CITY OF SACRAMENTO

1231 I Street, Sacramento, CA 95814

Permit No: 0111607
Insp Area: 4
Thos Bros: 277C5

Site Address: 3025 STONECREEK DR SAC
Parcel No: 225-0464-011

Sub-Type: RES
Housing (Y/N): N

CONTRACTOR

OWNER
ESPINOZA SAMUEL G/HILDA
3025 STONECREEK DR
SACRAMENTO CA 95833

ARCHITECT

Nature of Work: TEAR OFF & REROOF 27 SQ'S W/CHESTNUT BROWN CONCRETE TILE.

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name_____ Lender'sAddress_____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class_____ License Number Date_____ Contractor Signature_____

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she isexempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of notmore than five hundred dollars ($500.00);

X I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

_____ I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

_____ I am exempt under Sec._____ B & PC for this reason:_____

X Date 9-10-01 Owner Signature_____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and herby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

X Date 9-10-01 Applicant/Agent Signature_____

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:
_____ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

_____ I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier Policy Number Exp Date

X (This section need not be completed if the permit is for $100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

X Date 9-10-01 Applicant Signature_____

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS ($100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

DEPARTMENT OF
NEIGHBORHOODS, PLANNING
AND DEVELOPMENT SERVICES

DEVELOPMENT SERVICES
DIVISION

CITY OF SACRAMENTO
CALIFORNIA

1231 I STREET
ROOM 200
SACRAMENTO, CA
95814-2904

916-264-7619
FAX 916-264-7046

EXHIBIT 1

I have read and am familiar with the contents of the City's Standard Owner-Builder Notification and Owner-Builder Verification, as required by California Health and Safety Code Section 19830 and 19831. I authorize my agent(s) Rogelio Vazquez to sign the Owner-Builder Verification on my behalf.

Signature Hilda Espinoza

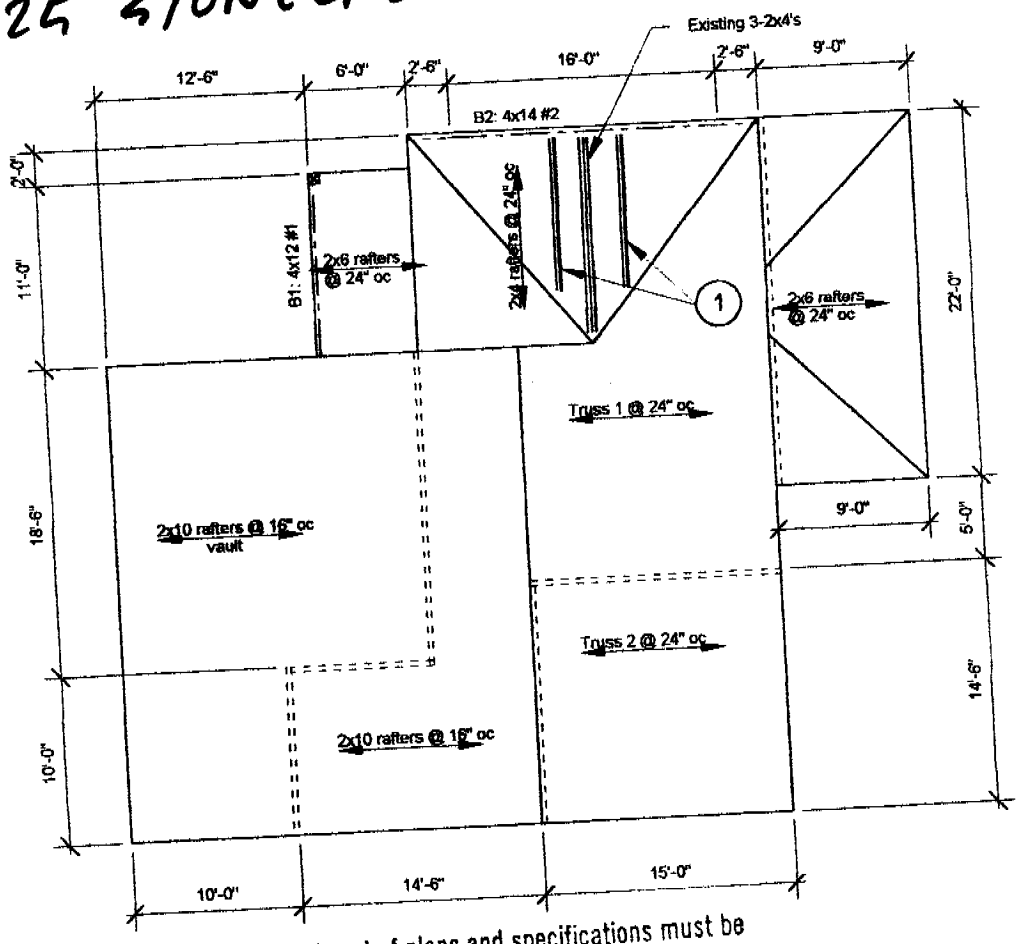
Print Name Hilda Espinoza

Address 3025 Stonecreek Dr.
Sacramento, Ca. 95833

Telephone (916) 929-8876

0111607

3025 STONECREEK DR.



This set of plans and specifications must be kept on the job at all times and it is unlawful to make any changes or alterations from the same without written permission from the Building Inspection Division.

The approval of this plan and specification SHALL NOT be held to permit or approve the violation of any City Ordinance or State Law.

DEPT. OF PLANNING & DEVELOPMENT APPROVED UNDER TITLE 15 CITY CODE City of SACRAMENTO, CA. BUILDING INSPECTION DIVISION

9/10/01

REGISTERED PROFESSIONAL ENGINEER PAUL ZACHER No. S3878 Exp. 3-31-03 STRUCTURAL STATE OF CALIFORNIA

FRAMING NOTES:
1. Scab a 2x4 to existing 2x4 rafters where the span is greater than 7'-9" (total 2).

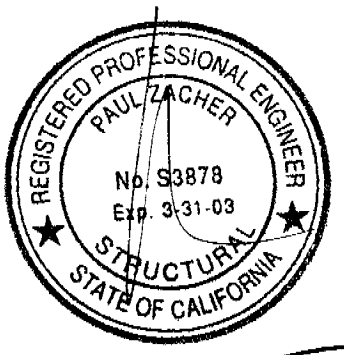
Notes:
A. This is a reroof project. The new roofing material shall be a Light Weight Concrete Tile. The tile shall weigh less than or equal to 7.0 psf.
B. All rafters are 2x6 DF#2 and hips and valleys are 2x8 DF#2 unless otherwise noted.
C. All existing rafter, hips, valleys, rafter ties, and purlins are braced per UBC Section 2320.1 "Roof and Ceiling Framing" unless otherwise shown.
D. All structural wood members that were observed appear to be in sound condition and without structural defect.

1 ROOF PLAN - ESPINOZA

Not to Scale 15

Espinoza
Paul Zacher – Structural Engineers
4701 Lakeside Way
Fair Oaks, CA 95628
TEL: 916.961.3960
FAX: 916.961.6552

September 10, 2001



Rojelio Roofing
8263 Bedford Cove Way
Sacramento, CA 95828
TEL: (916) 508-2171
FAX:

Attn.: Mr. Rojelio,

re: Job 2001_276: ESPINOZA

Subject: Structural Investigation Report of the Roof for the Residence located at 3025 Stonecreek Drive, Sacramento, CA 95833.

As requested by Mr. Rojelio, this is a report to determine what needs should be addressed to correct any structural deficiencies of the roof. Paul Zacher visited the site September 10, 2001. The investigation was made to determine the existing condition of the structure. All information, data and analysis contained within this report are based on the 1997 Uniform Building Code.

The following is based on visual observations with no subsurface investigation being made.

DESCRIPTION:

Type of Facility:	Residence.
Year Built:	Estimated 1980's vintage.
Occupancy:	Residential.
No. of Stories:	Two.
Dimensions:	Approximately 2000 square feet with a first story plate height of 8 feet.

CONSTRUCTION:

Roof:

The roof covering will consist of a Light Weight Concrete Tile over 1/2" solid sheathing. The living area is framed with pre-engineered wood trusses spaced at 24" on center except for the vaulted ceiling areas. The vaulted ceiling is constructed of 2x10 rafters spaced at 16" on center.

CONCLUSIONS:

Roof:

The roof structure currently lacks sufficient structural capacity for the applied live and dead loads. See "Recommendations" for location and repair to bring the roof structure up to the required capacity.

Espinoza

Paul Zacher – Structural Engineers
4701 Lakeside Way
Fair Oaks, CA 95628

TEL: 916.961.3960
FAX: 916.961.6552

RECOMMENDATIONS:

If any of the following recommendations do not correspond to actual field conditions, the engineer of record shall be notified for further investigation and evaluation before continuing work.

Roof Structure:

1. Scab a 2x4 rafter to the existing 2x4 rafters with 16d's @ 12" on center where the span is greater than 7'-9". See detail 1.

It shall be noted that small hairline cracking may occur at exterior stucco and interior gypboard finished walls that are load bearing or distributing roof strut loads. These cracks are a natural occurrence as the existing structure re-distributes the new roof weight. They are cosmetic in nature and are not an indication of a structural hazard or failure.

It shall be noted that some deflection of the rafters may be evident after installation of the tile. The existing roof framing has deflected but this may not be readily evident due to the uneven nature of the existing roofing material. Concrete tile is a very consistent and uniform product and when installed in an even plane, even small deflections can become apparent. This is only a cosmetic issue and not a structural concern.

The inspection consisted of visual observation only, made solely to determine the structural capacity of the existing roof. Analysis does not determine any effects on the overall structure under lateral forces or effects on the foundation unless specifically noted in the calculations and in this document. No warranties, expressed or implied, are made or intended in conjunction with this report. The inspection was made only to the portions that were accessible. The specific items noted were those that were observable and there may be defects that are not observable, or are hidden by architectural and structural materials.

If you have any questions on the above, do not hesitate to call.

Sincerely,

Paul Zacher, P.E., S.E.
file

DESIGN LOADING:

Roof Pitch	4	in 12
Pitch Adjustment Factor	1.05	

LOCATION: ROOF

MATERIAL	WEIGHT	
Light Weight Tile	7.00	psf
Roofing felt	0.30	psf
1x4 skip sht'g	1.09	psf
1/2" OSB/ plywood	1.50	psf
2x4 rafters @ 24" oc	0.64	psf
Load	10.5	psf
Roof Pitch Adjustment	0.57	psf
Total Load	11.1	psf

LOCATION: VAULT

MATERIAL	WEIGHT	
Light Weight Tile	7.00	psf
Roofing felt	0.30	psf
1/2" OSB/ plywood	1.50	psf
1x4 skip sht'g	1.09	psf
2x10 rafters @ 16" oc	2.54	psf
Batt/blown insul	0.50	psf
1/2" Gypboard	2.50	psf
Load	15.4	psf
Roof Pitch Adjustment	0.83	psf
Total Load	16.3	psf

LOCATION: TOP CHORD

MATERIAL	WEIGHT	
Light Weight Tile	7.00	psf
Roofing felt	0.30	psf
1/2" OSB/ plywood	1.50	psf
1x4 skip sht'g	1.09	psf
2x4 truss @ 24" oc	0.64	psf
Load	10.5	psf
Roof Pitch Adjustment	0.57	psf
Total Load	11.1	psf

LOCATION: BOTTOM CHORD

MATERIAL	WEIGHT	
Batt/blown insul	0.50	psf
2x4 truss @ 24" oc	1.28	psf
1/2" Gypboard	2.50	psf
Load	4.3	psf

P.K. Zacher, S.E.
4701 Lakeside Way
Fair Oaks, CA 95628
TEL: (916) 961-3960
FAX: (916) 961-6552

Job #: 01-276

Date: 9/10/01

LOADING

RAFTER

D_R = 11.1 psf × 2' = 22.2 plf 2×4 @ 24" O.C.

L_R = 16.0' × 2' = 32

22.2/32
7'-9"

RAFTER

D_R = 11.1 psf × 2' = 22.2 plf 2×4 @ 24" O.C.

L_R = 16.0' × 2' = 32

22.2/32
9'-9"

RAFTER

D_R = 11.1 psf × 2' = 22.2 plf 2×4 @ 24" O.C.

L_R = 16.0' × 2' = 32 plf

22.2/32
11'-0"

RAFTER

D_R = 11.1 psf × 2' = 22.2 plf 2×6 @ 24" O.C.

L_R = 16.0' × 2' = 32

22.2/32
12'-0"

VAULT

D_R = 16.3 psf × 4/3 = 21.7 plf 2×10 @ 24" O.C.

L_R = 16.0' × 2' = 21.3

21.7/21.3
10'-0"

B1

D_R = 11.1 psf × 3' = 33 plf 4×12 @ 1

L_R = 16.0' × 3' = 48'

33/48
11'-0"

B2

D_R = 11.1 psf × 6' = 67 plf 4×14 @ 2

L_R = 16.0' × 6' = 96'

67/98
16'-0"

Paul Zacher - Structural Engineers
4701 Lakeside Way
Fair Oaks
TEL: (916) 961-3960
FAX: (916) 961-6552

Title :
Dsgnr:
Description :

Job #
Date: 10:45AM, 10 SEP 01

Scope :

Rev: 510304
User: KW-0602844, Ver 5.1.3, 22-Jun-1999, Win32
(c) 1983-99 ENERCALC

Timber Beam & Joist

c:\enercalc\test.ecw:Calculations

Description RAFTERS AND BEAMS

Timber Member Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

		rafter	rafter	rafter	rafter	vault	B1	B2
Timber Section		2x4	2-2x4	3-2x4	2x6	2x10	4x12	4x14
Beam Width	in	1.500	3.000	4.500	1.500	1.500	3.500	3.500
Beam Depth	in	3.500	3.500	3.500	5.500	9.250	11.250	13.250
Le: Unbraced Length	ft	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Timber Grade		ouglas Fir - Larch,	ouglas Fir - Larch	Douglas Fir - Larch,	Douglas Fir - Larch	Douglas Fir - Larch	Douglas Fir - Larch,	Douglas Fir - Larch,
Fb - Basic Allow	psi	875.0	875.0	875.0	875.0	875.0	1,000.0	875.0
Fv - Basic Allow	psi	95.0	95.0	95.0	95.0	95.0	95.0	95.0
Elastic Modulus	ksi	1,600.0	1,600.0	1,600.0	1,600.0	1,600.0	1,700.0	1,600.0
Load Duration Factor		1.250	1.250	1.250	1.250	1.250	1.250	1.250
Member Type		Sawn	Sawn	Sawn	Sawn	Sawn	Sawn	Sawn
Repetitive Status		Repetitive	Repetitive	Repetitive	Repetitive	Repetitive	No	No

Center Span Data

Span	ft	7.75	9.75	11.00	12.00	20.00	11.00	16.00
Dead Load	#/ft	22.20	22.20	22.20	22.20	21.70	33.00	67.00
Live Load	#/ft	32.00	32.00	32.00	32.00	21.30	48.00	96.00

Results

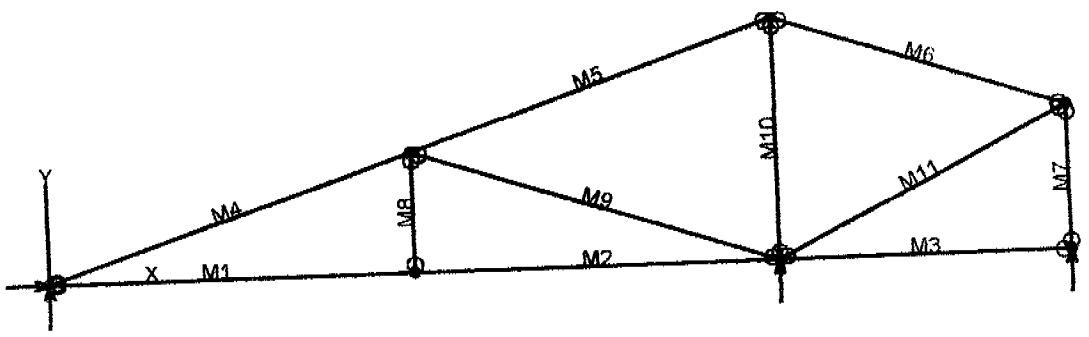
	Ratio =	0.8451	0.6688	0.5675	0.9467	0.8717	0.1448	0.5588
Mmax @ Center	in-k	4.88	7.73	9.84	11.71	25.80	14.70	62.59
@ X =	ft	3.87	4.87	5.50	6.00	10.00	5.50	8.00
fb : Actual	psi	1,594.5	1,261.8	1,070.7	1,548.1	1,206.1	199.1	611.2
Fb : Allowable	psi	1,886.7	1,886.7	1,886.7	1,635.2	1,383.6	1,375.0	1,093.8
		Bending OK	Bending OK	Bending OK	Bending OK	Bending OK	Bending OK	Bending OK
fv : Actual	psi	55.7	35.6	27.0	54.9	43.1	14.1	36.4
Fv : Allowable	psi	118.8	118.8	118.8	118.8	118.8	118.8	118.8
		Shear OK	Shear OK	Shear OK	Shear OK	Shear OK	Shear OK	Shear OK

Reactions

@ Left End	DL	lbs	86.02	108.22	122.10	133.20	217.00	181.50	536.00
	LL	lbs	124.00	156.00	176.00	192.00	213.00	264.00	768.00
	Max. DL+LL	lbs	210.02	264.22	298.10	325.20	430.00	445.50	1,304.00
@ Right End	DL	lbs	86.02	108.22	122.10	133.20	217.00	181.50	536.00
	LL	lbs	124.00	156.00	176.00	192.00	213.00	264.00	768.00
	Max. DL+LL	lbs	210.02	264.22	298.10	325.20	430.00	445.50	1,304.00

Deflections

		Ratio OK	Deflection OK	Deflection OK	Deflection OK	Deflection OK	Deflection OK	Deflection OK
Center DL Defl	in	-0.210	-0.263	-0.284	-0.311	-0.494	-0.015	-0.091
L/Defl Ratio		442.6	444.5	464.3	462.6	486.3	8,572.6	2,109.7
Center LL Defl	in	-0.303	-0.379	-0.410	-0.449	-0.484	-0.022	-0.130
L/Defl Ratio		307.0	308.4	322.1	320.9	495.4	5,893.6	1,472.4
Center Total Defl	in	-0.513	-0.643	-0.694	-0.760	-0.978	-0.038	-0.221
Location	ft	3.875	4.875	5.500	6.000	10.000	5.500	8.000
L/Defl Ratio		181.3	182.1	190.2	189.5	245.4	3,492.5	867.2


Y
X
M1
M2
M3
M4
M5
M6
M7
M8
M9
M10
M11

VisualAnalysis 3.50.c Report

09/10/01 10:57:01
Project: Truss 1
File: C:\Program Files\IES\VA35\truss 1.vap
Company: PK Associates Engineers
Engineer: Paul Zacher
Default Units: Feet, Pounds, Degrees, °Fahrenheit, Seconds.

Nodes

Node	X *ft*	Y *ft*	Fix DX	Fix DY	Fix RZ
N1	0.00	0.00	Yes	Yes	No
N2	7.50	0.00	No	No	"
N3	15.00	0.00	"	Yes	"
N4	21.00	0.00	"	"	"
N5	7.50	2.50	"	No	"
N6	15.00	5.00	"	"	"
N7	21.00	3.00	"	"	"

Member Elements

Member	Section	Material	Length *ft*
M1	SS2x4	Wood	7.50
M2	"	"	7.50
M3	"	"	6.00
M4	"	"	7.91
M5	"	"	7.91
M6	"	"	6.32
M7	"	"	3.00
M8	"	"	2.50
M9	"	"	7.91
M10	"	"	5.00
M11	"	"	6.71

Section Properties

Category	Section	Ax *in^2*	Iz *in^4*	Sy+ *in^3*	Sy- *in^3*
Wood Sha	SS2x4	5.25	5.36	3.06	3.06

Material Properties

Material	Strength *psi*	Elasticity *psi*	Poisson	Density *lb/ft^3*
Wood	-NA-	1700000.00	0.36	40.47

Load Combination Summary

Equation Case: Equation Case 1
Combination: +1D+1L+1Lr
Contributing Cases & Source
Service Case 1 (Dead loads)
Service Case 2 (Roof Live loads)

Member Uniform Loads

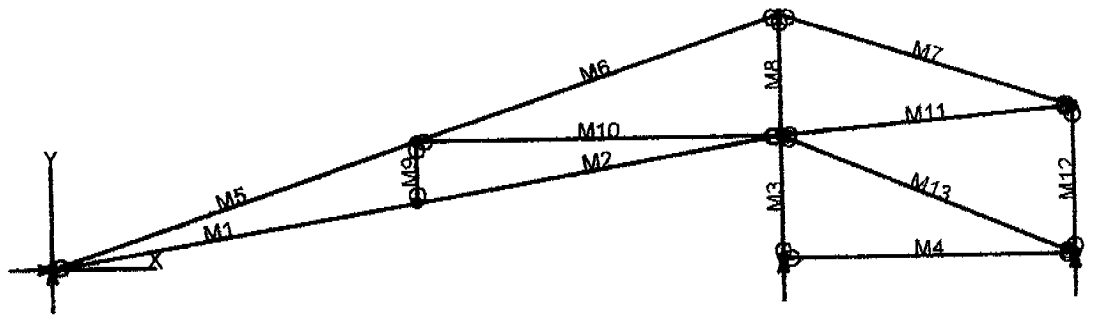
This item is empty. Check the selection state, or report properties.

Nodal Reactions

Node	Load Case	FX *lbs*	FY *lbs*	MZ *lb-ft*
N1	Equation Case 1	**0.00**	357.47	-NA-
N3	"	-NA-	**1164.50**	-NA-
N4	"	-NA-	**-141.60**	-NA-

Member Results

Member	Axial *lbs*	Vy *lbs*	Mz *lb-ft*	Dy *in*
M1	504.74	-37.25	-37.52	-0.0401
"	504.74	-15.75	28.6016	-0.0604
"	504.74	5.7472	41.1086	-0.0519
"	**504.74**	27.2472	0.0000	-0.0000
M2	504.74	-33.47	-46.64	-0.0000
"	504.74	-11.97	10.0140	-0.0213
"	504.74	9.5339	13.0543	-0.0358
"	504.74	31.0339	-37.52	-0.0401
M3	0.0000	-18.03	0.0000	0.0000
"	0.0000	-0.8263	18.7667	-0.0082
"	0.0000	16.3737	3.2194	-0.0043
"	0.0000	33.5737	-46.64	-0.0000
M4	-583.26	153.66	0.0000	-0.0000
"	-538.09	18.1643	225.51	-0.2143
"	-492.93	-117.34	94.8418	-0.1709
"	-447.76	**-252.84**	**-392.01**	-0.0402
M5	334.44	**252.84**	-392.01	-0.0402
"	379.61	117.34	94.8418	-0.1721
"	424.77	-18.16	225.51	**-0.2170**
"	469.94	-153.66	-0.0000	-0.0038
M6	364.52	-162.60	0.0000	0.0023
"	400.65	-54.20	227.96	-0.1759
"	436.78	54.2000	**227.96**	-0.1779
"	472.92	162.60	0.0000	-0.0038
M7	159.63	0.0000	0.0000	0.0054
"	159.63	0.0000	0.0000	0.0070
"	159.63	0.0000	0.0000	0.0086
"	159.63	0.0000	0.0000	**0.0102**
M8	68.2867	0.0000	0.0000	0.0051
"	68.2867	0.0000	0.0000	0.0059
"	68.2867	0.0000	0.0000	0.0067
"	68.2867	0.0000	0.0000	0.0075
M9	**-950.76**	0.0000	0.0000	-0.0355
"	-950.76	0.0000	0.0000	-0.0226
"	-950.76	0.0000	0.0000	-0.0097
"	-950.76	0.0000	0.0000	0.0032
M10	-598.19	0.0000	0.0000	-0.0102
"	-598.19	0.0000	0.0000	-0.0068
"	-598.19	0.0000	0.0000	-0.0034
"	-598.19	0.0000	0.0000	-0.0001
M11	-444.12	-0.0000	0.0000	-0.0046
"	-444.12	-0.0000	-0.0000	-0.0036
"	-444.12	-0.0000	-0.0000	-0.0027
"	-444.12	-0.0000	-0.0000	-0.0018


Y
X
M1
M2
M3
M4
M5
M6
M7
M8
M9
M10
M11
M12
M13

VisualAnalysis 3.50.c Report

09/10/01 10:55:23
Project: Truss 2
File: Untitled.Vap
Company: PK Associates Engineers
Engineer: Paul Zacher
Default Units: Feet, Pounds, Degrees, °Fahrenheit, Seconds.

Nodes

Node	X ft	Y ft	Fix DX	Fix DY	Fix RZ
N1	0.00	0.00	Yes	Yes	No
N2	7.50	1.25	No	No	"
N3	15.00	2.50	"	"	"
N4	15.00	0.00	"	Yes	"
N5	21.00	0.00	"	"	"
N6	7.50	2.50	"	No	"
N7	15.00	5.00	"	"	"
N8	21.00	3.00	"	"	"

Member Elements

Member	Section	Material	Length ft
M1	SS2x4	Wood	7.60
M2	"	"	7.60
M3	"	"	2.50
M4	"	"	6.00
M5	"	"	7.91
M6	"	"	7.91
M7	"	"	6.32
M8	"	"	2.50
M9	"	"	1.25
M10	"	"	7.50
M11	"	"	6.02
M12	"	"	3.00
M13	"	"	6.50

Section Properties

Category	Section	Ax in^2	Iz in^4	Sy+ in^3	Sy- in^3
Wood Sha	SS2x4	5.25	5.36	3.06	3.06

Material Properties

Material	Strength psi	Elasticity psi	Poisson	Density lb/ft^3
Wood	-NA-	1700000.00	0.36	40.47

Load Combination Summary

Equation Case: Equation Case 1
Combination: +1D+1L+1Lr
Contributing Cases & Source

Service Case 1 (Dead loads)
Service Case 2 (Roof Live loads)

Member Uniform Loads

This item is empty. Check the selection state, or report properties.

Nodal Reactions

Node	Load Case	FX *lbs*	FY *lbs*	MZ *lb-ft*
N1	Equation Case 1	**0.00**	348.62	-NA-
N4	"	-NA-	**1199.49**	-NA-
N5	"	-NA-	**-165.96**	-NA-

Member Results

Member	Axial *lbs*	Vy *lbs*	Mz *lb-ft*	Dy *in*
M1	926.47	29.4488	0.0000	-0.0000
"	930.06	7.9488	47.2555	-0.0991
"	933.64	-13.55	40.1558	-0.1462
"	937.22	-35.05	-21.30	-0.1498
M2	925.54	35.0512	-21.30	-0.1498
"	929.12	13.5512	40.1558	-0.1486
"	932.70	-7.9488	47.2555	-0.1039
"	936.29	-29.45	0.0000	-0.0072
M3	-1173.69	8.2923	-20.73	0.0200
"	-1173.69	8.2923	-13.82	0.0221
"	-1173.69	8.2923	-6.9102	0.0224
"	-1173.69	8.2923	0.0000	0.0218
M4	-8.2923	-25.80	0.0000	0.0000
"	-8.2923	-8.6000	34.3140	-0.0239
"	-8.2923	8.6000	34.3140	-0.0239
"	-8.2923	25.8000	0.0000	-0.0000
M5	-1020.78	157.14	0.0000	-0.0000
"	-975.62	21.6429	**234.68**	-0.2672
"	-930.45	-113.86	113.18	-0.2645
"	-885.28	**-249.36**	**-364.51**	-0.1506
M6	835.60	**249.36**	-364.51	-0.1506
"	880.77	113.86	113.18	-0.2669
"	925.93	-21.64	234.68	**-0.2721**
"	**971.10**	-157.14	-0.0000	-0.0072
M7	855.78	-162.60	0.0000	0.0047
"	891.91	-54.20	227.96	-0.1751
"	928.04	54.2000	227.96	-0.1786
"	964.18	162.60	0.0000	-0.0061
M8	-915.32	-8.2923	-0.0000	-0.0018
"	-915.32	-8.2923	6.9102	-0.0090
"	-915.32	-8.2923	13.8204	-0.0154
"	-915.32	-8.2923	20.7306	-0.0200
M9	71.0694	0.0000	0.0000	0.0340
"	71.0694	0.0000	0.0000	0.0354
"	71.0694	0.0000	0.0000	0.0367
"	71.0694	0.0000	0.0000	**0.0380**
M10	**-1790.28**	-0.0000	0.0000	-0.1461
"	-1790.28	-0.0000	-0.0000	-0.0987
"	-1790.28	-0.0000	-0.0000	-0.0513
"	-1790.28	-0.0000	-0.0000	-0.0039
M11	-866.27	-0.0000	0.0000	-0.0056
"	-866.27	-0.0000	-0.0000	-0.0038
"	-866.27	-0.0000	-0.0000	-0.0021

"	-866.27	-0.0000	-0.0000	-0.0003
M12	188.30	0.0000	0.0000	0.0125
"	188.30	0.0000	0.0000	0.0156
"	188.30	0.0000	0.0000	0.0186
"	188.30	0.0000	0.0000	0.0217
M13	8.9833	-0.0000	0.0000	0.0040
"	8.9833	-0.0000	-0.0000	0.0055
"	8.9833	-0.0000	-0.0000	0.0069
"	8.9833	-0.0000	-0.0000	0.0083

BENDING & COMP: TRUSS 2 - MEMBER 5
Design based on 1997 UBC 2321 Division V and ANSI/TPI 1-1995

Grading:
2x or 4x Doug-fir larch: No. 2
Assumptions:
Solid sheathing on top chord of truss. Therefore,
continuous lateral support is provided along compression face
Maximum center-center spacing = 24"

Width, b	1.5 inches	
Depth, d	3.5 inches	
Length	7.91 feet	
Max Axial Comp, C	885 feet	
Max Reaction, R	249 feet	
Max Moment, M	364 feet	
Max LL Deflection	0.07 feet	
Max TL Deflection	0.15 feet	
LL Defl Criteria = L/	240	
TL Defl Criteria = L/	180	
Duration factor, Cd	1.25	
Repetitive Factor, Cr	1.15	
Size Factor, Cf bending	1.5	1.5 for 2x4, 1.3 for 2x6
Size Factor, Cf comp	1.15	1.15 for 2x4, 1.1 for 2x6
Buckling Factor, CT =	1.22	
fc =	169 psi	
Fce=	844 psi	
Fc*=	2084 psi	
F'c=	758 psi	
fb=	1426 psi	
F'b=Fb*=	2156 psi	
Shear D/C ratio	0.60 < 1.0, Member OK	
Interaction equation:		
(fc/F'c)^2 +		
fb/ (F'b(1-fc/Fce)) =	0.88 < 1.0, Member OK	
Live Load defl ratio	0.18 < 1.0, Member OK	
Total Load defl ratio	0.28 < 1.0, Member OK	

BENDING & COMP: TRUSS 1 - MEMBER 4
Design based on 1997 UBC 2321 Division V and ANSI/TPI 1-1995

Grading:
2x or 4x Doug-fir larch: No. 2
Assumptions:
Solid sheathing on top chord of truss. Therefore,
continuous lateral support is provided along compression face
Maximum center-center spacing = 24"

Width, b	1.5	inches
Depth, d	3.5	inches
Length	7.91	feet
Max Axial Comp, C	447	feet
Max Reaction, R	252	feet
Max Moment, M	392	feet
Max LL Deflection	0.02	feet
Max TL Deflection	0.04	feet
LL Defl Criteria = L/	240	
TL Defl Criteria = L/	180	
Duration factor, Cd	1.25	
Repetitive Factor, Cr	1.15	
Size Factor, Cf bending	1.5	1.5 for 2x4, 1.3 for 2x6
Size Factor, Cf comp	1.15	1.15 for 2x4, 1.1 for 2x6
Buckling Factor, CT =	1.22	
fc =	85	psi
Fce=	844	psi
Fc*=	2084	psi
F'c=	758	psi
fb=	1536	psi
F'b=Fb*=	2156	psi
Shear D/C ratio	0.61 < 1.0, Member OK	
Interaction equation:		
(fc/F'c)^2 +		
fb/ (F'b(1-fc/Fce)) =	0.80 < 1.0, Member OK	
Live Load defl ratio	0.05 < 1.0, Member OK	
Total Load defl ratio	0.08 < 1.0, Member OK	

0111607

OWNER-BUILDER VERIFICATION

ATTENTION PROPERTY OWNERS

An owner-builder building permit has been applied for in your name and bearing your signature.

Please complete and return this information in the envelope provided at your earliest opportunity to avoid unnecessary delay in processing and issuing your building permit. No building permit will be issued until this verification is received.

1. I personally plan to provide the major labor and materials for construction of the proposed Improvement (yes or no) YES

2. I (have/have not) YES signed an application for A building permit for the proposed work.

3. I have contracted with the following person (firm) to provide the proposed construction:

   Name Rosalio Address 8765 Bedford Court

   City Sedo Ca 95828 Telephone 308 2171

   Contractors License No. _____

4. I plan to provide portions of the work, but I have hired the following person to coordinate, Supervise, and provide the major work.

   Name _____ Address _____

   City _____ Telephone _____

   Contractors License No. _____

5. I will provide some of the work but I have contracted (hired) the following to provide the Work indicated:

Name	Address	Phone	Type of work

Signed _____

Job Address 3023 Stonecreek Dr.

Permit No: 0111607