

**CITY OF SACRAMENTO**

**Permit No: 9714069**

**1231 I Street, Sacramento, CA 95814**

**Insp Area: 1**

**Site Address: 4613 C ST SAC**

**Sub-Type: AOTHR**

**Parcel No: 0040135013**

**Housing (Y/N): N**

**CONTRACTOR**

GARDNER HOMES  
321 ROSS WAY  
SAC CA 95864

**OWNER**

HOFFINE SUSAN L  
4613 C ST  
SACRAMENTO CA

**ARCHITECT**

CHARLES E MCGINN  
2525 OVERHILL LN  
DAVIS CA

95819

95616

**Nature of Work: SECOND STORY ADDITION TO A SINGLE FAMILY DWELLING**

**CONSTRUCTION LENDING AGENCY :** I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name N/A Lender's Address \_\_\_\_\_

**LICENSED CONTRACTORS DECLARATION:** I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class B License Number 711223 Date 5/22/98 Contractor Signature [Signature]

**OWNER-BUILDER DECLARATION:** I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

\_\_\_\_ I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

\_\_\_\_ I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

\_\_\_\_ I am exempt under Sec. \_\_\_\_\_ B & PC for this reason: \_\_\_\_\_

Date \_\_\_\_\_ Owner Signature \_\_\_\_\_

**IN ISSUING THIS BUILDING PERMIT,** the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 5/22/98 Applicant/Agent Signature [Signature]

**WORKER'S COMPENSATION DECLARATION:** I hereby affirm under penalty of perjury one of the following declarations:

\_\_\_\_ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

\_\_\_\_ I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier no employees Policy Number \_\_\_\_\_

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 5/22/98 Applicant Signature [Signature]

**WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.**

**THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.**

9714069R (X)  
 4613 C St S

CITY OF SACRAMENTO  
DEVELOPMENT SERVICES DIVISION

# EXPRESS PLAN REVIEW

DATES					
1ST REVIEW		RECHECK		2ND RECHECK	
IN	OUT	IN	OUT	IN	OUT
10/17	1/1	1/1	1/1	1/1	1/1

PLAN CHECK NO. <u>9714069R (X)</u>	COMM.	RES.
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CONTACT PERSON: Chuck McGinn      PHONE: 753-0979  
 PROJECT ADDRESS: 4613 C St Sac 95814      FAX: 753-7627  
 DESCRIPTION OF WORK: 2<sup>ND</sup> STORY ADDITION

DISCIPLINE	1ST REVIEW			RECHECK			2ND RECHECK		
	EPR	OC	APPR	EPR	OC	APPR	EPR	OC	APPR
LIFE SAFETY			13 A7						
STRUCTURAL			13 A7						
MECHANICAL/PLUMBING			13 A7						
ELECTRICAL			13 A7						
FIRE									
PLANNING									

Legend:      EPR = OK for Express Plan Review  
                  OC = OK for Over the Counter Recheck  
                  APPR = Approved as submitted

**Structural Systems**

G.M.P. - 6/8/98

HOFFINE RESIDENCE  
4613 "C" STREET  
SACRAMENTO, CA

SHT #1 OF 8

158  
UNDEVELOPED

REDESIGN OF 2<sup>ND</sup> FLOOR SYSTEM.

- DESIGN BEAM/COL SYSTEM TO CARRY 2<sup>ND</sup> FLOOR LOADS TO FOOTINGS - ROOF LOADS BRG. ON F&R WALLS

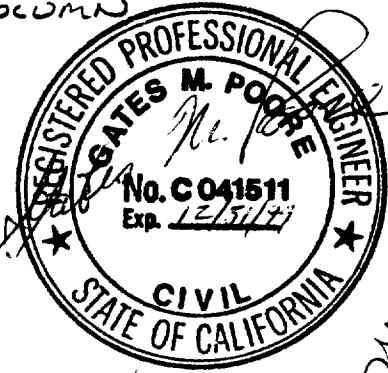
- BM WILL SUPPORT THIS SPACING 22' & 13'
- BM WILL SPAN FROM (EXIST.) LIVING RM / PORCH WALL TO REAR EXTERIOR WALL (27'-8") W/ INT. COLUMN @ (E) FLOOR FURNACE LOCATION.

(N) BEAM SPANS = (12'-6" & 15')

PAID  
CITY OF SACRAMENTO

JUN 15 1998

NEIGHBORHOODS, PLANNING  
AND DEVELOPMENT SERVICES



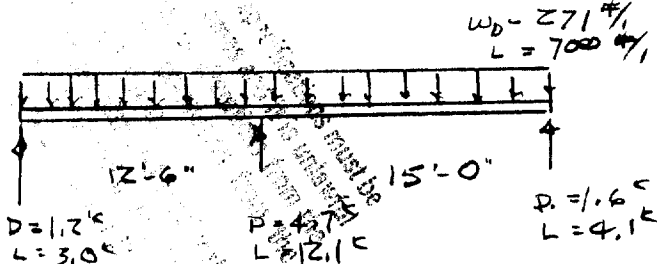
Sht # 27/4069

LOAD ON BM

$DL = 1/2 (22 + 13)(110) + 96$

$LL = 1/2 (22 + 13)(40)$

USE W8x18 (A36)



SEE COMPUTER TO.  
SHT # 2 OF  
(SPIN DATA # 1 & 2)

STRESSES & DEF. OK,  
NOTE: construction details are shown on shts 5 & 8 & 7 of 8 (DT)

COLUMN LOADS (EXT.)  $P_D = 1.6^k$   $P_L = 4.1^k$  @ BM, ENDS

USE 4x6 IN WALL COLUMN  
DF # 2 COND. FROM FTG TO STEEL BM.

(OK SEE COMP. PO. SHT # 2 OF)

- PROVIDE SOLID BLOCKING BETWEEN FOUNDATION SILL PLATE & COLUMN END.

CHECK FOUNDATION FOR ADDITIONAL COLUMN LOADS

$P_D = 1.6^k + \text{ROOF \& EXT WALL LOADS}$

$P_L = 4.1 + \text{ROOF LL}$

ROOF  $P_L = 10 \text{ psf}$

LL = 16 psf

$P_{ROD} = 10 \times 32 / 2 = 160 \#$

$P_{LL} = 16 \times 33 / 2 = 256 \#$

John Tang

6/14/98

Job Copy

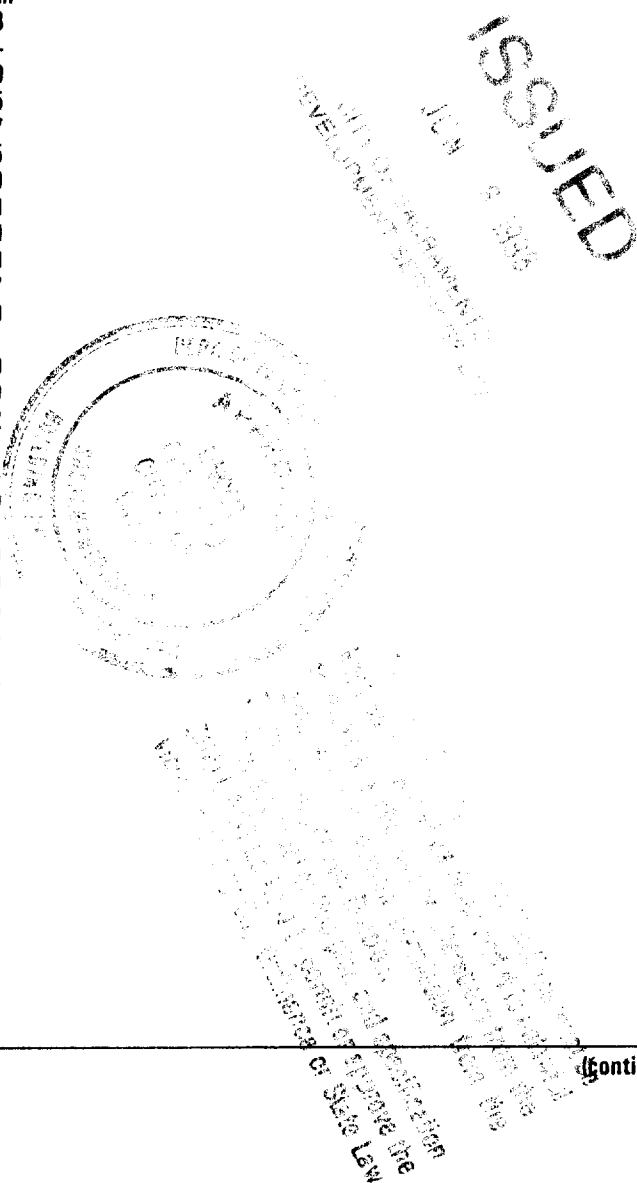
STRUCTURAL SYSTEMS  
2221 CLAREMONT RD  
CARMICHAEL, CA. 95608  
Tel (916) 483-5388

HOFFINE RESIDENCE  
4613 "C" STREET  
SACRAMENTO, CA  
CONT. 2ND FLOOR BM  
G.M.P.  
Date: 06/09/98

MULTI-SPAN STEEL BEAM DESIGN

DESIGN OF SECOND FLOOR BEAM REPLACEMENT FOR WALL SUPPORT

GENERAL DATA		1	2
All Spans Simple Support ??	:	NO	
Span Lengths	ft :	12.50	15.00
End Fixity:	:	Pin:Pin	Pin:Pin
AISC Section	:	w8x18	w8x18
CALCULATED VALUES		-OK-	-OK-
Fb - Allowable	psi :	23760	23760
fb - Actual.	psi :	18555	18555
fv - Actual	psi :	4246	4727
Moment @ Left	k-ft :	0.0	-23.5
Moment @ Right	k-ft :	-23.5	0.0
Max. Mom. @ Mid-Span	k-ft :	9.0	16.8
X-Dist	ft :	4.33	9.10
Shears: Left	k :	4.2	8.9
Right	k :	8.0	5.7
Reactions: Left:Dead	k :	1.17	4.69
Live	k :	3.02	12.11
Total	k :	4.19	16.80
Right:Dead	k :	4.69	1.59
Live	k :	12.11	4.12
Total	k :	16.80	5.71
Max. Defl. @ Mid Span	in :	-0.085	-0.304
X-Dist	ft :	4.75	8.40
BEAM DESIGN DATA			
L <sub>e</sub> : Unsupported Length	ft :	0.00	0.00
F <sub>y</sub>	ksi :	36.0	36.0
Section Area	in <sup>2</sup> :	5.26	5.26
Beam Depth	in :	8.140	8.140
Beam Width	in :	5.250	5.250
Flange Thickness	in :	0.330	0.330
Web Thickness	in :	0.230	0.230
I <sub>xx</sub>	in <sup>4</sup> :	61.9	61.9
I <sub>yy</sub>	in <sup>4</sup> :	7.97	7.97
rt	in :	1.39	1.39
APPLIED LOADS			
Use Live Load on This Span	?	Yes	Yes
Uniform DL	k/ft :	0.27	0.27
LL	k/ft :	0.70	0.70
QUERY VALUES			
Location .....	ft :	0.00	0.00
Shear	k :	4.19	8.85
Moment	k-ft :	0.00	-23.52
Deflection	in :	0.000	0.000



(continued on next page....)

STRUCTURAL SYSTEMS  
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Tel (916) 483-5388

HOFFINE RESIDENCE  
4613 "C" STREET  
SACRAMENTO, CA  
CONT. 2ND FLOOR BM  
G.M.P.  
Date: 06/09/98

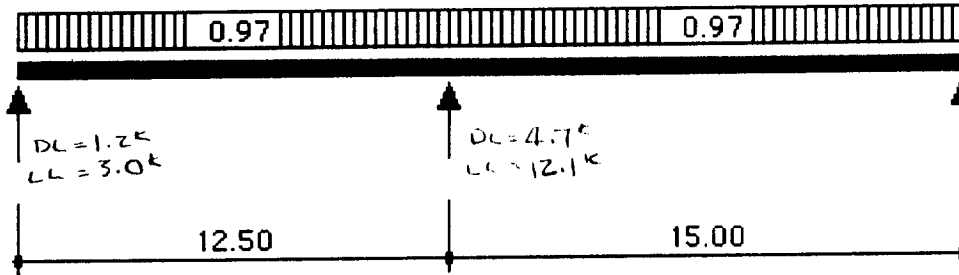
5/17 # 3 of 8

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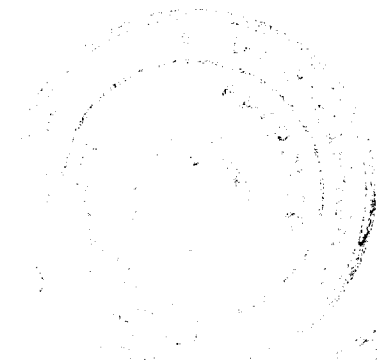
MULTI-SPAN STEEL BEAM DESIGN

DESIGN OF SECOND FLOOR BEAM REPLACEMENT FOR WALL SUPPORT

(.....continued)



**ISSUED**  
JUN 15 1998



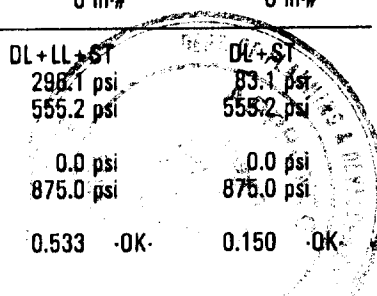
This seal and the name of the professional must be placed on all drawings and specifications prepared by the professional in the State of California. It is unlawful for any person to use this seal or the name of the professional on any drawing or specification unless the person is the professional named in the seal or the name of the professional is the name of the professional named in the seal.

**TIMBER COLUMN DESIGN**

**EXTERIOR END SUPPORT COLS**

Design Data		Allowable Stresses		
Column Depth	- 3.50 in	Fc: Parallel to Grain	-	1300 psi
Column Width	- 5.50 in	Fb: Bending	-	875 psi
		Load Duration Factor	-	1.00
		Elastic Modulus	-	1600000 psi
Total Column Ht	- 11.25 ft	Le for X-X Axis Axial Loads	-	8.00 ft
		Le for Y-Y Axis Axial Loads	-	0.00 ft
		Le for X-X Bending	-	8.00 ft
Applied Loads				
		Dead	Live	Short Term
Axial Load	-	1600.0 #	4100.0 #	0.0 #
...Eccentricity	-	0.00 in (along "depth" axis)		
Transverse Moment	-	0 in-#	0 in-#	0 in-#
Eccentric Side Load	-	0 #	0 #	0 #
....Eccentricity	-	0.00 in	0.00 in	0.00 in
....Distance above Base	-	0.00 ft	0.00 ft	0.00 in
Calculated Moments				
Equivalent Load @ Mid-Height	-	0 #	0 #	0 #
Side Load Moment	-	0 in-#	0 in-#	0 in-#
Max. Design Moment	-	0 in-#	0 in-#	0 in-#
Design Summary				
		DL+LL	DL+LL+ST	DL+S1
Actual Axial Stress	-	296.1 psi	296.1 psi	83.1 psi
Allowable " "	-	555.2 psi	555.2 psi	555.2 psi
Actual Bending Stress	-	0.0 psi	0.0 psi	0.0 psi
Allowable " "	-	875.0 psi	875.0 psi	875.0 psi
Stress Interaction Value		0.533 OK	0.533 OK	0.150 OK

**ISSUED**  
 JUN 8 1998



PROFESSIONAL ENGINEER  
 GREGORY J. ...  
 STATE OF CALIFORNIA  
 LICENSE NO. ...

FOOTING STEM WGT. =  $\frac{(8' \times 6')}{144} \times 145 = 109 \#/\text{ft}$  ✓  
 FTG = 7" THICK

- CHECK CAPACITY BASED ON 1000 PST. ALL BEG UNTESTED  
SOIL CAPACITY

LOAD ON FTG. -  $(160 + 109) + 8' \times 12 = 365 \#/\text{ft}$  ✓  
 DL - 256 #/ft

FOOTING -  $(7' \times 145) = 84.5 \#/\text{ft}$  ✓

TOT. =  $365 + 256 + 84.5 = 705.5 \#/\text{ft}$  ✓  
 w/ Col.

Cap. =  $1' \times \frac{14}{12} \times 1000 = 1167 \#/\text{ft}$

ADDL CAP. AVAIL. =  $1167 - 705.5 = 461.5 \#/\text{ft}$  ✓

TOT. ADDL CAP. REQD =  $1.6 + 4.1 = 5.7' \text{ } \llcorner$

$l = 5700 / 462 = 12.3'$   
 DO NOT PRACTICE IN FRONT OR REAR.  
 (N) FOOTINGS REQD

Use 3'-6" SPR. FTG. x 12" THK.

$P_{DL} = \frac{(410 + 84.5)}{1000} \times 3.5 + 1.6 = 3.33 \text{ K}$

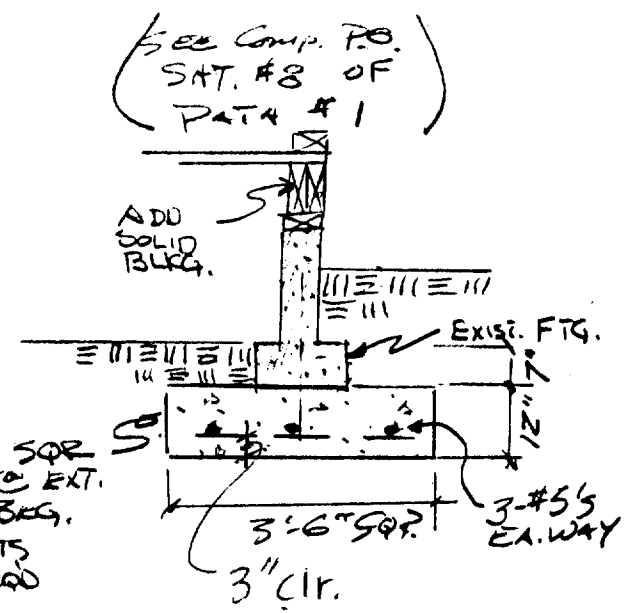
$P_{LL} = \frac{(299)}{1000} \times 3.5 + 4.1 = 5.14 \text{ K}$

Soil Brg. = 836 pst.

(OR AS ALTERNATE W/O EXCAVATING UNDER (E) FOOTING.)

ADDITIONAL AREA REQD  
 $3.5^2 - 1.167 \times 3.5 = 8.165 \text{ SF}$

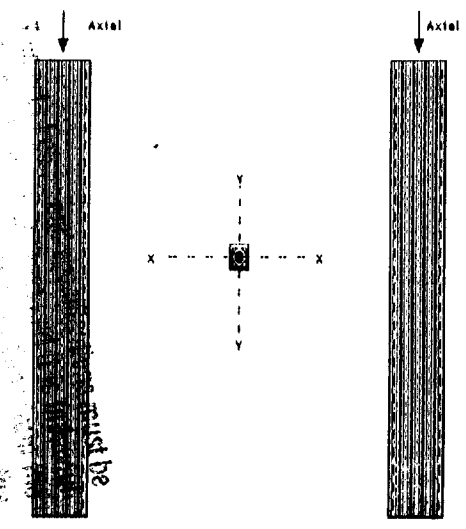
NEW SPR FTG. @ EXT. COL. BKG. POINTS (2) REQD



**STEEL COLUMN DESIGN**

**INTERIOR STEEL COLUMN**

COLUMN DATA		APPLIED LOADS	
AISC Section	1s3x3x3/16	Axial Dead Load	= 4.70 k
Column Height	= 11.25 ft	Live Load	= 12.10 k
Fixity @ Top	: Pinned	Short Term	= 0.0 k
Fixity @ Bottom	: Pinned		
Unbraced Lengths...			
For Y-Y Buckling	= 8.00 ft	X-X Axis Moments:	
For X-X Buckling	= 8.00 ft	Top	= --None--
		Bottom	= --None--
		Btwn Ends	= --None--
		Top	= --None--
		Bottom	= --None--
		Btwn Ends	= --None--
		X-X Axis	= --None--
		Y-Y Axis	= --None--
		X-X	= --None--
		Y-Y	= --None--
DESIGN DATA		SUMMARY	
Fy	= 36 ksi	Combined Stress Ratios...	
Load Duration Factor	= 1.330	Formula 1.6 - 1a	= 0.157
Live & Short Loads Don't Combine		Formula 1.6 - 1b	= 0.108
Sidesway... X-X Axis :	Restrained	Formula 1.6 - 2	= 0.118
Y-Y Axis :	Restrained	Actual & Allowable Stresses...	
Effective Length Factors...		Fa : Allowable	= 14.83
X-X Axis	= 1.00	fa : Actual	= 2.33
Y-Y Axis	= 1.00	Fb:xx : Allow [F1-6]	= 23.76
		" " [F1-7 & F1-8]	= 23.76
		fb:xx Actual	= 0.00
		Fb:yy : Allow [F1-6]	= 23.76
		" " [F1-7 & F1-8]	= 23.76
		fb:yy Actual	= 0.00
		Max X-X Axis Deflection	= 0.000 in at 0.00 ft from column base
		Max Y-Y Axis Deflection	= 0.000 in at 0.00 ft from column base
SECTION DATA		Intermediate Stress Calculation Values	
Depth	= 3.00 in	F'ex (DL+LL)	= 20856 psi
Width	= 3.00 in	F'ey (DL+LL)	= 20856 psi
Top Thickness	= 0.188 in	F'ex (DL+LL+ST)	= 27738 psi
Web Thickness	= 0.188 in	F'ey (DL+LL+ST)	= 27738 psi
Area	= 2.020 in <sup>2</sup>	Cm:x (DL+LL)	= 0.60
Weight	= 6.859 plf	Cm:y (DL+LL)	= 0.60
rT	= 0.000 in	Cm:x (DL+LL+ST)	= 0.60
Ixx	= 2.6 in <sup>4</sup>	Cm:y (DL+LL+ST)	= 0.60
Sxx	= 1.7 in <sup>3</sup>	Cb:x (DL+LL)	= 1.75
Rxx	= 1.13 in	Cb:y (DL+LL)	= 1.75
Iyy	= 2.6 in <sup>4</sup>	Cb:x (DL+LL+ST)	= 1.75
Syy	= 1.7 in <sup>3</sup>	Cb:y (DL+LL+ST)	= 1.75
Ryy	= 1.13 in		

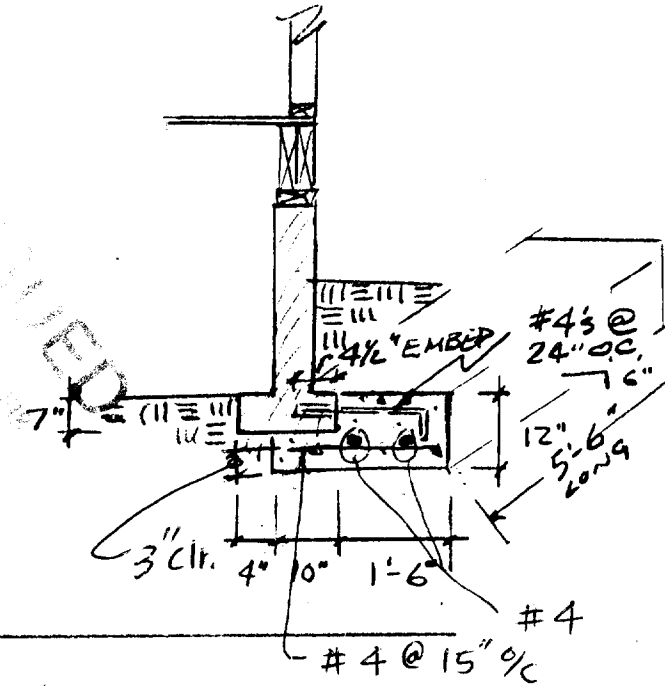


X-X AXIS LOADS Pdl = 4.70 k Pll = 12.10 k Y-Y AXIS LOADS



ALTERNATE DETAIL  
FOR EXT. COLUMN FTG ADDITION

FOOTING LENGTH = 5'-6"  
CENTERED ON EXT. COL.



FOOTING FOR INTERIOR COLUMN & COLUMN

$P_{DC} = 4.7^k$   $P_{LL} = 12.1^k$  (SEE SHT #1)

UNBROCED LENGTH = 8'-0"

USE T5 3x3x3/16 COLUMN  
W/ BASE PL & TOP PL.

FOOTING DESIGN  $F_{REQ} = 1000$  PST

USE 4'-6" SQR x 12" THK FTG  
W/ 4-#5'S EA. WAY

$$\frac{16.8^k}{4 \times 7} = .60, M = .60 \times 2' \times 1' = 1.2$$

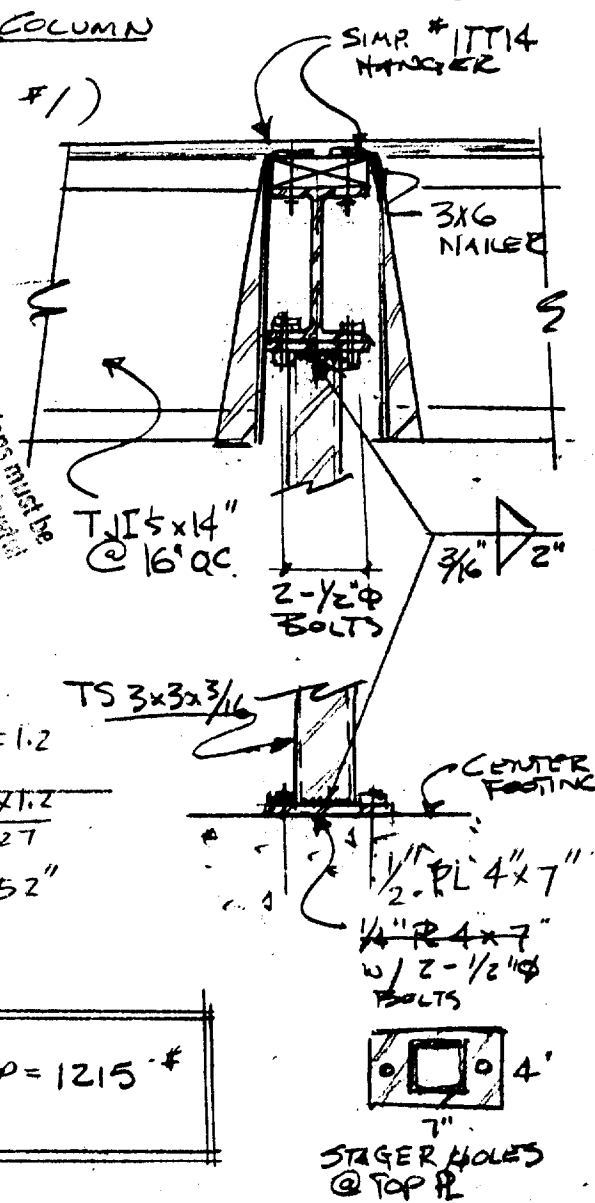
MAX. T.J.I. REACTION  $\frac{bt^2}{6} = \frac{1.2}{27}, t^2 = \frac{6 \times 1.2}{27}$

$$R = \frac{1}{2} \times 10 \times 22 \times 1.33 = 146^{\#}$$

$$L = \frac{1}{2} \times 40 \times 22 \times 1.33 = 585^{\#}$$

} 731^{\#}

USE "ITT" SERIES W/ 3x NAILER  $CAP = 1215^{\#}$   
W/ 6-16d's x 2 1/2" SIMPSON #ITT14



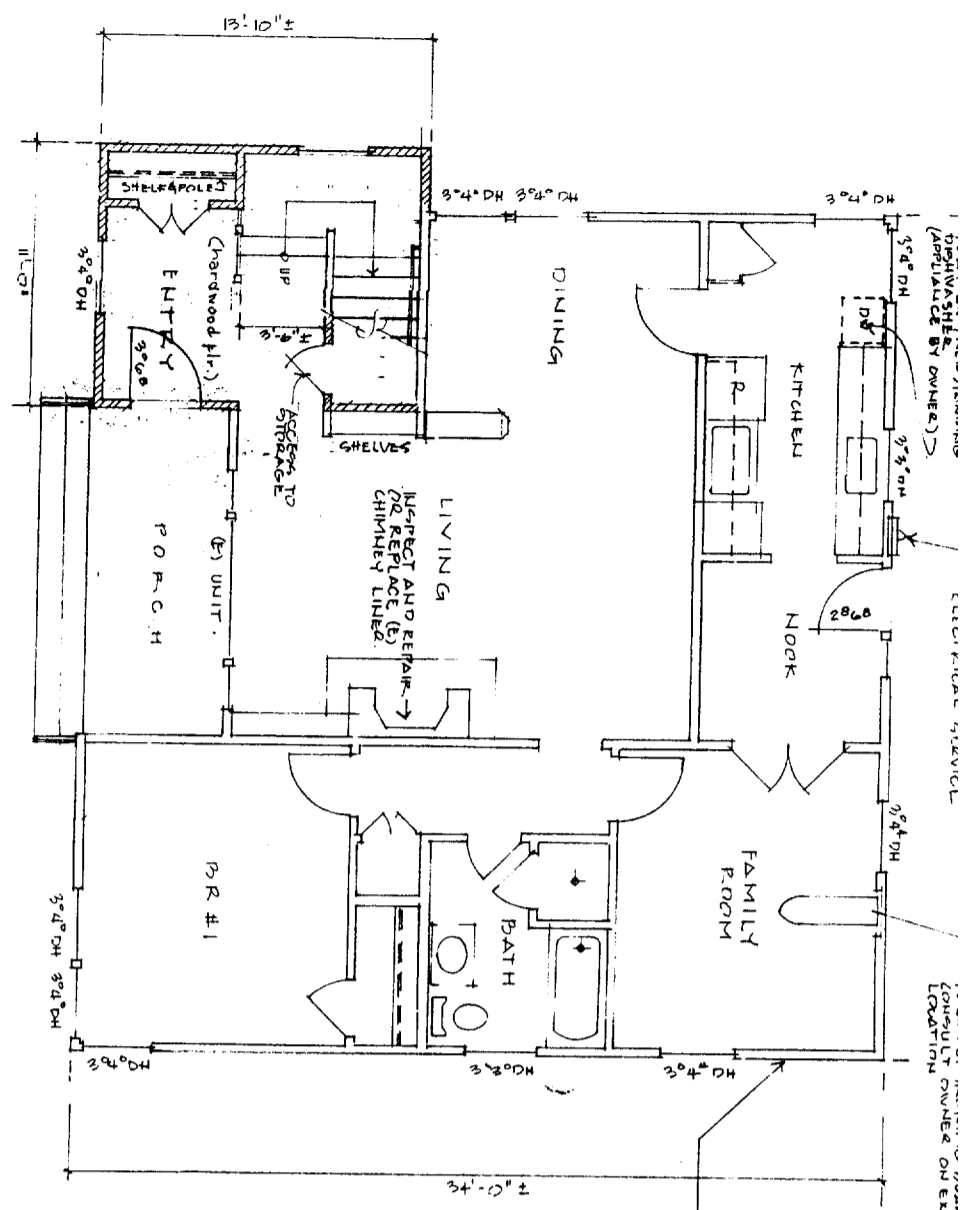
STAGER HOLES  
@ TOP PL

SQUARE FOOTING DESIGN

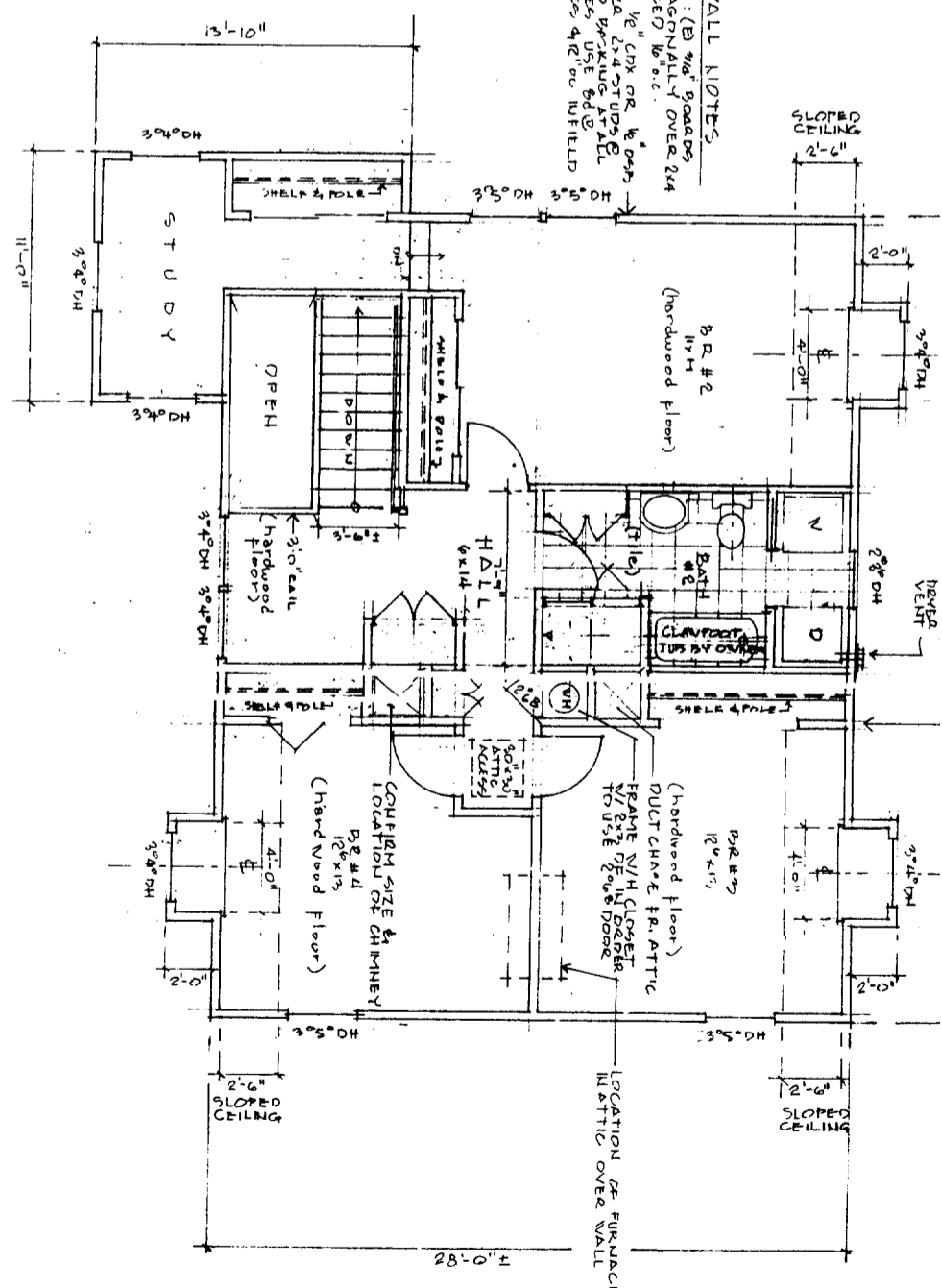
LOADING DATA		1	2
Dead Load	k:	3.33	4.70
Live Load	k:	5.14	12.10
Short Term Load	k:	0.00	0.00
Seismic Zone	:	0	0
Overburden Weight	psf:	0.0	0.0
Combining Live & Short Term Loads	?	Yes	Yes
FOOTING DATA			
Size (Length & Width)	ft:	3.50	4.50
Thickness	in:	12.00	12.00
# of Bars	:	5	5
Rebar Size Number	#:	5	5
Column Dimension	in:	6.0	6.0
f'c	psi:	2500	2500
Fy	psi:	40000	40000
Cover over rebar	in:	3.0	3.0
Concrete weight	pcf:	145.0	145.0
SOIL DATA			
Basic Allow Soil Brg Press.	psf:	1000	1000
Short Term Duration Factor	:	1.33	1.33
Ftg. Depth Below Soil	ft:	0.00	0.00
CALCULATED FORCES			
Maximum Static Soil Pressure	psf:	886.4	974.6
Allow Static Soil Pressure	psf:	1000.0	1000.0
Max. Short Term Soil Pressure	psf:	836.4	974.6
Allow Short Term Soil Pressure	psf:	1330.0	1330.0
1-Way: Allowable Shear	psi:	100.0	100.0
Vu/Phi	psi:	11.4	22.2
2-Way: Allowable Shear	psi:	200.0	200.0
Vu/Phi	psi:	32.1	66.7
Mn	k-ft:	12.3	9.7
Mu/Phi	k-ft:	1.6	3.4
REINFORCING			
Actual Bar Depth	in:	8.69	8.69
Min. Allow % Reinforcing	:	0.0014	0.0014
200/Fy	:	0.0050	0.0050
Req'd Per Analysis	:	0.0005	0.0010
USE.....	%	0.0019	0.0019
Rebar Area Req'd	in <sup>2</sup> /ft:	0.20	0.20
Total Area Req'd	in:	0.71	0.91
REBAR CHOICES			
Quantity of:			
#4 Bars	:	5	6
#5 Bars	:	3	4
#6 Bars	:	3	3
#7 Bars	:	2	3
#8 Bars	:	2	2
#9 Bars	:	2	2
#10 Bars	:	2	2

ISSUED  
 JUN 11 1998  
 CIVIL ENGINEER  
 STATE OF CALIFORNIA

Reinforcing steel specifications must be taken from the approved set of drawings.



FIRST FLOOR PLAN



SECOND FLOOR PLAN

- FINISH NOTES**
- Walls and Ceiling**
    - Drywall: 1/2" drywall with texture to match existing. Use 5/8" type X drywall under stair.
    - Paint: Use premium quality latex paint from any major paint manufacturer. Use undercoater with two coats of eggshell finish paint. In bath use semi-gloss latex. Use gloss enamel paint on all baseboards, trim, casings and moldings.
  - Floors**
    - Entry: Hardwood to match existing.
    - Nook: Linoleum.
    - Stairs: Hardwood.
    - 2nd floor: 1/4" x 2" Oak strip flooring. Except bath.
  - Bath #2**
    - Floor: Ceramic tile. Allow \$3.50 psf for material. Selection by owner.
    - Tub: Clawfoot tub and tub faucet provided by owner.
    - Lavatory: Pedestal type. Allow \$225. Selection by owner.
    - Showers: Allow \$50 for lav faucet. Selection by owner.
    - Tile floor and surround: Tempered glass door. Delta single handle shower valve with scald guard temperature control. Delta 1324 or sim.
  - Exterior**
    - Walls: Two coats exterior grade paint on all new and existing stucco surfaces.
    - Eaves: Match walls.
    - Gutter and fascia: Exterior latex gloss enamel in contrasting color. Prime fascia before mounting gutter.
    - Windows: Exterior latex gloss enamel over factory primed wood.
  - Lighting**
    - Allow \$20 per fixture. Selection of fixtures with owner's approval.

**OWNER'S NAME** Susan Hoffine  
**PROJ. ADDRESS** 4613 C St.  
**A.P.N.** 004-0135-013  
**BLDG. PERMIT #:** 97-14069 R  
**NEW FLOOR AREAS:** 1104 sq New addition

**HOFFINE RESIDENCE**  
**RESIDENCE ADDITION**  
 4613 C STREET  
 SACRAMENTO, CA 95819

**CHARLES E. MCG**  
**ARCHITECT**  
 2525 OVERHILL LANE  
 DAVIS, CA 95616  
 tel. 753-0979 fax 753-762

Revisions	Date	By	File Name
4	10-09-91	CEM	
5	02-07-91		
6	02-09-91		
7	1-17-97		
1	1-15-97		

Job No. 97-16-E  
 Sheet Title  
**1st FLOOR PLAN**  
**2nd FLOOR PLAN**  
 Sheet No.  
**A3**