

CITY OF SACRAMENTO

1231 I Street, Sacramento, CA 95814

Permit No: 0112081

Insp Area: 2

Thos Bros: 337J6

Site Address: 4800 IDAHO DR SAC

Parcel No: 117-0533-001

Sub-Type: RES

Housing (Y/N): N

CONTRACTOR

V & M RESTORATION
2509 DEL MONTE ST
W SAC CA 95691

OWNER

VANG LEE
4800 IDAHO DR
SACRAMENTO CA 95823

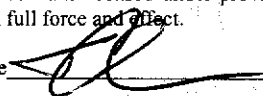
ARCHITECT

Nature of Work: FIRE REPAIR REPLACE ROOF FROM TOP
PLATE, PLUMBING, ELEC, DRYWALL, STUCCO

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

X License Class B License Number 718813 Date 9-19 X Contractor Signature 

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

____ I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

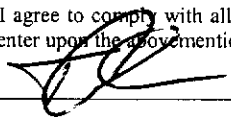
____ I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

____ I am exempt under Sec. _____ B & PC for this reason: _____

Date _____ Owner Signature _____

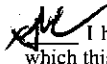
IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the aforementioned property for inspection purposes.

X Date 9-19 Applicant/Agent Signature 

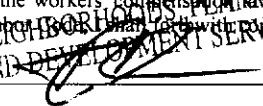
WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

____ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

X  I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier FIDELITY Policy Number DRE3835001 Exp Date 01/15/2002

____ (This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall comply with those provisions.

X Date 9-19 Applicant Signature 

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

PAID
CITY OF SACRAMENTO
SEP 19 2001
NEIGHBORHOODS, PLANNING AND DEVELOPMENT SERVICES

Sacramento Fire Department - Incident Report

Incident No : 010036344 Call# : 1081616 Date: 08/07/01 Time: 7:08
Address : 4800 IDAHO DR
Type : 11 BUILDING FIRE
Action Taken: 12 VENTILATION, EXTINGUISH, SALVAGE, OVERHAUL
Property : 1-2 FAMILY RESIDENTIAL: SINGLE FAMILY
UBC : DWELLINGS AND LODGING HOUSES

Weather : 80 Degrees / Clear
Resources : 2 Engines, 2 Trucks 2 Medics
1 Other Apparatus
1 Fire Rescue Unit

Fire Casualties : None

Fire Damage : Confined to structure of origin
Smoke Damage : Confined to structure of origin
Property Loss : \$100,000 Contents Loss : \$40,000
Property Value : \$140,000 Contents Value: \$50,000
Area of Origin : Insufficient information to classify
Caused by : No equipment involved
Form of Heat : Undetermined
Ignition Factor : Undetermined
Type of Material : Undetermined
Form of Material : Undetermined
Type of Material : Undetermined
Form of Material : Undetermined
Other Factors : Acts or Omissions Insufficient information
Extinguished by : Water from hydrant, draft, standpipe
Structure Type : Building with one specific property use
Structure Status : In use
Occupied
Construction Type: Type V - Wood Frame
Roof Type : Wood shake - untreated
Number of Stories: 1

Level: A01

Detector Type : Smoke detector - photoelectric
Power : Battery
Performance : Undetermined/not reported
Reason Failed : Undetermined/not reported

Extinguishing Sys: Undetermined/not reported
Performance : Undetermined/not reported
Reason Failed : Undetermined/not reported

Report Author : P751



ROOF TRUSS CALCULATIONS

9/12/01

**FERNANDEZ CONSTRUCTION
941 WASHINGTON BLVD, #316
ROSEVILLE, CA 95678**

**FIRE JOB
IDAHO DRIVE**



MiTek Industries, Inc.

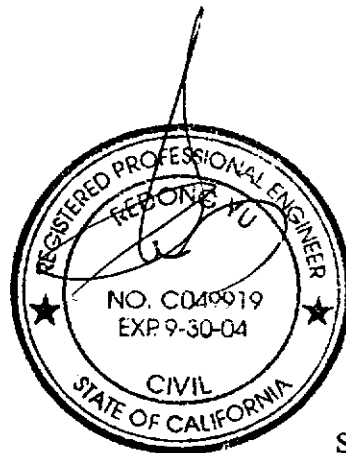
3033 GOLD CANAL DRIVE
SUITE 200
RANCHO CORDOVA CA 95670
USA
FAX (916) 631 8225
TELEPHONE (916) 631 7811

Re: F1-115
FERNANDEZ CONST

The truss drawing(s) referenced below have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Arden Lumber and Truss

Pages or sheets covered by this seal: R534312 thru R534318

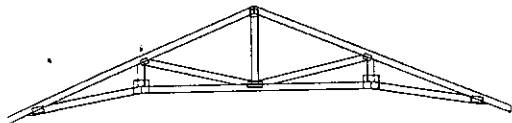
My license renewal date for the state of California is September 30, 2004.



September 12, 2001

Yu, Ray

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1995 Sec. 2.



ARDEN LUMBER

AND TRUSS

1260 FURNEAUX ROAD MARYSVILLE, CA 95901
(530) 749-6500 FAX (530) 749-6505

DATE 8/21/01 INVOICE # _____ SALESMAN M. FREED

NAME FERNANDEZ CONST. PHONE _____

ADDRESS ROOF FRAMING PLAN CITY _____

JOB IDAHO FIRE JOB PLAN SFD ELEV ROOF LOT _____

SLOPE 6:12

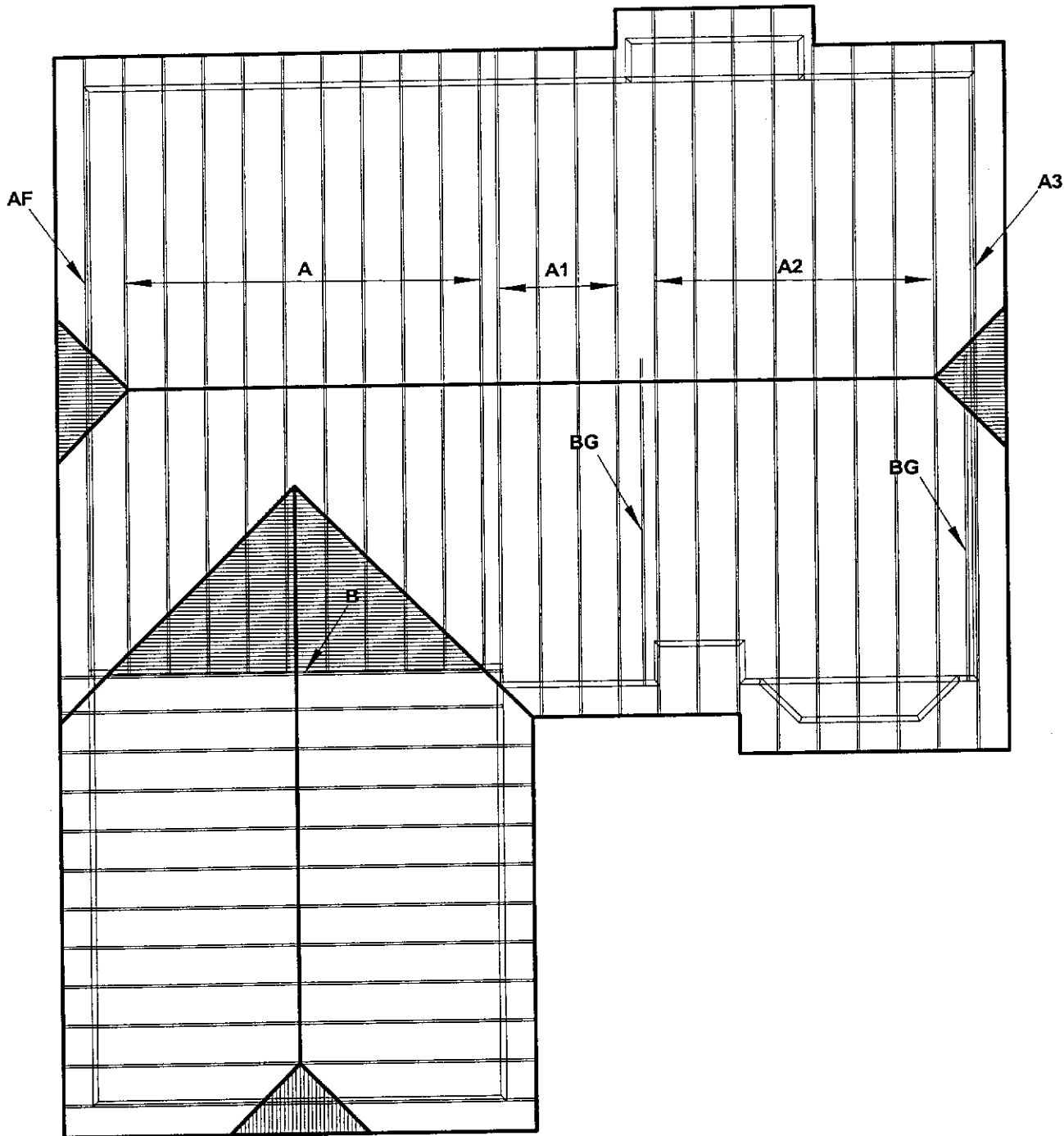
DEPTH _____

SEAT CUT 3 1/2

T/C SIZE 2x4

B/C SIZE 2x4

SHAKE COMP TILE



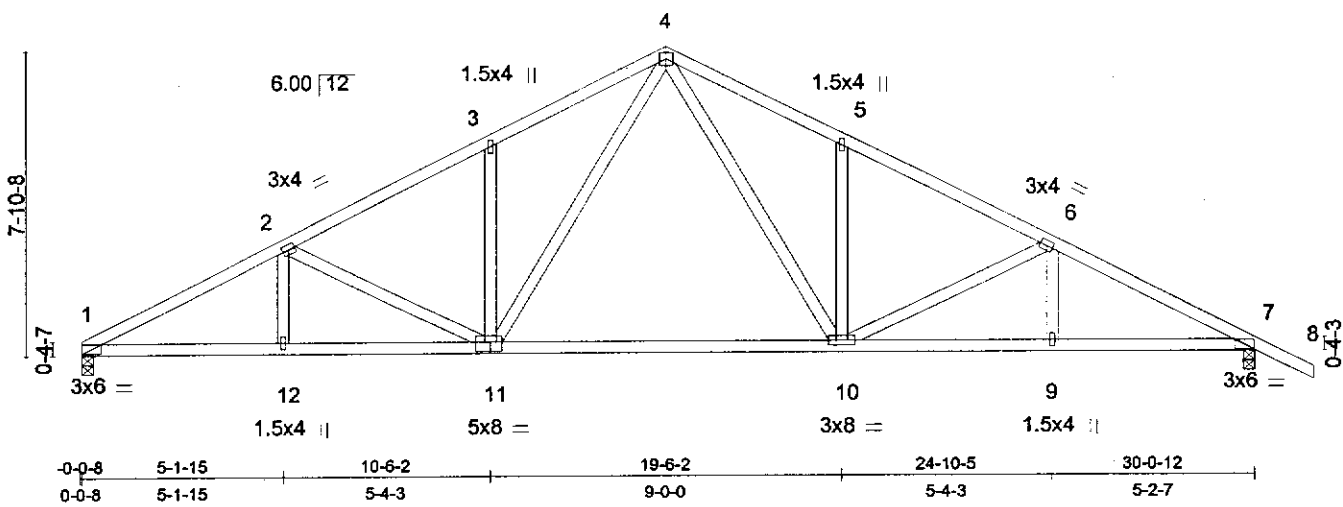
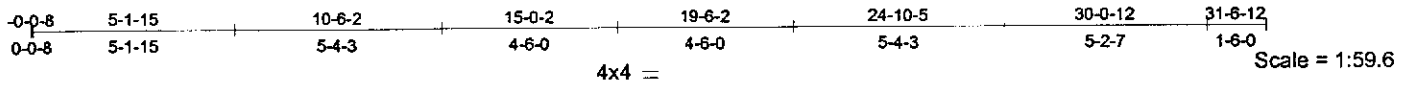


Plate Offsets (X,Y): [11:0-2-12,0-3-0]					
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.27	in (loc) l/defl	MII20	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.90	Vert(LL) -0.19 10-11 >999		
BCLL 0.0	Rep Stress Incr NO	WB 0.31	Vert(TL) -0.55 10-11 >651		
BCDL 7.0	Code UBC97/ANSI95		Horz(TL) 0.06 7 n/a		
			1st LC LL Min l/defl = 360	Weight: 147 lb	

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr-G
BOT CHORD 2 X 4 DF No.1&Btr-G
WEBS 2 X 4 DF Std-G

BRACING
TOP CHORD Sheathed or 4-3-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1 = 1192/0-3-8, 7 = 1290/0-3-8

FORCES (lb) - First Load Case Only
TOP CHORD 1-2 = -2160, 2-3 = -1762, 3-4 = -1762, 4-5 = -1765, 5-6 = -1765, 6-7 = -2170, 7-8 = 20
BOT CHORD 1-12 = 1912, 11-12 = 1912, 10-11 = 1189, 9-10 = 1924, 7-9 = 1924
WEBS 3-11 = -295, 5-10 = -295, 4-11 = 758, 4-10 = 764, 2-12 = 73, 6-9 = 73, 2-11 = -374, 6-10 = -385

- NOTES (7)**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) This truss has been designed for the wind loads generated by 0 mph winds at 25 ft above ground level, using 7.0 psf top chord dead load and 7.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 3) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - 5) A plate rating reduction of 20% has been applied for the green lumber members.
 - 6) This truss has been designed with ANSI/TPI 1-1995 criteria.
 - 7) HVAC mechanical load applied in bottom chord panel(s) indicated in load case(s).

LOAD CASE(S) Standard
1) Regular: Lumber Increase = 1.25, Plate Increase = 1.25
Uniform Loads (plf)
Vert: 1-11 = -14.0, 10-11 = -34.0, 7-10 = -14.0, 1-4 = -60.0, 4-8 = -60.0



September 12, 2001

WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installation and Bracing Recommendation available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53718

MiTek Industries, Inc.

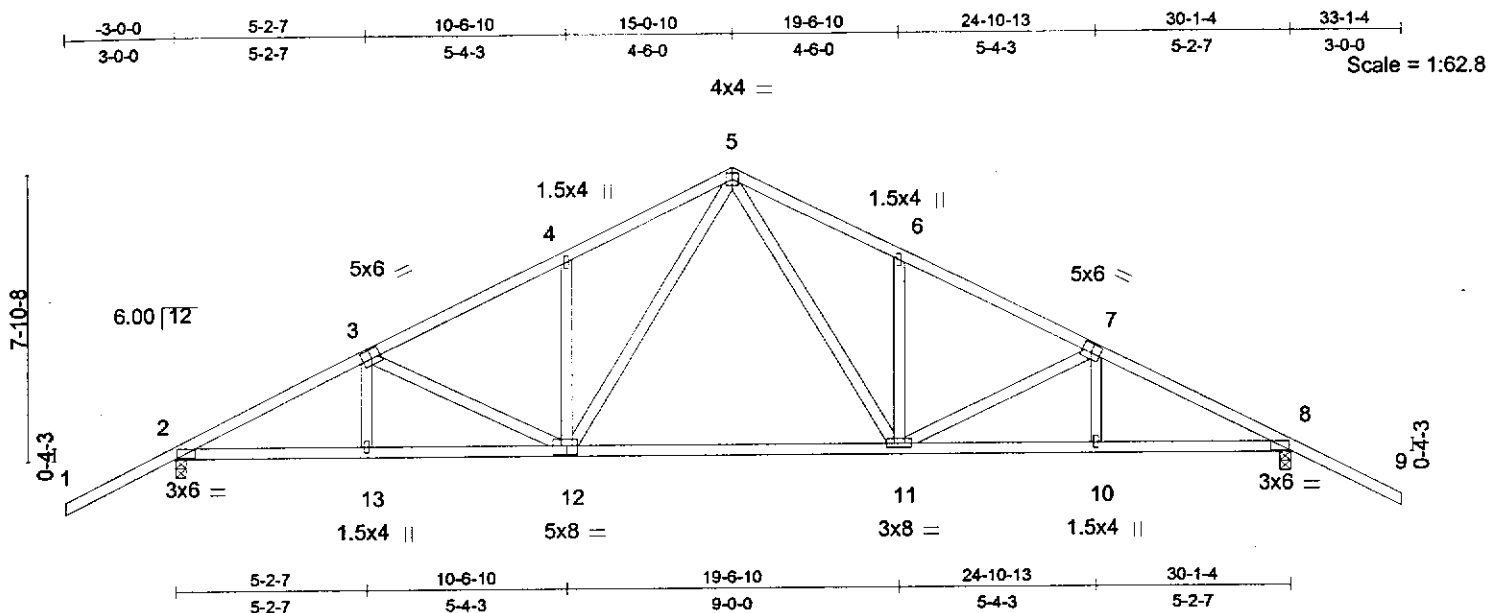


Plate Offsets (X,Y): [2:0-2-12,0-1-8], [3:0-3-0,0-3-0], [7:0-3-0,0-3-0], [8:0-2-12,0-1-8], [12:0-2-12,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.51	in (loc) l/defl	MII20	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.90	Vert(LL) -0.19 11-12 >999		
BCLL 0.0	Rep Stress Incr NO	WB 0.31	Vert(TL) -0.55 11-12 >651		
BCDL 7.0	Code UBC97/ANSI95		Horz(TL) 0.06 8 n/a		
			1st LC LL Min l/defl = 360	Weight: 154 lb	

LUMBER
 TOP CHORD 2 X 4 DF No.1&Btr-G
 BOT CHORD 2 X 4 DF No.1&Btr-G
 WEBS 2 X 4 DF Std-G

BRACING
 TOP CHORD Sheathed or 4-3-9 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2 = 1382/0-3-8, 8 = 1382/0-3-8

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=40, 2-3=-2170, 3-4=-1769, 4-5=-1769, 5-6=-1769, 6-7=-1769, 7-8=-2170, 8-9=40
 BOT CHORD 2-13=1924, 12-13=1922, 11-12=1192, 10-11=1922, 8-10=1924
 WEBS 4-12=-293, 6-11=-293, 5-12=763, 5-11=763, 3-13=73, 7-10=73, 3-12=-381, 7-11=-381

- NOTES (7)**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) This truss has been designed for the wind loads generated by 0 mph winds at 25 ft above ground level, using 7.0 psf top chord dead load and 7.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 3) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - 5) A plate rating reduction of 20% has been applied for the green lumber members.
 - 6) This truss has been designed with ANSI/TPI 1-1995 criteria.
 - 7) HVAC mechanical load applied in bottom chord panel(s) indicated in load case(s).

LOAD CASE(S) Standard
 1) Regular: Lumber Increase = 1.25, Plate Increase = 1.25
 Uniform Loads (plf)
 Vert: 2-12=-14.0, 11-12=-34.0, 8-11=-14.0, 1-5=-60.0, 5-9=-60.0



September 12, 2001

WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installation and Bracing Recommendation available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



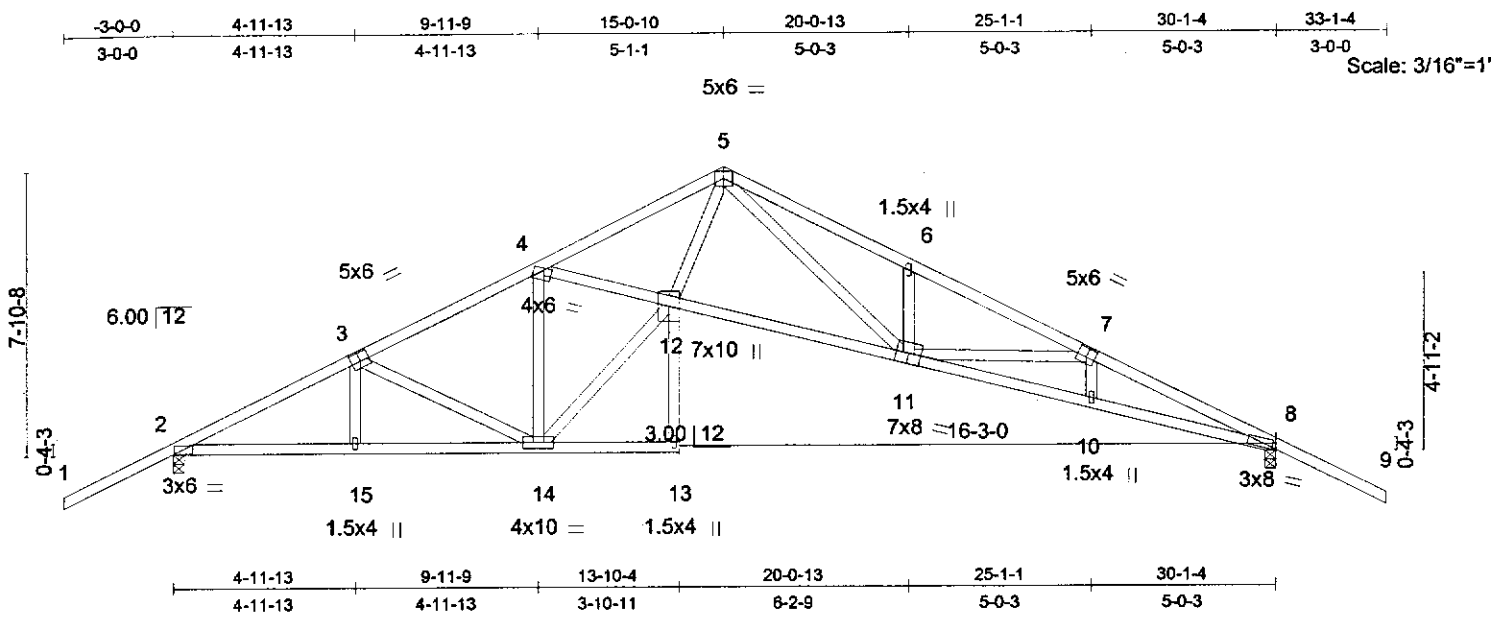


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [4:0-2-12,0-0-12], [7:0-3-0,0-3-0]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl
TCLL 16.0	Plates Increase	1.25	TC 0.58	Vert(LL)	-0.20 11	>999
TCCL 14.0	Lumber Increase	1.25	BC 0.60	Vert(TL)	-0.48 11-12	>749
BCLL 0.0	Rep Stress Incr	NO	WB 0.86	Horz(TL)	0.32 8	n/a
BCCL 7.0	Code	UBC97/ANSI95		1st LC LL Min l/defl	= 360	
				PLATES		GRIP
				M120		220/195
				Weight: 156 lb		

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr-G
BOT CHORD 2 X 4 DF No.1&Btr-G *Except*
12-13 2 X 4 DF Std-G
WEBS 2 X 4 DF Std-G

BRACING
TOP CHORD Sheathed or 3-0-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1292/0-3-8, 8=1292/0-3-8

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=40, 2-3=-1990, 3-4=-1618, 4-5=-3127, 5-6=-3109, 6-7=-3114, 7-8=-3734, 8-9=39
BOT CHORD 2-15=1764, 14-15=1762, 13-14=-0, 12-13=26, 4-12=1396, 11-12=2178, 10-11=3398, 8-10=3402
WEBS 5-12=1925, 5-11=958, 7-11=-512, 4-14=-1331, 12-14=2126, 3-15=69, 3-14=-355, 6-11=-297, 7-10=71

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) This truss has been designed for the wind loads generated by 0 mph winds at 25 ft above ground level, using 7.0 psf top chord dead load and 7.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category 1, condition 1 enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 3) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - 5) A plate rating reduction of 20% has been applied for the green lumber members.
 - 6) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 7) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

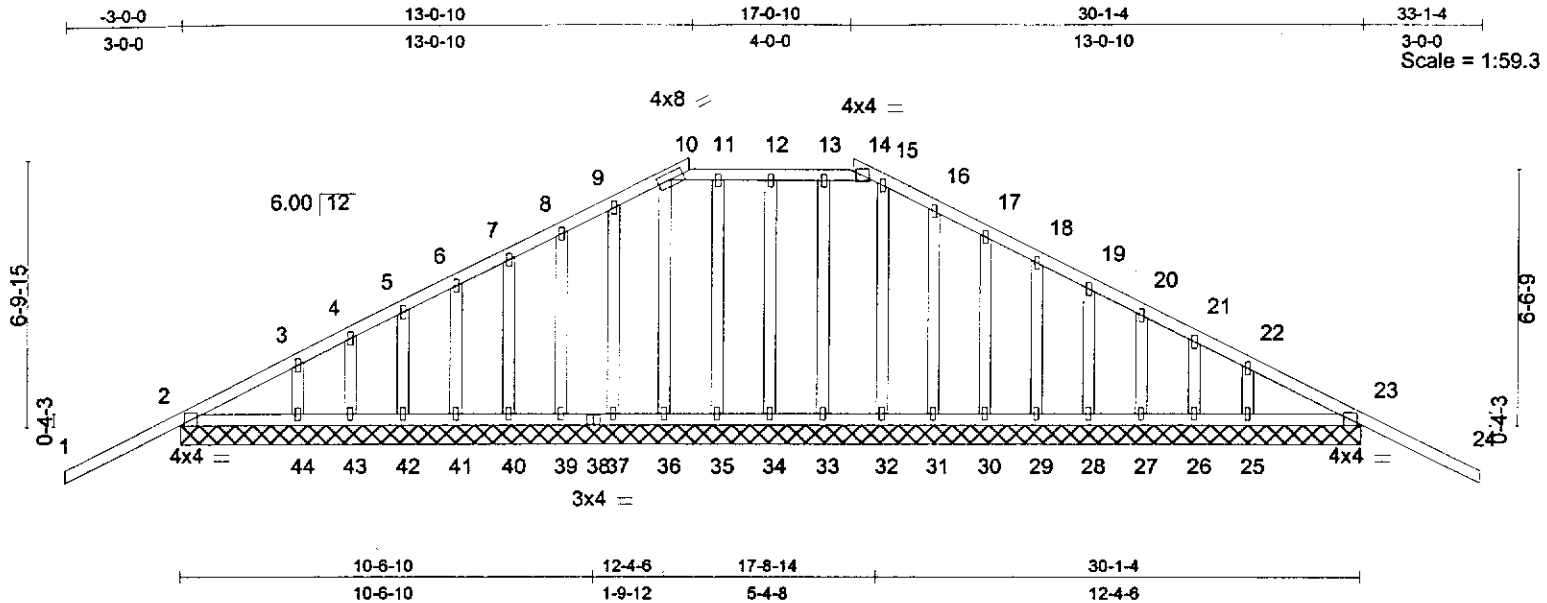


September 12, 2001

WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installation and Bracing Recommendation available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

MiTek Industries, Inc.



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 16.0	2-0-0	TC 0.48	in (loc) l/defl	MII20	220/195
TCDL 14.0	Plates Increase 1.25	BC 0.22	Vert(LL) n/a - n/a		
BCLL 0.0	Lumber Increase 1.25	WB 0.06	Vert(TL) 0.15 1-2 >236		
BCDL 7.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.00 23 n/a		
	Code UBC97/ANSI95		1st LC LL Min l/defl = 360	Weight: 199 lb	

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr-G
BOT CHORD 2 X 4 DF No.1&Btr-G
WEBS 2 X 4 DF Std-G
OTHERS 2 X 4 DF Std-G

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=394/30-1-4, 38=5/30-1-4, 33=106/30-1-4, 34=98/30-1-4, 35=85/30-1-4, 37=109/30-1-4, 39=94/30-1-4, 40=98/30-1-4, 41=102/30-1-4, 42=86/30-1-4, 43=149/30-1-4, 44=23/30-1-4, 32=102/30-1-4, 31=98/30-1-4, 30=99/30-1-4, 29=98/30-1-4, 28=102/30-1-4, 27=85/30-1-4, 26=156/30-1-4, 25=3/30-1-4, 23=398/30-1-4, 36=99/30-1-4

Max Grav 2=394(load case 1), 38=12(load case 2), 33=106(load case 7), 34=99(load case 7), 35=86(load case 7), 37=109(load case 6), 39=94(load case 1), 40=98(load case 1), 41=102(load case 6), 42=86(load case 1), 43=149(load case 6), 44=98(load case 2), 32=102(load case 7), 31=98(load case 1), 29=98(load case 1), 29=98(load case 1), 28=102(load case 7), 27=85(load case 1), 26=156(load case 7), 25=83(load case 2), 23=398(load case 1), 36=99(load case 6)

FORCES (lb) - First Load Case Only
TOP CHORD 10-11=-10, 11-12=-10, 12-13=-10, 13-14=-10, 1-2=79, 2-3=-69, 3-4=17, 4-5=-24, 5-6=13, 6-7=13, 7-8=13, 8-9=14, 9-10=14, 14-15=-29, 15-16=-28, 16-17=-29, 17-18=-29, 18-19=-29, 19-20=-28, 20-21=-30, 21-22=-19, 22-23=-74, 23-24=79

BOT CHORD 2-44=4, 43-44=4, 42-43=4, 41-42=4, 40-41=4, 39-40=4, 38-39=4, 37-38=4, 36-37=4, 35-36=10, 34-35=10, 33-34=10, 32-33=10, 31-32=10, 30-31=10, 29-30=10, 28-29=10, 27-28=10, 26-27=10, 25-26=10, 23-25=10

WEBS 13-33=-85, 12-34=-79, 11-35=-66, 9-37=-94, 8-39=-77, 7-40=-80, 6-41=-80, 5-42=-78, 4-43=-90, 3-44=-74, 15-32=-81, 16-31=-79, 17-30=-80, 18-29=-80, 19-28=-80, 20-27=-77, 21-26=-95, 22-25=-61, 10-36=-80

- NOTES**
- This truss has been checked for unbalanced loading conditions.
 - This truss has been designed for the wind loads generated by 0 mph winds at 25 ft above ground level, using 7.0 psf top chord dead load and 7.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
 - Provide adequate drainage to prevent water ponding.
 - As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
 - All plates are 1.5x4 MII20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 1-4-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - A plate rating reduction of 20% has been applied for the green lumber members.
 - This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard

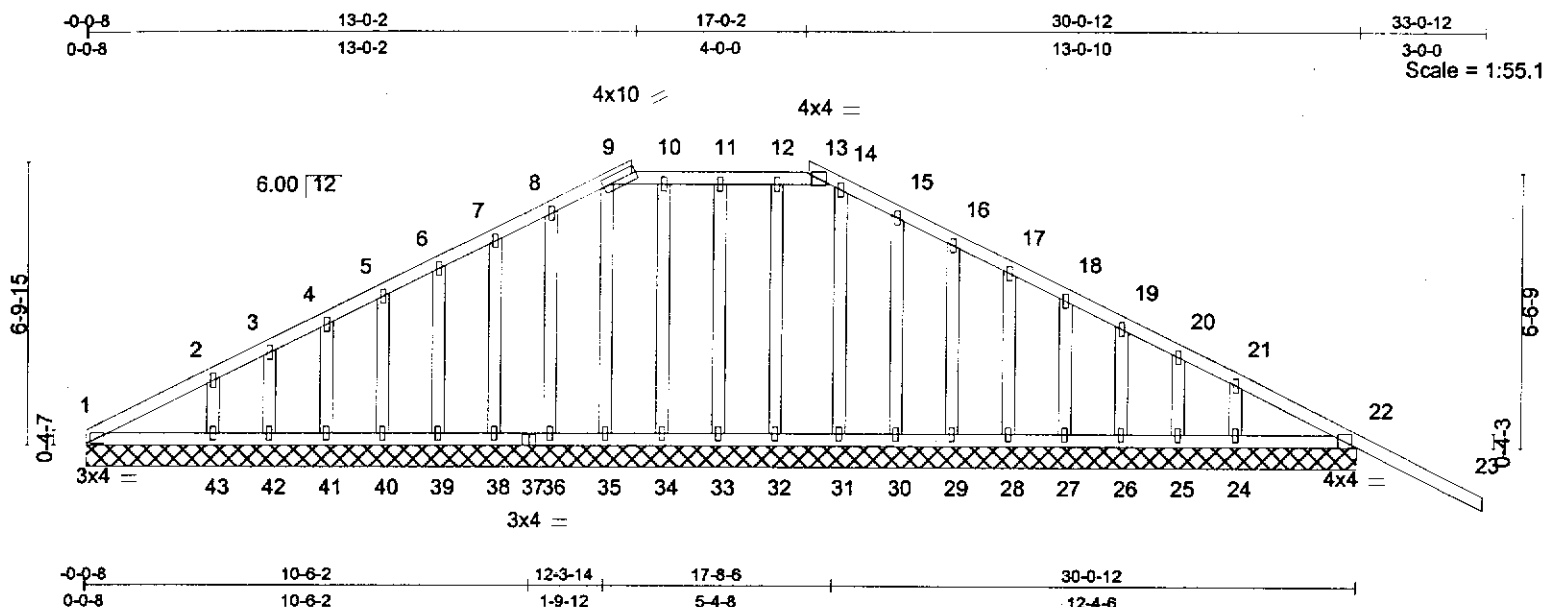


September 12, 2001

WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installation and Bracing Recommendation available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

MII
MiTek Industries, Inc.



LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	16.0	Plates Increase	2-0-0	TC	0.48	Vert(LL)	n/a	MII20		220/195	
TCDL	14.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	0.15 22-23 >238				
BCLL	0.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00 22 n/a				
BCDL	7.0	Code	UBC97/ANSI95	(Matrix)		1st LC LL Min l/defl	= 360			Weight: 194 lb	

LUMBER
 TOP CHORD 2 X 4 DF No.1 & Btr-G
 BOT CHORD 2 X 4 DF No.1 & Btr-G
 OTHERS 2 X 4 DF Std-G

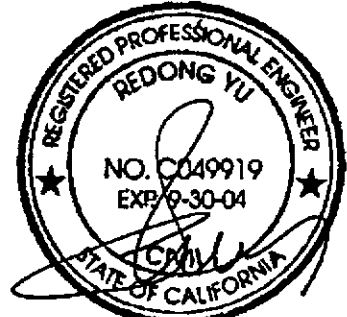
BRACING
 TOP CHORD Sheathed or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=95/30-0-12, 37=5/30-0-12, 32=105/30-0-12, 33=98/30-0-12, 34=96/30-0-12, 35=96/30-0-12, 36=97/30-0-12, 38=96/30-0-12, 39=100/30-0-12, 40=96/30-0-12, 41=111/30-0-12, 42=42/30-0-12, 43=228/30-0-12, 31=102/30-0-12, 30=98/30-0-12, 29=99/30-0-12, 28=98/30-0-12, 27=102/30-0-12, 26=85/30-0-12, 25=156/30-0-12, 24=3/30-0-12, 22=398/30-0-12
 Max Grav 1=95(load case 1), 37=11(load case 2), 32=105(load case 7), 33=99(load case 6), 34=97(load case 7), 35=96(load case 6), 36=97(load case 6), 38=96(load case 1), 39=100(load case 6), 40=96(load case 6), 41=111(load case 1), 42=42(load case 6), 43=228(load case 6), 31=103(load case 7), 30=98(load case 7), 29=99(load case 1), 28=98(load case 7), 27=102(load case 7), 26=85(load case 1), 25=156(load case 7), 24=83(load case 2), 22=398(load case 1)

FORCES (lb) - First Load Case Only
TOP CHORD 9-10=-9, 10-11=-9, 11-12=-9, 12-13=-9, 1-2=36, 2-3=-40, 3-4=12, 4-5=-27, 5-6=9, 6-7=9, 7-8=9, 8-9=10, 13-14=-28, 14-15=-27, 15-16=-27, 16-17=-27, 17-18=-27, 18-19=-27, 19-20=-28, 20-21=-18, 21-22=-73, 22-23=79
BOT CHORD 1-43=8, 42-43=8, 41-42=8, 40-41=8, 39-40=8, 38-39=8, 37-38=8, 36-37=8, 35-36=8, 34-35=9, 33-34=9, 32-33=9, 31-32=9, 30-31=9, 29-30=9, 28-29=9, 27-28=9, 26-27=9, 25-26=9, 24-25=9, 22-24=9
WEBS 12-32=-85, 11-33=-80, 10-34=-78, 9-35=-76, 8-36=-82, 7-38=-80, 6-39=-80, 5-40=-79, 4-41=-86, 3-42=-45, 2-43=-166, 14-31=-82, 15-30=-79, 16-29=-80, 17-28=-80, 18-27=-80, 19-26=-77, 20-25=-95, 21-24=-61

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) This truss has been designed for the wind loads generated by 0 mph winds at 25 ft above ground level, using 7.0 psf top chord dead load and 7.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
 - 4) Provide adequate drainage to prevent water ponding.
 - 5) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
 - 6) All plates are 1.5x4 MII20 unless otherwise indicated.
 - 7) Gable requires continuous bottom chord bearing.
 - 8) Gable studs spaced at 1-4-0 oc.
 - 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - 10) A plate rating reduction of 20% has been applied for the green lumber members.
 - 11) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



September 12, 2001

WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult **QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installation and Bracing Recommendation** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

MiTek Industries, Inc.

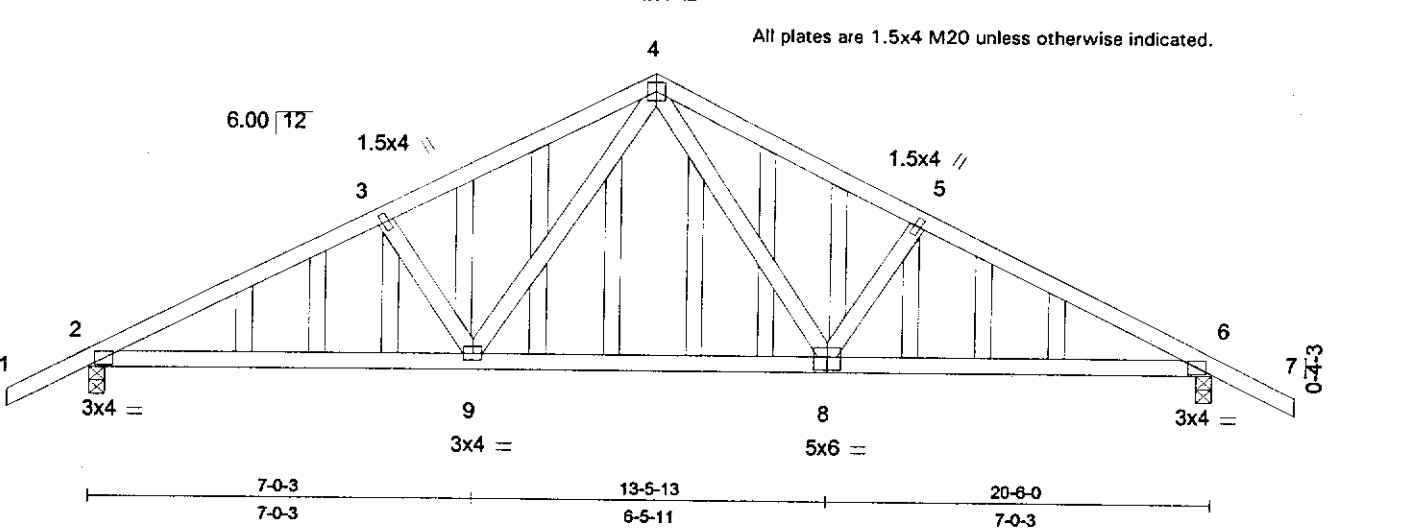
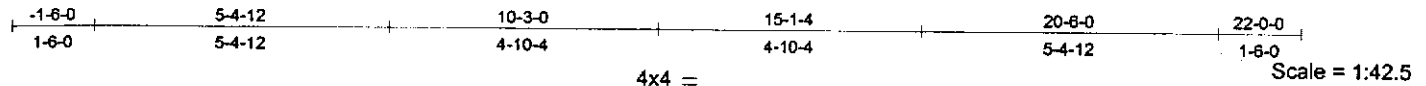


Plate Offsets (X,Y): [8:0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.26	Vert(LL) -0.06	8-9	>999	MII20	220/195
TCDL 14.0	Lumber Increase	1.25	BC 0.43	Vert(TL) -0.12	8-9	>999		
BCLL 0.0	Rep Stress Incr	NO	WB 0.15	Horz(TL) 0.02	6	n/a		
BCDL 7.0	Code	UBC97/ANSI95		1st LC LL Min l/defl = 360			Weight: 130 lb	

LUMBER
TOP CHORD 2 X 4 DF No.1 & Btr-G
BOT CHORD 2 X 4 DF No.1 & Btr-G
WEBS 2 X 4 DF Std-G
OTHERS 2 X 4 DF Std-G

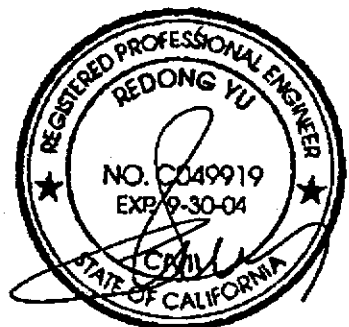
BRACING
TOP CHORD Sheathed or 5-10-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2 = 846/0-3-8, 6 = 846/0-3-8

FORCES (lb) - First Load Case Only
TOP CHORD 1-2 = 20, 2-3 = -1178, 3-4 = -1017, 4-5 = -1017, 5-6 = -1178, 6-7 = 20
BOT CHORD 2-9 = 1044, 8-9 = 717, 6-8 = 1044
WEBS 3-9 = -252, 4-9 = 362, 4-8 = 362, 5-8 = -252

- NOTES**
- 1) This truss has been checked for unbalanced loading conditions.
 - 2) This truss has been designed for the wind loads generated by 0 mph winds at 25 ft above ground level, using 7.0 psf top chord dead load and 7.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33 and the plate grip increase is 1.33
 - 3) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - 5) A plate rating reduction of 20% has been applied for the green lumber members.
 - 6) This truss has been designed with ANSI/TPI 1-1995 criteria.

LOAD CASE(S) Standard



September 12, 2001

WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult QST-88 Quality Standard, DSB-88 Bracing Specification, and HIB-91 Handling Installation and Bracing Recommendation available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

