15
Size Factor, Cf bending	1.5 1.5 for 2x4, 1.3 for 2x6
Size Factor, Cf comp	1.15 1.15 for 2x4, 1.1 for 2x6
Buckling Factor, CT =	1.22
fc =	85 psi
Fce =	844 psi
Fc* =	2084 psi
F'c =	758 psi
fb =	1536 psi
F'b = Fb* =	2156 psi
Shear D/C ratio	0.61 < 1.0, Member OK
Interaction equation:	
(fc/F'c)^2 +	
fb / (F'b(1-fc/Fce)) =	0.80 < 1.0, Member OK
Live Load defl ratio	0.05 < 1.0, Member OK
Total Load defl ratio	0.08 < 1.0, Member OK

0111607

OWNER-BUILDER VERIFICATION

ATTENTION PROPERTY OWNERS

An owner-builder building permit has been applied for in your name and bearing your signature.

Please complete and return this information in the envelope provided at your earliest opportunity to avoid unnecessary delay in processing and issuing your building permit. No building permit will be issued until this verification is received.

- ✓ 1. I personally plan to provide the major labor and materials for construction of the proposed Improvement (yes or no) YES
- ✗ 2. I (have/have not) YES signed an application for A building permit for the proposed work.

✗ 3. I have contracted with the following person (firm) to provide the proposed construction:

Name Rogelio Address 8763 Bedford Court
 City Santa Ca 95878 Telephone 308-2171
 Contractors License No. _____

4. I plan to provide portions of the work, but I have hired the following person to coordinate, Supervise, and provide the major work.

~~Name _____ Address _____
 City _____ Telephone _____
 Contractors License No. _____~~

5. I will provide some of the work but I have contracted (hired) the following to provide the Work indicated:

Name	Address	Phone	Type of work

✓ Signed [Signature]

Job Address 3025 Stonecreek Dr.

Permit No: 0111607