

**CITY OF SACRAMENTO**  
1231 I Street, Sacramento, CA 95814

**Permit No: 0107203**

**Insp Area: 3**

Thos Bros:

Sub-Type: REM

Housing (Y/N): N

**Site Address: 3729 62ND ST SAC**

Parcel No 015-0286-017

CONTRACTOR

OWNER

MEZZANARI'S TOMMY J VANESSA E  
3729 62ND ST  
SACRAMENTO CA 95820

ARCHITECT

**Nature of Work: REMODEL AND INCREASING GARAGE SQ FOOTAGE**

**CONSTRUCTION LENDING AGENCY:** I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C.).

Lender's Name \_\_\_\_\_ Lender's Address \_\_\_\_\_

**LICENSED CONTRACTORS DECLARATION:** I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class \_\_\_\_\_ License Number \_\_\_\_\_ Date \_\_\_\_\_ Contractor Signature \_\_\_\_\_

**OWNER-BUILDER DECLARATION:** I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code, any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 3 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00).

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code). The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.

**CITY OF SACRAMENTO**

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code). The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law.

PAID  
AUG 10 2001

I am exempt under Sec. \_\_\_\_\_ B & P for this reason: \_\_\_\_\_

Date 8/6/01

Owner Signature

*Tom Mezzanari*

**NEIGHBORHOODS PLANNING AND DEVELOPMENT SERVICES**

**IN ISSUING THIS BUILDING PERMIT,** the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the above mentioned property for inspection purposes.

Date 8/6/01

Applicant Agent Signature

*Tom Mezzanari*

**WORKER'S COMPENSATION DECLARATION:** I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier Exempt

Policy Number

Exp Date

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 8/6/01

Applicant Signature

*Tom Mezzanari*

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

**THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.**

Date of Request: 5/4/01  
By: \_\_\_\_\_

# CITY OF SACRAMENTO DEVELOPMENT SERVICES DIVISION PLANNING AND ZONING INFORMATION REQUEST

Project Address: 3729 62<sup>nd</sup> Street  
Assessor's Parcel Number: ~~XXXXXXXXXX~~ 015-0286-017

Previous Use: Garage (same)  
Description of Request/Proposed Use: ~~Expand~~ EXISTING ONE CAR  
(detached) garage & Build Two CAR (detached)  
garage.

Is This a Change of Use? No

Zoning Designation: Residential (R1)

Prior Applications for Project Site (P#, Z#, DRPB#): \_\_\_\_\_

Comments: LOT SQ FT 6160  
~~XXXXXXXXXX~~ 1<sup>st</sup> FUR 775 GAR 228  
PREV. ADDITION 309 CURRENT ADDN 345 TOTAL 1557

LOT COV 25% 9' GARAGE MUST BE AT  
LEAST 60' FROM FRONT PROPERTY LINE IF SO, NO  
Are There Any Planning Issues?: (circle one) YES  NO   
INTERIOR SIDE  
YARD SETBACK  
REQUIRED  
EXPANSION OF  
CURRENT GARAGE  
FROM 228 SQ FT  
TO 573 SQ FT  
NO OTHER WORK

- \* Staff Site Plan Check Required? (Circle one)
- \* Field Inspection Required? (Circle one)
- \* Design Review/Preservation Required? (Circle one)

Planning Review by/Date: [Signature]

A list of items that must be reviewed by Planning is provided on the reverse side of this form.

MICROFILM AFTER FINAL

OWNER-BUILDER VERIFICATION

ATTENTION PROPERTY OWNERS

An owner-builder building permit has been applied for in your name and bearing your signature.

Please complete and return this information in the envelope provided at your earliest opportunity to avoid unnecessary delay in processing and issuing your building permit. No building permit will be issued until this verification is received.

1. I personally plan to provide the major labor and materials for construction of the proposed Improvement (yes or no) yes
2. I (have/have not) have signed an application for A building permit for the proposed work.

3. I have contracted with the following person (firm) to provide the proposed construction:

Name \_\_\_\_\_ Address \_\_\_\_\_

City \_\_\_\_\_ Telephone \_\_\_\_\_

Contractors License No. \_\_\_\_\_

4. I plan to provide portions of the work, but I have hired the following person to coordinate, Supervise, and provide the major work.

Name \_\_\_\_\_ Address \_\_\_\_\_

City \_\_\_\_\_ Telephone \_\_\_\_\_

Contractors License No. \_\_\_\_\_

5. I will provide some of the work but I have contracted (hired) the following to provide the Work indicated:

Name	Address	Phone	Type of work

X Signed [Signature]

X Job Address 3729 62 STREET

X Date 8/6/01

Permit No: \_\_\_\_\_

**Meznanares Garage**

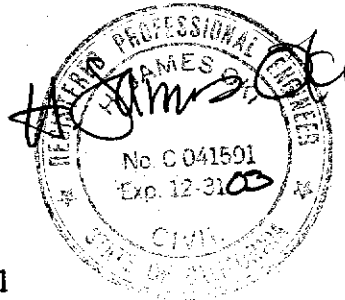
**3729 62<sup>nd</sup> Street**

**Sacramento, California**

MICROFILM AT FINAL

Calculation Prepared by:

H. James Oki Engineering  
542 Messick Road  
Yuba City, California 95991



 plan check 7.9.01

# **Mezzanares Garage**

**Sacramento, CA**

---

<i>Design Loads</i>	<b>1</b>
<i>Roof Framing</i>	<b>3</b>
<i>Foundation</i>	<b>4</b>
<i>Lateral Analysis</i>	<b>5</b>

H. JAMES OKI ENGINEERING Consulting Engineer Yuba City, California 95991 (530) 674-7755	project Mezzanares Garage Sacramento, CA	
	by HJO	no.
	date 6/5/01	1

Design Loads

△ plan check  
7.9.01

DESIGN LOADS (Vertical)

<u>Roof</u>	Roofing	(Asphalt Roof)	4.0	
	Sheathing	(5/8" SP)	1.8	
	Framing	(2x4 @ 24" oc)	<del>2.0</del> 3.5	
	Insulation	<del>Truss</del>	1.5	
	Mech, Elec		1.5	
	Misc	(Mech,Elect,etc)	1.2	
		DL		<u>12.0 psf</u>
	LL		16.0 psf	(7:12)
<u>Wall</u>	Ext Finish	(Stucco)	10.0	
	Int Finish	(Gyp Board)	2.2	
	Framing	(2x6 @ 16" oc)	1.7	
	Insulation		0.5	
	Misc	(Mech,Elect,etc)	0.6	
		DL		<u>15.0 psf</u>

△  
14  
12.0 psf  
16.0 psf

H. JAMES OKI ENGINEERING Consulting Engineer Yuba City, California 95991 (530) 674-7755	project Mezzanares Garage Sacramento, CA	no. 2
	by HJO	
	date 6/5/01	

Design Loads

DESIGN LOADS (Lateral)

Wind (Method 2)  $P=C_e C_q q_s I$   $C_q = 1.3$   $I = 1.00$

Speed: 75 mph  $q_s = 14.5$  Exposure: C

(0'-15')	$P = (1.06)(1.3)(14.5)(1) =$	20.0 psf
(15'-20')	$P = (1.13)(1.3)(14.5)(1) =$	21.3 psf
(20'-25')	$P = (1.19)(1.3)(14.5)(1) =$	22.4 psf
(25'-30')	$P = (1.23)(1.3)(14.5)(1) =$	23.2 psf
(30'-40')	$P = (1.31)(1.3)(14.5)(1) =$	24.7 psf
(40'-60')	$P = (1.43)(1.3)(14.5)(1) =$	27.0 psf

Seismic - 97

(1997 UBC)

$$V = \frac{2.5 \times C_a \times I}{R} W$$

Zone = 3.0  $Z = 0.30$   
 $R = 5.5$   $I = 1.00$

$V = 0.164 W$  Soil type = SD  $C_a = 0.36$

$\rho = 2 - \frac{20}{r_{max} (A_B)^{1/2}}$   $A_B = 560 \text{ ft}^2$   $r_{max} = 0.79$  (See p. 6)

$\rho = 0.93$  Use 1.0

$E_h = V / 1.4 = 0.117 W$  (Service Load Base Shear)

$E = \rho E_h + E_v$   $E_v = 0$  (Working Stress Design)

$= 0.117 W$

DESIGN CRITERIA

Code: 1997 Uniform Building Code

Soils Report: 1997 Uniform Building Code

Report: Table 18-I-A Dated: 1/1/98  
Allowable bearing: D+L = 1000 psf  
D+L+Lateral = 1333 psf


H. JAMES OKI ENGINEERING  
 Consulting Engineer  
 Yuba City, California 95991 (530) 674-7755

project Mezzanares Garage  
 Sacramento, CA

by HJO  
 date 6/5/01

no.  
 3

Roof Framing

 PLAN CHECK  
 7.9.01

Trusses 

$L = 14.0 \text{ ft}$

$w = (14D + 16L)(2') = 60 \text{ plf}$

$V = w \times L / 2 = 420 \text{ lb}$

Check:  $L / 240$

$M = w \times L^2 / 8 = 1470 \text{ ft-lb}$

$\Delta_A = 0.7 \text{ in}$

$EI_{req} = \frac{5wL^4 (12"/1')^3}{384 \Delta} = 74 \times 10^6 \text{ in}^2\text{-lb}$

Use Prefab Wd Truss at 24" cc

Header @ Garage Door

$L = 11.5 \text{ ft} \quad C_D = 1.25$

$C_F = 1.10$

$95 \text{ plf}$

$w = (14D + 16L)(2') + (15W)(1') + 20 =$

$V = wL/2 = 546 \text{ lb}$

$M = \frac{wL^2}{8} = 1570 \text{ ft-lb}$

$A_{req} = \frac{3V}{2F_v C_D} = 6.9 \text{ in}^2$

DF No. 2

$F_v = 95 \quad \text{Check: } L / 240$

$S_{req} = \frac{M(12"/1')}{F_b C_D C_F} = 15.7 \text{ in}^3$

$F_b = 875 \quad \Delta_A = 0.58 \text{ in}$

$E = 1,600,000$

$I_{req} = \frac{5 w L^4}{384 E \Delta} = 40.6 \text{ in}^4$

Use 4x12 DF No. 2

$A = 39.4, S = 73.8, I = 415.3$



H. JAMES OKI ENGINEERING Consulting Engineer Yuba City, California 95991 (530) 674-7755	project <i>Mezzanares Garage</i> <i>Sacramento, CA</i>	
	by <i>HJO</i>	no. <i>4</i>
	date <i>6/5/01</i>	

**Foundation**

**Exterior Ftg**

*(Uniform Load)*

$$w = (14D + 16L)(7') + (15W)(8') =$$

$$W_{REQ} = \frac{330 \text{ plf}}{1000 \text{ psf}} = 0.33 \text{ ft}$$

*330 plf*

<i>Use 12" wide x 12" deep</i>
<i>Continuous Footing</i>

H. JAMES OKI ENGINEERING  
Consulting Engineer  
Yuba City, California 95991 (530) 674-7755

project Mezzanares Garage  
Sacramento, CA

by HJO

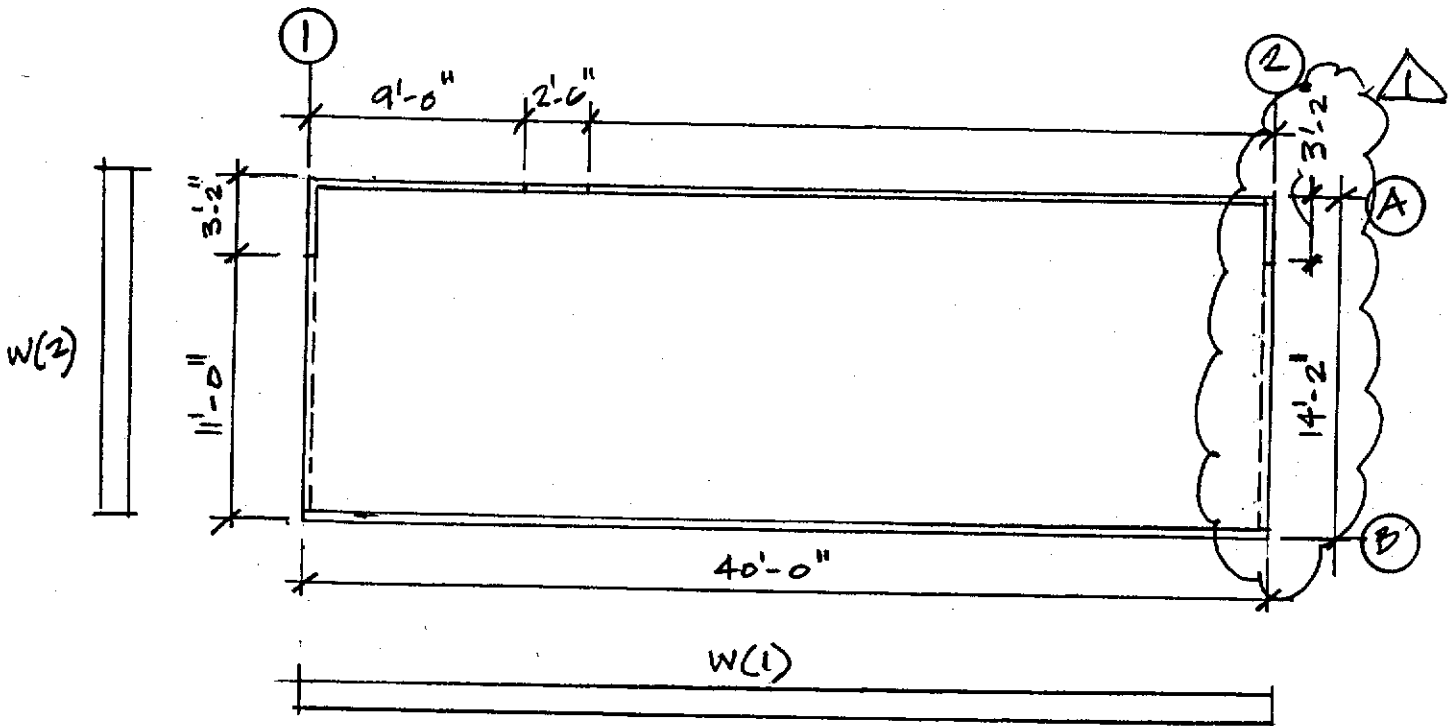
no.

date 6/5/01

5

Lateral Analysis

⚠ PLAN CHECK  
7.9.01



↑  
ASSUME  
NORTH

H. JAMES OKI ENGINEERING Consulting Engineer Yuba City, California 95991 (530) 674-7755	project Mezzanares Garage Sacramento, CA	
	by HJO	no. 6
		date 6/5/01

Lateral Analysis

△ *plu checked 7/2/01*

North / South

W(1): Wind = (20)(8') = 160 plf  
 Seismic = 0.117[14(14')+15(4')(2)] = 37 plf

Line 1: Vw = (160plf)(20') = 3200 lb  
 Vs = (37plf)(20') = 740 lb

Line 2: Vw = (160plf)(20') = 3200 lb  
 Vs = (37plf)(20') = 740 lb

Total Seismic Base Shear = 1480 lb

Diaphragm 1 △

$V_{diaph} = \frac{(160plf)(40'/2)}{14'} = 229 \text{ plf}$

$F_{chord} = \frac{(160plf)(40')^2}{8(14')} = 2286 \text{ lb}$

- 1/2" SP w/8d @ 6/6/12
- Blocked: (270 #/')
- 20-16d @ PL Splice
- (2793 #)

Line 1 & 2 △

V<sub>wind</sub> = 3200 lb  
 V<sub>seismic</sub> = 740 + 0.117[(15psf)(8')(14')] = 937 lb

$V_{walls} = \frac{3200}{3.17+3.17} = 505 \text{ plf}$

$r = (V/V_T)(10/l_w) = (740/1480)(10/6.34) = 0.79$

- 3/8" SP w/8d @ 2" cc
- (640 #/')
- (5/8"dia AB @ 24"cc)

Overturning

(3.17' Wall)  
 M<sub>OT</sub> = (505plf)(3.17')(2)(8') = 25614 ft-lb  
 M<sub>RES</sub> = 2/3[(15psf)(8')(3.17')(3.17/2)] = 402 ft-lb

$T_{up} = \frac{25614 - 402}{2.17} = 11618 \text{ lb}$

- Simpson HD14A
- (13380 #)

H. JAMES OKI ENGINEERING Consulting Engineer Yuba City, California 95991 (530) 674-7755	project Mezzanares Garage Sacramento, CA	
	by HJO	no. 7
	date 6/5/01	

Lateral Analysis

East / West

$W(2):$  Wind =  $(20)(6.67') =$  133 plf  
 Seismic =  $0.117[14(40') + 15(4')(2)] =$  80 plf

Line A:  $V_w = (133\text{plf})(7') =$  931 lb  
 $V_s = (80\text{plf})(7') =$  560 lb

Line B:  $V_w = (133\text{plf})(7') =$  931 lb  
 $V_s = (80\text{plf})(7') =$  560 lb

Total Seismic Base Shear = 1120 lb

Line A:  $V_{wind} =$  931 lb  
 $V_{seismic} =$  560 +  $0.117[(15\text{psf})(8')(40')] =$  1122 lb

$V_{walls} = \frac{1122}{28.5} =$  39 plf  
 $r = (V / V_T)(10 / I_w) = (560 / 1120)(10 / 28.5) =$  0.18

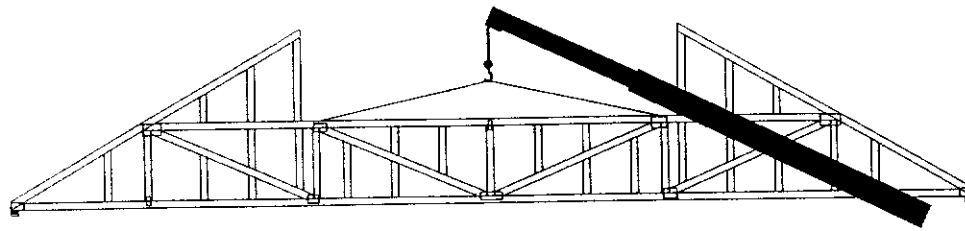
3/8" SP w/8d @ 6" cc (260 #/) (5/8" dia AB @ 48" cc)
--

Overturning (28.5' Wall)

$M_{OT} = (39\text{plf})(28.5')(8') =$  8892 ft-lb  
 $M_{RES} = 2/3[(15\text{psf})(8)](28.5')(28.5'/2) =$  32490 ft-lb

$T_{up} = \frac{8892 - 32490}{27.5} =$  0 lb

No HDs Req'd
--------------



# DDOLAN'S LUMBER

5150 FLORIN PERKINS RD. SACRAMENTO, CA 95826  
(916) 383-6501 FAX (916) 383-8157

## ROOF AND FLOOR TRUSS ENGINEERING CALCULATIONS

CUSTOMER: TOM MEZZANARES

PROJECT: GARAGE

PLAN: CUSTOM

ELEV: ROOF TRUSS

NO CHANGES AT FINAL

DESIGNER: BRIAN BAILEY

DATE: JULY 6, 2001

*Δ plan check 7.9.01*



**MiTek Industries Inc**

3033 GOLD CANAL DRIVE  
SUITE 200  
RANCHO CORDOVA CA 95670  
USA  
FAX (916) 631 8225  
TELEPHONE (916) 631 7811

December 3, 1998

RE: UBC97 Compliance

To Whom This May Concern:

This is to confirm that truss component designs produced with MI2000™ truss engineering software, version 3.3T dated January 17, 1997, version 4.0 dated December 16, 1997, and A.C.E.S.® version 1.1 for roof truss designs, version 7.1 for floor truss designs and all subsequent versions conform with all provisions of the 1997 Uniform Building Code related to metal plate connected wood truss design.

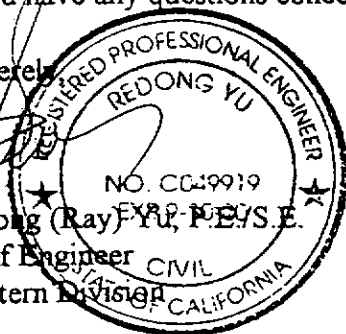
The 1997 Uniform Building Code adopts by reference the Truss Plate Institutes consensus design standard ANSI / TPI 1-1995.

By conforming to ANSI / TPI 1-1995 which is clearly stated on the drawings, UBC97 compliance is ensured. The UBC code currently referenced on the truss engineering output is 1994. This will be updated to reference UBC97 on subsequent software releases.

If you have any questions concerning this matter, I can be contacted at 800-772-5351.

Sincerely,

  
Redong (Ray) Yu, P.E./S.E.  
Chief Engineer  
Western Division



RY/ek

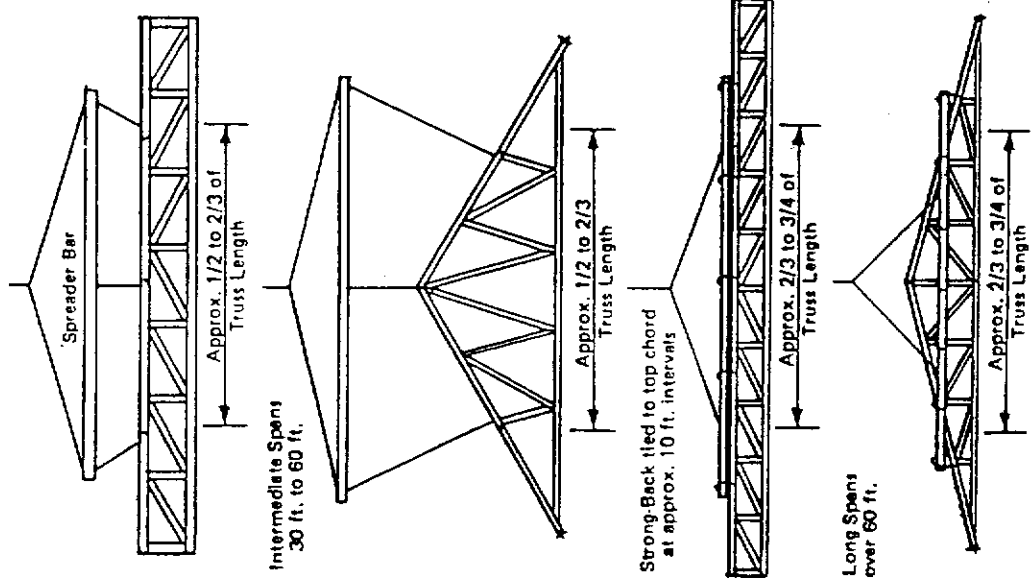
**DOLAN'S LUMBER**  
 5150 FLORIN PERKINS RD.  
 SACRAMENTO, CA  
 95826-4810  
 (916) 383-6501  
 FAX (916) 383-8157

# IMPORTANT NOTICE

**Mechanical Erection:** For trusses erected by mechanical means (cranes, fork-lifts or others), adequate slings, booms and/or spreader bars must be used to prevent lateral strain. When large trusses require a boom, the lines from the boom to the truss must be vertical or "toe-in," not "toe-out" as toeing-out will tend to cause buckling of the truss. This is especially important when long trusses or flat trusses are lifted by cranes. Trusses may be erected singly or in groups of four or five banded together. This will be largely dependent on the length and slope of the trusses.

It is extremely important that a rope be tied to the truss or group of trusses so it can be guided to the proper position on the top plate or well.

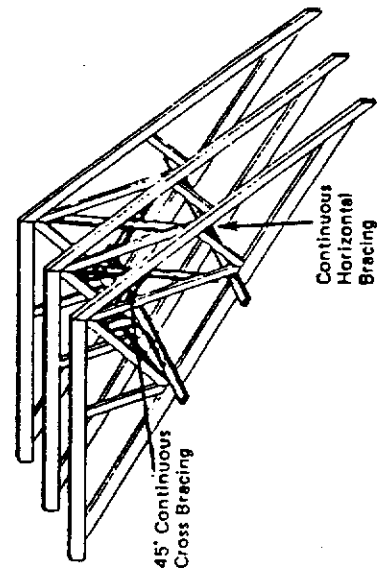
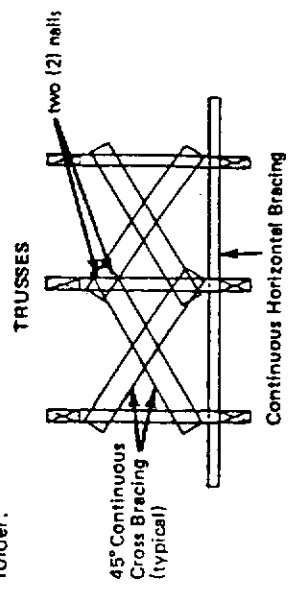
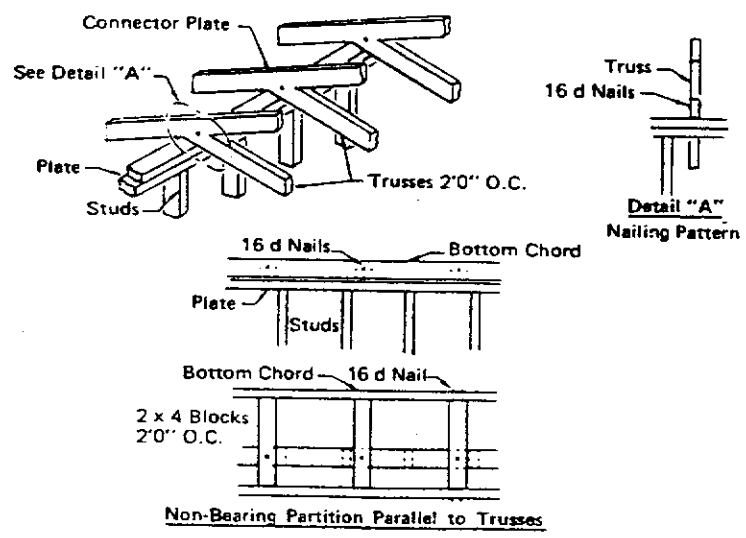
*Use common sense and caution and your truss erection job will be relatively simple and safe.*



During erection and before any application of plywood or roofing, etc., is applied; all trusses must be well braced. If you have any questions, be sure to consult with your architect or engineer for further bracing details.

Special care should be used in lifting all trusses. Protect both the trusses and erection personnel by using either a spread bar or strong back support as described in this folder.

## MISCELLANEOUS INSTALLATION DETAILS



## ROOF PERSPECTIVE

Bracing is an important and necessary part of your roof system.

## How to Erect Trussed Rafters:

These recommendations are presented only as a guide! In all cases, the erection agent — whether the fabricator or building contractor — is responsible for field handling, erection, temporary and permanent bracing since he is most familiar with local climate conditions and the special job site problems.

**Unloading Trussed Rafters:** Trusses should be unloaded onto relatively smooth ground. They should not be unloaded onto rough ground as this could cause undue lateral strains... and possibly distort or otherwise damage the trusses. Dumping is possible providing the trusses are banded into bundles. Banding them into bundles is necessary to safeguard their structural integrity as much as possible.

**How To Brace Trusses:** During erection temporary bracing shall be installed to hold the truss true and plumb and in good condition until permanent bracing can be applied. After all bracing has been applied, then the application of plywood and roofing materials will contribute to the rigidity of the complete roof structure. All components must be permanently fastened into place before the application of any loads except erection weights. There are two distinct types of bracing, "Temporary" and "Permanent." Each type is important and plays a major role in the structural integrity of the total roof system.

**Temporary Bracing:** The most common cause of dominoing is inadequate temporary bracing. Three types of temporary bracing are important.

1. **Gable End (or first truss) Bracing:** The gable end or first truss must be adequately anchored to serve as the starter. Note figure 2 which shows the inside and outside bracing of the gable end.

Gable and bracing and positioning to the existing wall of the building may be used during erection in lieu of bracing to the ground providing high winds are not experienced.

Fig. 1

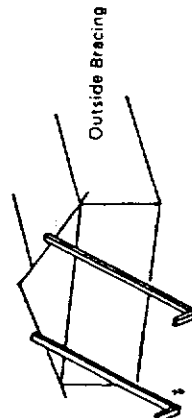
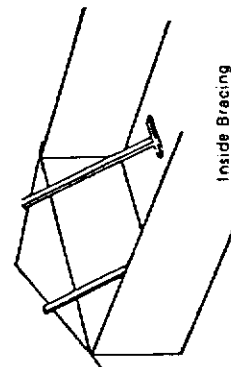


Fig. 2



2. **Truss Spacers:** Figure 3 shows short pieces of 1 x 4 or 2 x 4 used not only as a lateral bracing of the top chord back to the gable end, but also as a truss spacer to maintain the 2'-0" o.c. increment of spacing throughout the roof structure.

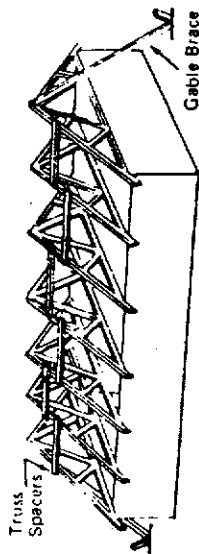


Fig. 3

3. **Cross Bracing:** As shown in figure 4, cross bracing is applied to one or more of the long diagonals within the attic space to prevent dominoing. It is important to start the cross bracing at or near the peak of one gable end and continue across the building to the base of the opposite gable end. 1 x 4's or 2 x 4's should be used for cross bracing.

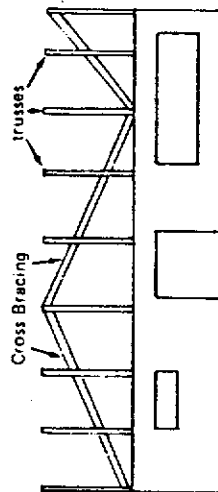


Fig. 4

**Permanent Bracing:** Top chords may be considered sufficiently laterally braced if properly nailed plywood sheathing is used. However, if other sheathing, such as spaced sheathing for shingles, metal roofing, etc., is used, additional lateral bracing may be required.

Wide spaced trusses or trusses with no ceiling materials applied direct to bottom chord require permanent bottom chord bracing. Two rows of bracing, at the third points or maximum 10'-0" o.c. tying the bottom chord to the end wall is required.

Long diagonals may require lateral bracing depending on the truss design. This bracing should start at a wall and be tied into all trusses and the opposite wall as per the truss design.

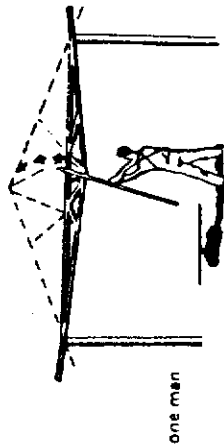
Special bracing as indicated on the roof design should be provided where required. Remember, bracing is simple if you start at a point of stability, such as a wall, and tie in all points that require bracing.

## Erection Procedure:

1. Start with one gable end truss and fasten it to the top plate with toe nails or anchors as conditions require. Be sure to brace the gable end as shown in figure 2.

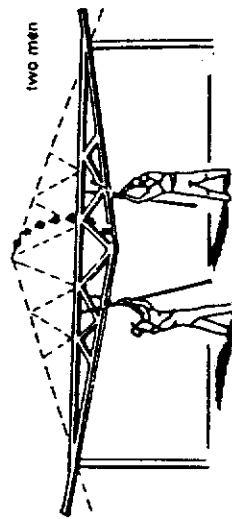
2. Next position the first common truss and fasten it to the top plate. Brace the first common truss as in figure 3.

3. Continue positioning, fastening and bracing trusses until all trusses are in place. Some erectors will follow up with a sheathing crew after about half the trusses are erected. Sheathing adds stability to the trusses when it is tied into a braced gable end.



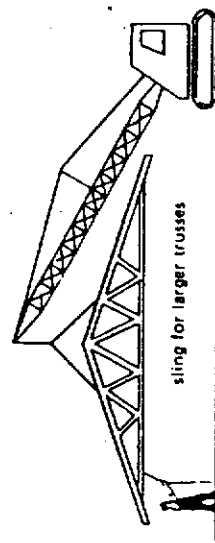
one man

Fig. 5



two men

Fig. 6



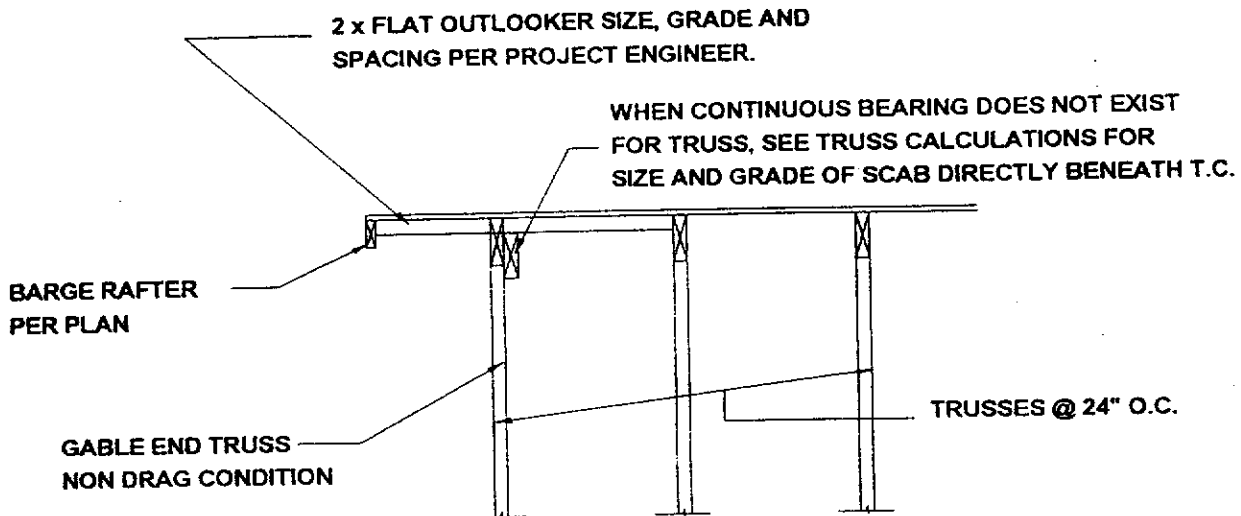
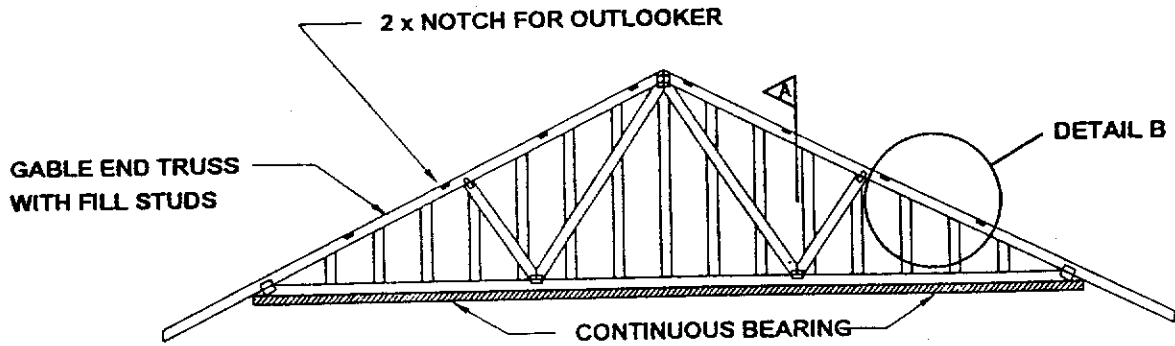
sling for larger trusses



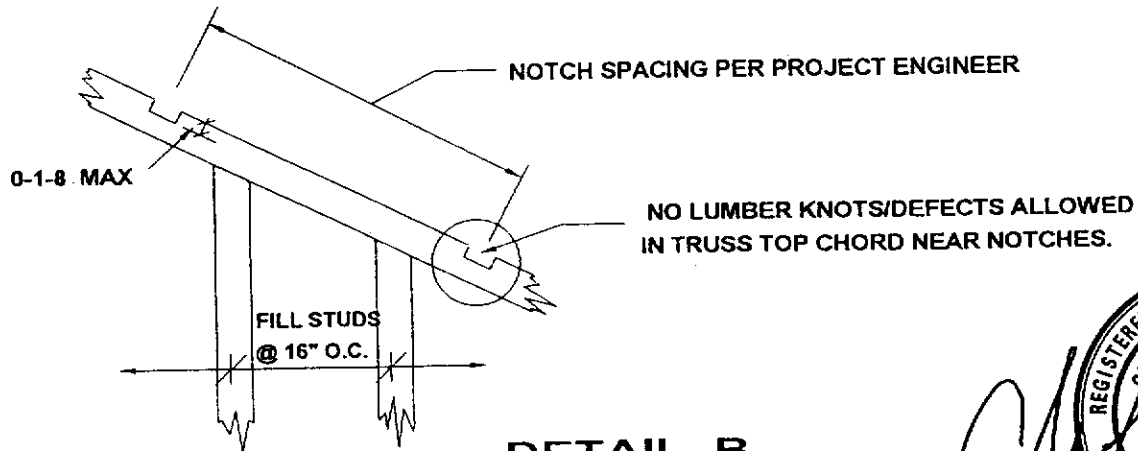
# DOLAN'S LUMBER

5150 FLORIN PERKINS RD. SACRAMENTO, CA 95826  
(916) 383-6501 FAX (916) 383-8157

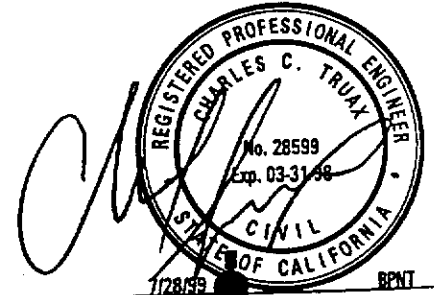
# OUTLOOKER NOTCH DETAIL



## SECTION A



GENERAL NOTES: This individual building component is designed in accordance with TPI specifications and is to be used in a building system designed by others. Metal connectors are to be of prime quality galvanized sheet steel in accordance with Building Code Standards and must be fully embedded into each truss face centered on the joint. Design assumes adequate drainage and a Dry-Condition use in a Non-Corrosive environment without the use of Fire-Retardant or Preservative-Treated lumber. Shim or wedge if necessary to provide full bearing area required. Cut all members to bear tightly against each other. Installation is entirely the responsibility of the contractor. All bracing, temporary and permanent, is the responsibility of others. For additional information contact the TRUSS PLATE INSTITUTE, Madison, WI



TRU-TRUSS ENGINEERING  
El Dorado Hills, CA 95762

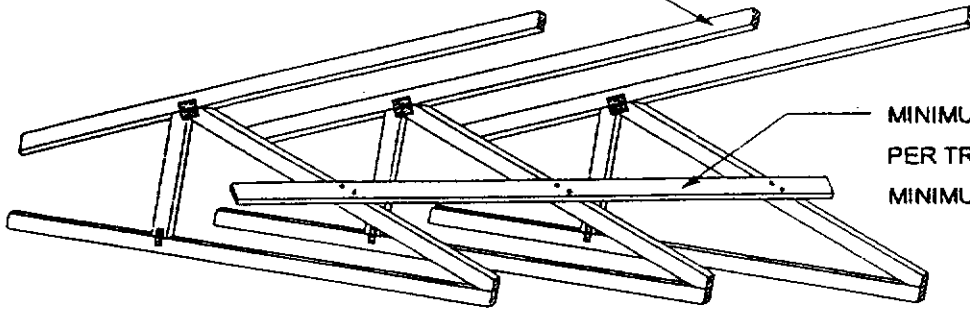
**DETAIL A-2**

**LATERAL WEB BRACING DETAIL**

**DOLAN'S LUMBER**

5150 FLORIN PERKINS RD. SACRAMENTO, CA 95826(916) 383-8801 FAX (916) 383-8157

TRUSSES @ 24" O.C. (TYP)



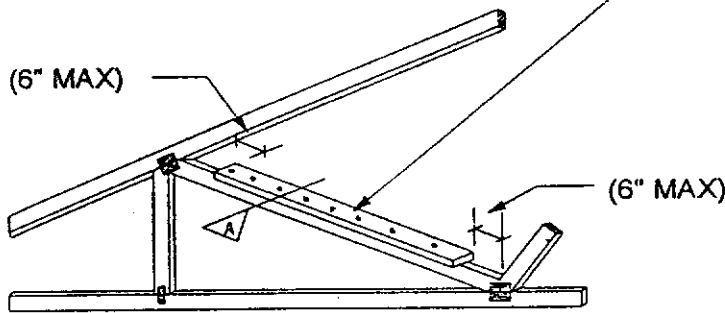
MINIMUM 1X4 OR 2X3 LATERAL BRACE PER TRUSS CALC. ATTACH USING A MINIMUM OF 2-10D NAILS PER WEB (TYP).

**NOTE:** RESTRAINT REQUIRED AT EACH END OF BRACE AND AT 20'-0" INTERVALS. RESTRAINT MAY BE PROVIDED BY TERMINATING LINE OF BRACING AT ROOF DIAPHRAM OR CEILING DIAPHRAM.

**DETAIL A-3**

**ALTERNATE LATERAL WEB BRACING DETAIL**

(6" MAX)

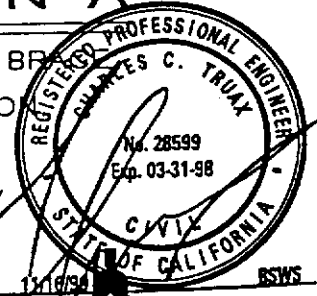


"T" BRACE OF EQUAL SIZE AND GRADE AS TRUSS WEB. ATTACH TO TRUSS USING 10D NAILS @ 4" O.C.

TRUSS WEB OF SIZE & GRADE REQUIRED BY TRUSS CALC

**SECTION A**

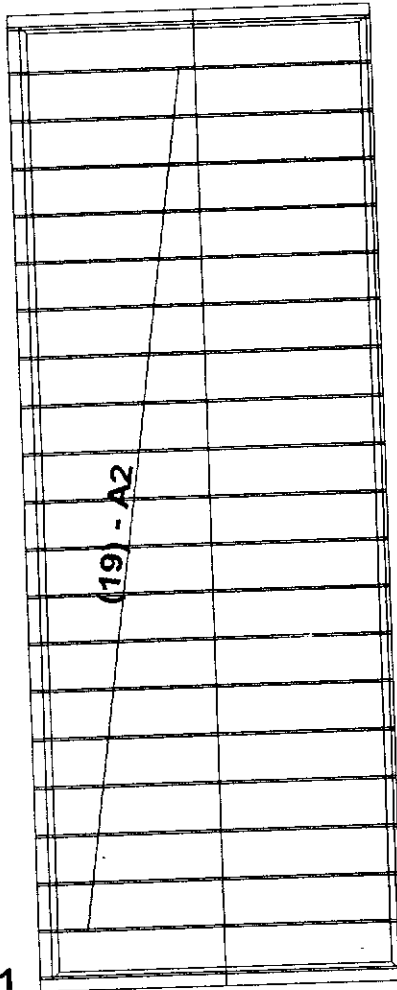
ALTERNATE "T" BRACE INSTALLATION



GENERAL NOTES: This individual building component is designed in accordance with TPI specifications and is to be used in building system designed by others. Metal connectors are to be of prime quality galvanized sheet steel in accordance with building Code Standards and must be fully embedded into each truss face centered on the joint. Design assumes adequate drainage and a Dry-Condition use in a Non-Corrosive environment without the use of Fire-Retardant or Preservative-Treated lumber. Shim or wedge if necessary to provide full bearing area required. Cut all members to bear tightly against each other. Installation is entirely the responsibility of the contractor. All bracing, temporary and permanent, is the responsibility of others. For additional information contact the TRUSS PLATE INSTITUTE, Madison, WI

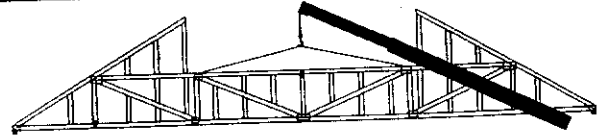
TRU-TRUSS ENGINEERING  
El Dorado Hills, CA 95762

A1



A1

CUSTOMER: TOM MEZZANARES	
JOB: GARAGE	
PLAN: CUSTOM	ELEV: ROOF TRUSS
DESIGNER: BRIAN BAILEY	DRWG #B1-054
OPTIONS:	SCALE 1/8" = 1'

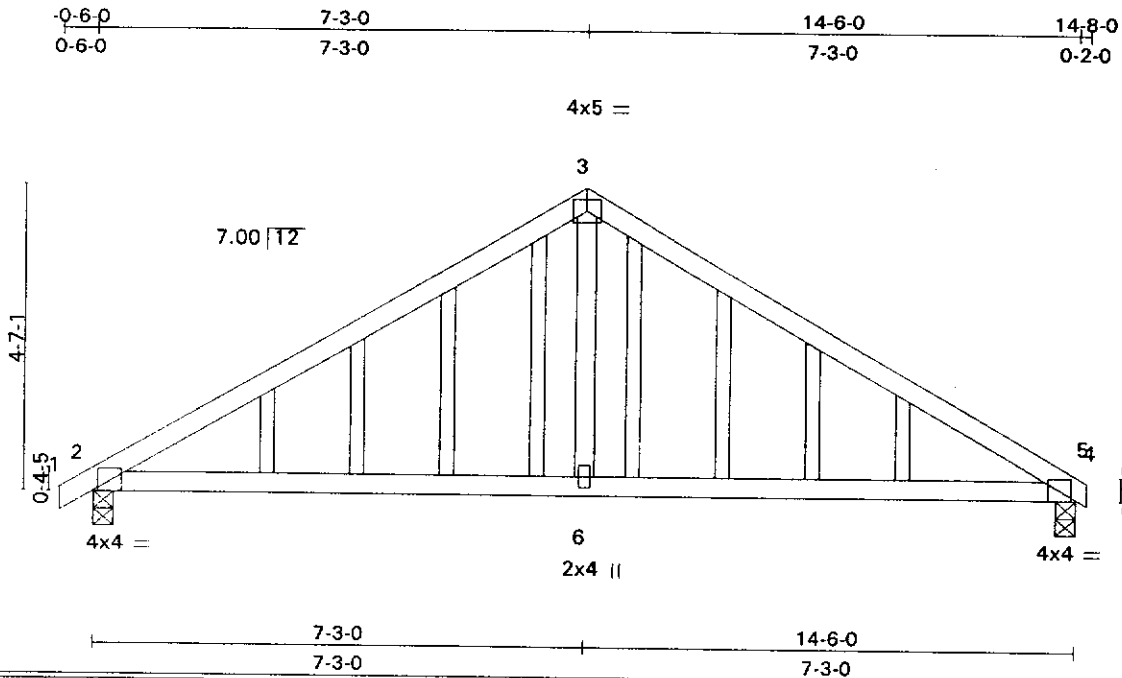


**DOLAN'S LUMBER**

5150 FLORIN PERKINS RD. SACRAMENTO, CA 95826  
(916) 363-6501 FAX (916) 363-6157

Job	Truss	Truss Type	Qty	Ply	
B1-054	a1-a2	KINGPOST	1	1	(optional)

DOLAN'S LUMBER, Sacramento, CA 4.201 SRT s Nov 16 2000 MITek Industries, Inc. Fri Jul 06 10:08:07 2001 Page 1



Scale = 1:34.0

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.58	Vert(LL)	-0.05	4-6	> 999	MII20	220/195
TCDL 14.0	Lumber Increase	1.25	BC 0.40	Vert(TL)	-0.09	4-6	> 999		
BCLL 0.0	Rep Stress Incr	NO	WB 0.10	Horz(TL)	0.01	4	n/a		
BCDL 7.0	Code	UBC97/ANSI95		1st LC LL Min l/defl	=	360			
								Weight: 68 lb	

**LUMBER**

TOP CHORD 2 X 4 DF No.1&Btr-G  
 BOT CHORD 2 X 4 DF No.1&Btr-G  
 WEBS 2 X 4 DF Std-G  
 OTHERS 2 X 3 DF Std-G

**BRACING**

TOP CHORD Sheathed or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS (lb/size)** 2 = 564/0-3-8, 4 = 544/0-3-8

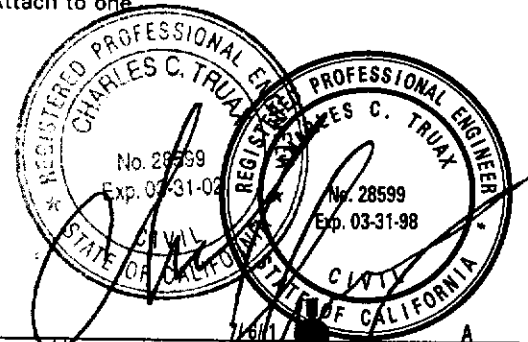
**FORCES (lb) - First Load Case Only**

TOP CHORD 1-2 = 7, 2-3 = -510, 3-4 = -510, 4-5 = 2  
 BOT CHORD 2-6 = 437, 4-6 = 437  
 WEBS 3-6 = 99

**NOTES (6-8)**

- 1) This truss has been checked for unbalanced loading conditions.
- 2) Gable studs spaced at 1-4-0 oc.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
- 4) A plate rating reduction of 20% has been applied for the green lumber members.
- 5) This truss has been designed with ANSI/TPI 1-1995 criteria.
- 6) California-Hip/Valley overframing and extended top chords to be supported 24" o.c. for uniform load distribution. Laterally brace flat top chord at indicated spacing. See Standard detail 'A1' for other framing alternatives. (When Applicable)
- 7) Alternate condition: This design is a structural truss which may contain, but does not require, vertical studs let in at 16" oc, attached with staples.
- 8) 4x2 4'oc outlooker notches requires continuous truss bearing & 16" oc gable studs, or; Attach to one side directly beneath outlookers an equal grade and size top chord scab using 10d 3" oc.

**LOAD CASE(S)** Standard



**GENERAL NOTES:** This individual building component is designed in accordance with TPI specifications and is to be used in a building system designed by others. Metal connectors are to be of prime quality galvanized sheet steel in accordance with Building Code Standards and must be fully embedded into each truss face centered on the joint. Design assumes adequate drainage and a Dry-Condition use in a Non-Corrosive environment without the use of Fire-Retardant or Preservative-Treated lumber. Shim or wedge if necessary to provide full bearing area required. Cut all members to bear tightly against each other. Installation is entirely the responsibility of the contractor. All bracing, temporary and permanent, is the responsibility of others. For additional information contact the TRUSS PLATE INSTITUTE, Madison, WI

TRU-TRUSS ENGINEERING  
 El Dorado Hills, CA 95762