

CITY OF SACRAMENTO
1231 I Street, Sacramento, CA 95814

Permit No: 0013258
Insp Area: T

Site Address: 1301 Q ST SAC
Parcel No: 006-0284-045

CAPITOL PARK HOMES LOT 35
Sub-Type: NSFR
Housing (Y/N): N

CONTRACTOR
CAPITOL PARK HOMES
818 19TH ST STE 200
SACRAMENTO CA 95814

OWNER
CAPITOL PARK HOMES

ARCHITECT

Nature of Work: MP 1609 3 STORY 6 ROOM SFR

CONSTRUCTION LENDING AGENCY: I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 309.1, Civ. Code)

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class B License Number 767087 Date 4-19-01 Contractor Signature [Signature]

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code): any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00).

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code). The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code). The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law.

I am exempt under Sec. _____ B & PC for this reason: _____
Date _____ Owner Signature _____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 4-19-01 Applicant/Agent Signature [Signature]

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier Binder Policy Number _____ Exp Date _____

This section need not be completed if the permit is for \$100 or less. I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 4-19-01 Applicant Signature [Signature]

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

Certification of Compliance

School District Development

Part I - To be completed by the APPLICANT

Owner's Name/Address SHASTA MOUNTAIN SINGLE FAMILY DEV. LLC
 Project Address 1501 Q ST
 Parcel Number 006-0274-045 Lot No. 35
 Subdivision Name CAPITOL PARK HOMES No. of Units _____
 Applicant's Signature [Signature] Title FM
 Phone No. 768-1701 Date 4-5-01

Notice to Applicant: Pursuant to Government Code Section 66020(d), this will serve to notify you that the 90-day approval period in which you may protest the fees or other payment identified above will begin to run on the date in which the building or installation permit for this project is issued or on which they are paid to the district(s) or to another public entity authorized to collect them on behalf of the district(s), whichever is earlier.

Part II - To be completed by the BUILDING DEPARTMENT

Plan Identification Number 11129
 Building Type (check one) Residential Apartment/Condominium Commercial/Industrial
 Square Feet of Chargeable Building Area ~~11129~~ 11130
 Signature/Title [Signature] Date 4-5-01

Part III - To be completed by the SCHOOL DISTRICT

School District 20050 Certificate No. 7040
 Exempt Comments \$452.22 per unit - SEE ATTACHMENT A
 Residential/Apartment/etc. _____ Square ft. x \$ 1.72 = \$ 452.22
 Commercial/Industrial _____ Square ft. x \$ _____ = \$ _____
 Total fees collected 11129 = \$ 452.22

This certification covers only the amount of square footage indicated above. Any additions or corrections to the square footage for this project will require an amendment to the Certificate of Compliance.

As the authorized school official, I hereby certify that the requirements of Government Code Section 65995 and any other authorized requirements have been complied with by the above signed applicant.

Signature [Signature] Date 4/12/01

CERTIFICATION OF INSULATION

PART I GENERAL

ADDRESS OR TRACT

SACRAMENTO BUILDING PRODUCTS

CNM CONST.

LOT # 350

- P.O. BOX 854, WEST SACRAMENTO, CA 95691 LIC. #202026
- 1309 MELODY ROAD, MARYSVILLE, CA 95901 LIC. #202026
- P.O. BOX 9651, FRESNO, CA 93793-9651 LIC. #202026
- P.O. BOX 1631, RENO, NV 89505 LIC. #10675
- 3326 A PONDEROSA WAY, LAS VEGAS, NV 89118 LIC. #10675

CAPITOL PARK HOMES

DATE INSULATION COMPLETED

4-10-02

PART II AREAS INSULATED

WALLS		CEILING			FLOORS	
(SQUARE FEET)		(SQUARE FEET)			(SQUARE FEET)	
TYPE OF INSULATION		TYPE OF INSULATION			TYPE OF INSULATION	
MATERIAL FIBERGLASS		MATERIAL FIBERGLASS			MATERIAL FIBERGLASS	
FORM BATTS		FORM BATTS & BLOW			FORM BATTS	
MANUFACTURER'S PRODUCT I.D.		MANUFACTURER'S PRODUCT I.D.			MANUFACTURER'S PRODUCT I.D.	
MANUFACTURER		MANUFACTURER			MANUFACTURER	
OCF		OCF			OCF	
R - VALUE INSTALLED		APPLIED THICKNESS		R - VALUE INSTALLED	APPLIED THICKNESS	MIN. INSTALLED WEIGHT PER SQUARE FOOT
13		3 1/2"		30	9'	


KNEE WALLS IF R-VALUE IS OTHER THAN WALLS ABOVE

MATERIAL FIBERGLASS	FORM BATTS	R VALUE	MANUFACTURER OCF
-------------------------------	----------------------	---------	----------------------------

AIR INFILTRATION SEALANT

MATERIAL FOAM	MANUFACTURER W R GRACE
-------------------------	----------------------------------

THIS IS TO CERTIFY THAT INSULATION AND/OR SEALANT HAS BEEN INSTALLED IN CONFORMANCE WITH APPLICABLE CODES, MATERIAL STANDARDS AND REGULATIONS.

SIGNATURE - INSULATION CONTRACTOR 	TITLE MANAGER	DATE 4-11-02
SIGNATURE - GENERAL CONTRACTOR	TITLE	DATE

REMARKS:

0013258 PLAN 3

RESIDENTIAL SUBDIVISION BUILDING PERMIT APPLICATION

Project Address: 1301 Q St. Assessor Parcel # 006-0284-045
Lot Number: 35 Subdivision CAPITOL PARK HOMES

OWNER INFORMATION:

Legal Property Owner: SHASTA/DOWNTOWN SINGLE FAM Phone# 449-8989
Owner Address: 818 19th ST City SACRAMENTO State CA Zip 95814

CONTRACTOR INFORMATION:

Contractor: CNM CONST Lic. # 767087 Phone # 449-8989 Fax 449-8865

PROJECT INFORMATION:

Land Use Zone RIA Occupancy Group R3 Construction Type VN Fed Code 1A
No. of Stories: 3 No. of Rooms: 10 Street Width: 20
1st Floor Area 1110 2nd Floor Area 499 Basement - Roof Material COMP

AREA IN SQUARE FOOT OF:

Dwelling/Living 1609
Garage/Storage 499 / 538
Decks/Balconies 130
Carports -

SCOPE OF WORK: NEW SINGLE FAMILY RESIDENCE

FOR OFFICE USE ONLY

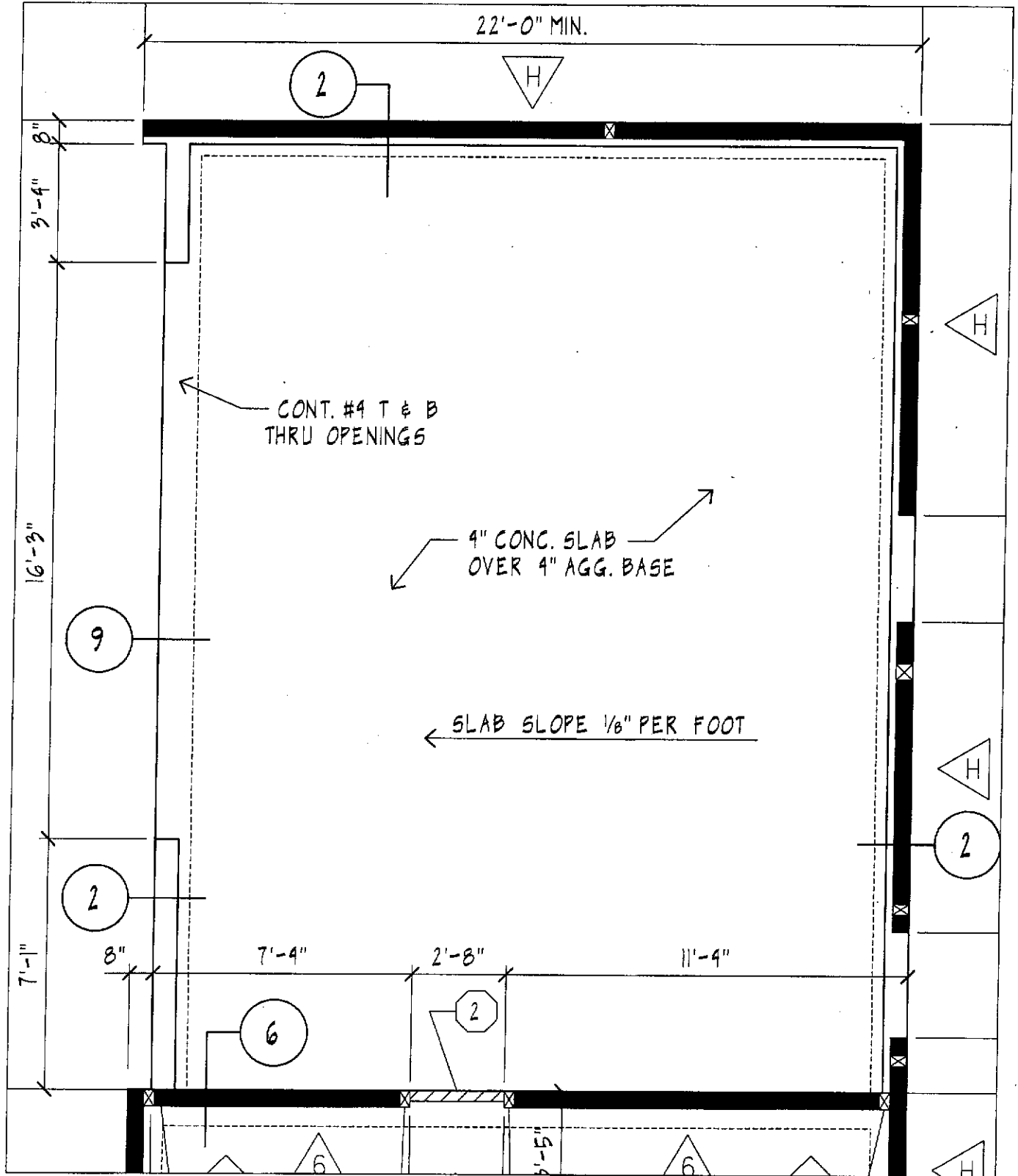
- | | | |
|---|---|---|
| <input type="checkbox"/> Information Above Complete | <input type="checkbox"/> AR Flood Waiver Required | <input type="checkbox"/> Planning Approval |
| <input type="checkbox"/> Violation Files Checked | <input type="checkbox"/> Flood Elevation Certificate Required | <input type="checkbox"/> Design Review Approval |
| <input type="checkbox"/> Standard Setbacks | <input type="checkbox"/> Water Development Infill Area | <input type="checkbox"/> Special Fee Districts Apply: |
| <input type="checkbox"/> County Sewer | | |

THE FOLLOWING MUST BE PROVIDED IN ORDER TO SUBMIT FOR PERMIT

- 2 COMPLETE PLOT PLANS, LEGIBLE & DRAWN TO SCALE
- 11 X 17 COPY OF FLOOR PLAN WITH FOLLOWING INFORMATION
 - a) Assessor's Parcel Number
 - b) New Floor Area
 - c) Owners Name
 - d) Project Address

Date: _____ Received by: (staff) _____ Permit # _____

PLAN ADDENDUM



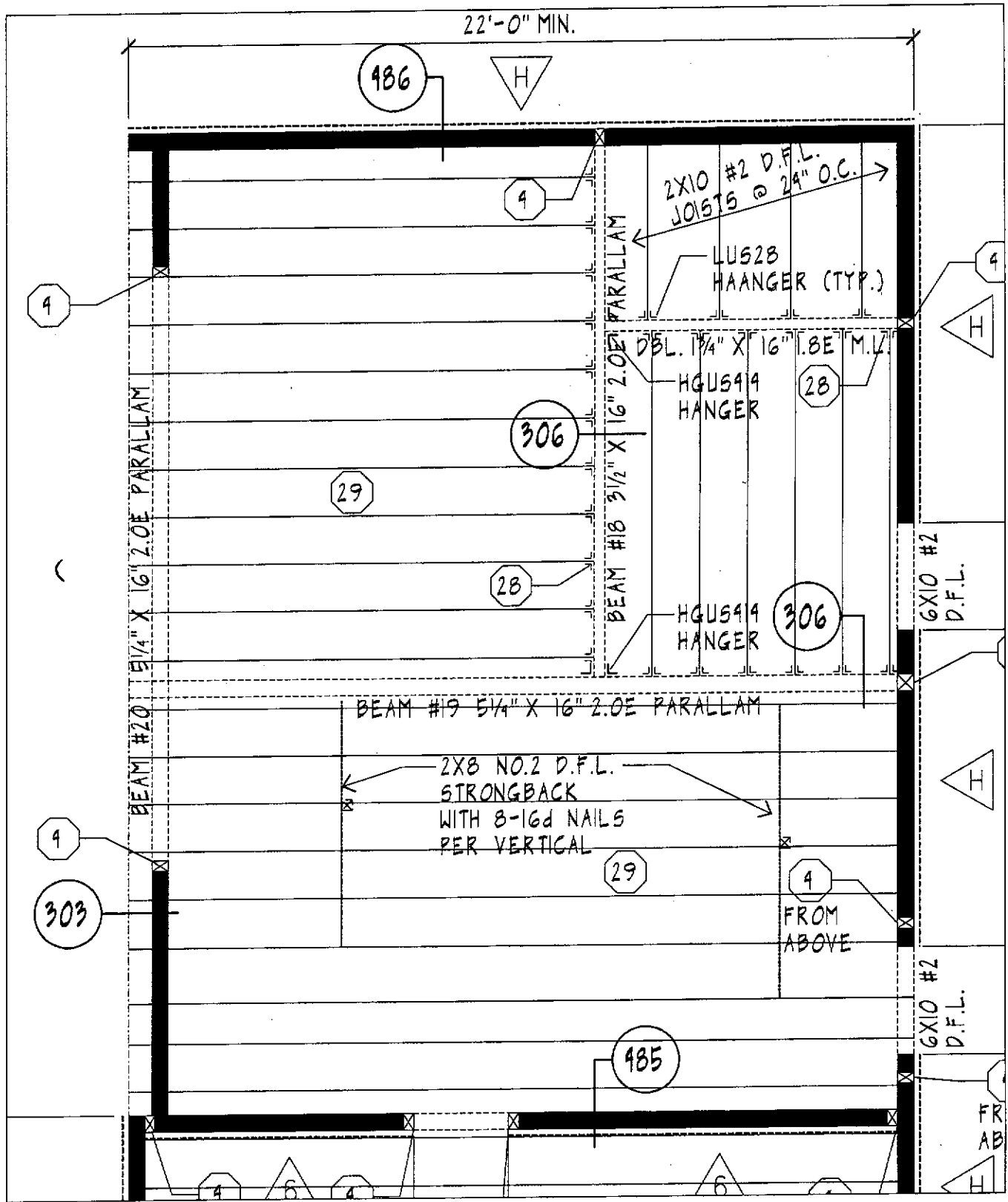
PLAN 3 - SIDE LOAD GARAGE OPTION

NORMAN SCHEEL
STRUCTURAL ENGINEER
 5022 SUNRISE BLVD
 FAIR OAKS, CA 95628
 VOICE (916) 536-9585
 FAX (916) 536-0260

PROJECT CAPITAL PARK
 CLIENT CAPITAL PARK HOMES
 JOB NO. 98119
 PROJECT MGR. PC
 DATE. 06-12-01
 PAGE 1 OF 12



PLAN ADDENDUM



NORMAN SCHEEL
STRUCTURAL ENGINEER
 5022 SUNRISE BLVD
 FAIR OAKS, CA 95628
 VOICE (916) 536-9585
 FAX (916) 536-0260

PROJECT _____
 CLIENT _____
 JOB NO. _____
 PROJECT MGR. _____
 DATE _____
 PAGE 2 OF 12



NORMAN SCHEEL
STRUCTURAL ENGINEER
5022 SUNRISE BLVD.
FAIR OAKS, CA 95628
(916) 536-9585

Title : CAPITAL PARK
 Dsgnr: DP
 Description : LEXINGTON HOMES

Job # 98119
 Date: 10:36AM, 13 JUN 01

3/12

Scope :

Rev: 510301
 User: KW-0602320, Ver 5.1.3, 22-Jun-1999, Win32
 (c) 1983-99 ENERCALC

General Timber Beam

Page 1

J:\jobfiles\98119\enercalc\98119.ecw:Calculat

Description PLAN 3 SIDE LOAD BEAM #18

General Information Calculations are designed to 1997 NDS and 1997 UBC Requirements

Section Name	3 1/2 x 16	Center Span	15.00 ftLu	0.00 ft
Beam Width	3.500 in	Left Cantilever	ftLu	0.00 ft
Beam Depth	16.000 in	Right Cantilever	ftLu	0.00 ft
Member Type		Truss Joist - MacMillan, MicroLam 1.8 E			
Bm Wt. Added to Loads		Fb Base Allow	2,600.0 psi		
Load Dur. Factor	1.000	Fv Allow	285.0 psi		
Beam End Fixity	Pin-Pin	Fc Allow	750.0 psi		
Wood Density	34.000 pcf	E	1,800.0 ksi		

Full Length Uniform Loads

Center	DL	120.00 #/ft	LL	320.00 #/ft
Left Cantilever	DL	#/ft	LL	#/ft
Right Cantilever	DL	#/ft	LL	#/ft

Trapezoidal Loads

#1 DL @ Left	200.00 #/ft	LL @ Left	200.00 #/ft	Start Loc	0.000 ft
DL @ Right	200.00 #/ft	LL @ Right	200.00 #/ft	End Loc	8.000 ft

Point Loads

Dead Load	1,500.0 lbs	lbs	lbs	lbs	lbs	lbs	lbs
Live Load	1,500.0 lbs	lbs	lbs	lbs	lbs	lbs	lbs
...distance	8.000 ft	0.000 ft	0.000 ft	0.000 ft	0.000 ft	0.000 ft	0.000 ft

Summary

Beam Design OK

Span= 15.00ft, Beam Width = 3.500in x Depth = 16.in, Ends are Pin-Pin							
Max Stress Ratio	0.923		: 1				
Maximum Moment Allowable	29.9 k-ft	32.4 k-ft	Maximum Shear * 1.5 Allowable	9.0 k	16.0 k		
Max. Positive Moment	29.86 k-ft	at 7.980 ft	Shear:	@ Left	7.15 k		
Max. Negative Moment	0.00 k-ft	at 0.000 ft		@ Right	5.85 k		
Max @ Left Support	0.00 k-ft		Camber:	@ Left	0.000 in		
Max @ Right Support	0.00 k-ft			@ Center	0.320 in		
Max. M allow	32.36		Reactions...	@ Right	0.000 in		
fb	2,399.22 psi	fv	161.24 psi	Left DL	2.87 k	Max	7.15 k
Fb	2,600.00 psi	Fv	285.00 psi	Right DL	2.23 k	Max	5.85 k

Deflections

Center Span...	Dead Load	Total Load	Left Cantilever...	Dead Load	Total Load
Deflection	-0.213 in	-0.526 in	Deflection	0.000 in	0.000 in
...Location	7.440 ft	7.440 ft	...Length/Defl	0.0	0.0
...Length/Defl	843.5	342.39	Right Cantilever...		
			Deflection	0.000 in	0.000 in
			...Length/Defl	0.0	0.0

NORMAN SCHEEL
STRUCTURAL ENGINEER
5022 SUNRISE BLVD.
FAIR OAKS, CA 95628
(916) 536-9585

Title : CAPITAL PARK
 Dsgnr: DP
 Description : LEXINGTON HOMES

Job # 98119

Date: 10:36AM, 13 JUN 01

Scope :

4/12

Rev: 510301
 User: KW-0602320, Ver 5.1.3, 22-Jun-1999, Win32
 (c) 1983-99 ENERCALC

General Timber Beam

Page 2

j:\jobfiles\98119\enercalc\98119.ecw:Calculat

Description PLAN 3 SIDE LOAD BEAM #18

Stress Calcs

Bending Analysis

Ck 21.339 Le 0.000 ft Sxx 149.333 in3 Area 56.000 in2
 1.000 Rb 0.000 Cl 0.000

	Max Moment	Sxx Req'd	Allowable fb
@ Center	29.86 k-ft	137.80 in3	2,600.00 psi
@ Left Support	0.00 k-ft	0.00 in3	2,600.00 psi
@ Right Support	0.00 k-ft	0.00 in3	2,600.00 psi

Shear Analysis

	@ Left Support	@ Right Support
Design Shear	9.03 k	7.88 k
Area Required	31.682 in2	27.654 in2
Fv: Allowable	285.00 psi	285.00 psi

Bearing @ Supports

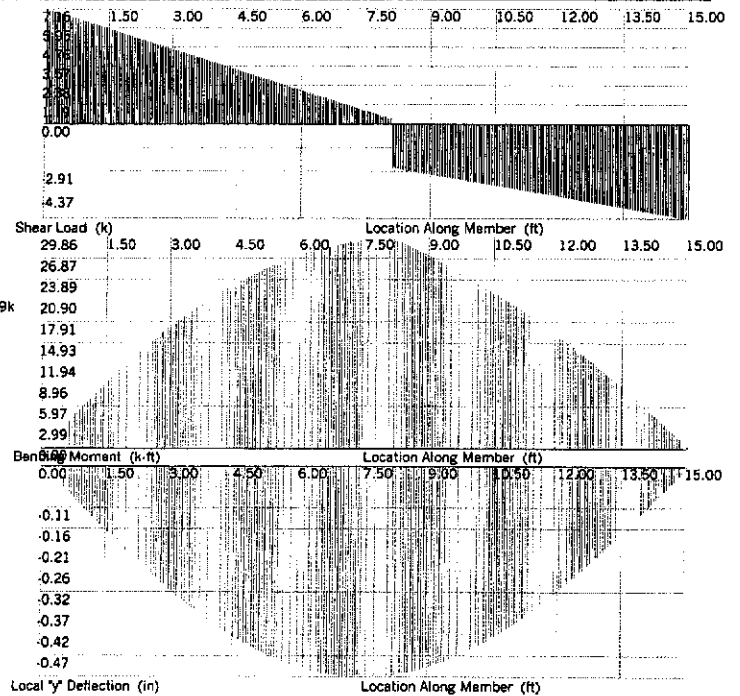
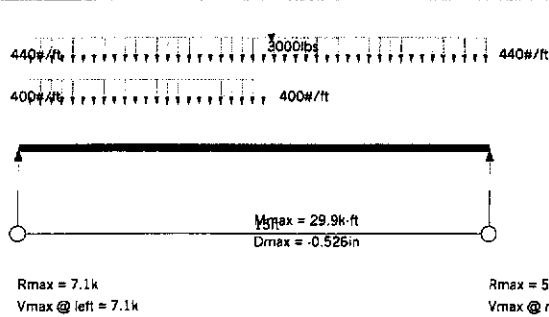
		Bearing Length Req'd
Max. Left Reaction	7.15 k	2.722 in
Max. Right Reaction	5.85 k	2.230 in

Query Values

M, V, & D @ Specified Locations

	Moment	Shear	Deflection
@ Center Span Location =	0.00 ft	0.00 k-ft	7.15 k
@ Right Cant. Location =	0.00 ft	0.00 k-ft	0.0000 in
@ Left Cant. Location =	0.00 ft	0.00 k-ft	0.0000 in

Sketch & Diagram



NORMAN SCHEEL
STRUCTURAL ENGINEER
5022 SUNRISE BLVD.
FAIR OAKS, CA 95628
(916) 536-9585

Title : CAPITAL PARK
 Dsgnr: DP
 Description : LEXINGTON HOMES

Job # 98119
 Date: 10:38AM, 13 JUN 01

Scope :

5/12

Rev: 510301
 User: KW-0602320, Ver 5.1.3. 22-Jun-1999, Win32
 (c) 1983-99 ENERCALC

General Timber Beam

Page 1

j:\jobfiles\98119\enercalc\98119.ecw:Calculat

Description PLAN 3 SIDE LOAD BEAM #19

General Information Calculations are designed to 1997 NDS and 1997 UBC Requirements

Section Name	Prilm: 5.25x16.0	Center Span	21.00 ftLu	0.00 ft
Beam Width	5.250 in	Left Cantilever	ftLu	0.00 ft
Beam Depth	16.000 in	Right Cantilever	ftLu	0.00 ft
Member Type		Truss Joist - MacMillan, Parallam 2.0E W			
Bm Wt. Added to Loads		Fb Base Allow	2,900.0 psi		
Load Dur. Factor	1.000	Fv Allow	290.0 psi		
Beam End Fixity	Pin-Pin	Fc Allow	650.0 psi		
Wood Density	34.000 pcf	E	2,000.0 ksi		

Full Length Uniform Loads

Center	DL	30.00 #/ft	LL	80.00 #/ft
Left Cantilever	DL	#/ft	LL	#/ft
Right Cantilever	DL	#/ft	LL	#/ft

Trapezoidal Loads

#1 DL @ Left	75.00 #/ft	LL @ Left	200.00 #/ft	Start Loc	12.000 ft
DL @ Right	75.00 #/ft	LL @ Right	200.00 #/ft	End Loc	91.000 ft

Point Loads

Dead Load	7,100.0 lbs	-1,430.0 lbs	lbs	lbs	lbs	lbs	lbs
Live Load	lbs	-1,060.0 lbs	lbs	lbs	lbs	lbs	lbs
...distance	12.000 ft	14.000 ft	0.000 ft	0.000 ft	0.000 ft	0.000 ft	0.000 ft

Summary

Beam Design OK

Span= 21.00ft, Beam Width = 5.250in x Depth = 16.in, Ends are Pin-Pin

Max Stress Ratio	0.737	:	1			
Maximum Moment Allowable	39.9 k-ft		54.1 k-ft	Maximum Shear * 1.5 Allowable	7.8 k	24.4 k
Max. Positive Moment	39.87 k-ft	at	12.012 ft	Shear:	@ Left	4.11 k
Max. Negative Moment	-0.00 k-ft	at	21.000 ft		@ Right	5.71 k
Max @ Left Support	0.00 k-ft			Camber:	@ Left	0.000 in
Max @ Right Support	0.00 k-ft				@ Center	0.938 in
Max. M allow	54.13				@ Right	0.000 in
fb	2,136.15 psi	fv	92.77 psi	Reactions...		
Fb	2,900.00 psi	Fv	290.00 psi	Left DL	3.23 k	Max 4.11 k
				Right DL	4.16 k	Max 5.71 k

Deflections

Center Span...	<u>Dead Load</u>	<u>Total Load</u>	Left Cantilever...	<u>Dead Load</u>	<u>Total Load</u>
Deflection	-0.625 in	-0.733 in	Deflection	0.000 in	0.000 in
...Location	10.836 ft	10.836 ft	...Length/Def	0.0	0.0
...Length/Def	403.1	343.64	Right Cantilever...		
			Deflection	0.000 in	0.000 in
			...Length/Def	0.0	0.0

NORMAN SCHEEL
STRUCTURAL ENGINEER
5022 SUNRISE BLVD.
FAIR OAKS, CA 95628
(916) 536-9585

Title : CAPITAL PARK
Dsgnr: DP
Description : LEXINGTON HOMES

Job # 98119

Date: 10:38AM, 13 JUN 01

Scope :

6/12

Rev: 510301
 User: KW-0602320, Ver 5.1.3, 22-Jun-1999, Win32
 (c) 1983-99 ENERCALC

General Timber Beam

Page 2

j:\jobfiles\98119\enercalc\98119.ecw:Calculat

Description PLAN 3 SIDE LOAD BEAM #19

Stress Calcs

Bending Analysis

Ck 21.298 Le 0.000 ft Sxx 224.000 in3 Area 84.000 in2
 1.000 Rb 0.000 Cl 0.000

	Max Moment	Sxx Req'd	Allowable fb
@ Center	39.87 k-ft	165.00 in3	2,900.00 psi
@ Left Support	0.00 k-ft	0.00 in3	2,900.00 psi
@ Right Support	0.00 k-ft	0.00 in3	2,900.00 psi

Shear Analysis

	@ Left Support	@ Right Support
Design Shear	5.91 k	7.79 k
Area Required	20.394 in2	26.870 in2
Fv: Allowable	290.00 psi	290.00 psi

Bearing @ Supports

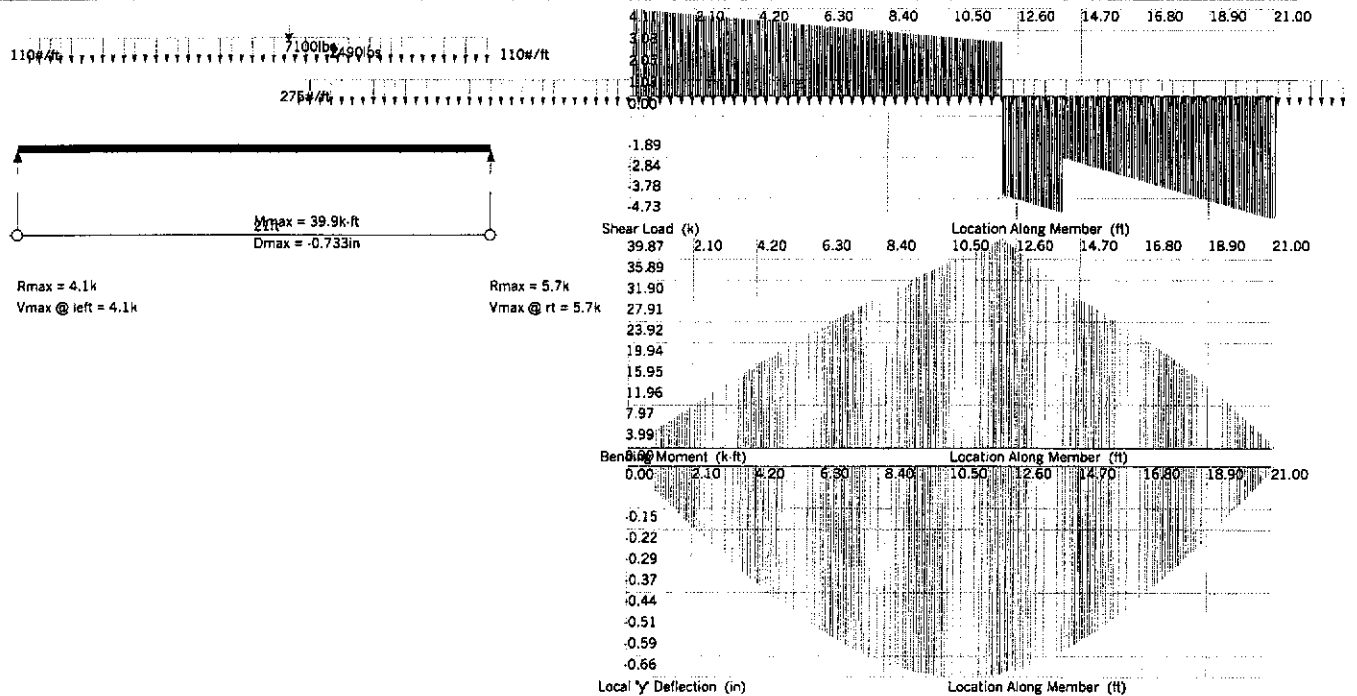
		Bearing Length Req'd
Max. Left Reaction	4.11 k	1.203 in
Max. Right Reaction	5.71 k	1.672 in

Query Values

M, V, & D @ Specified Locations

	Location	Moment	Shear	Deflection
@ Center Span Location =	0.00 ft	0.00 k-ft	4.11 k	0.0000 in
@ Right Cant. Location =	0.00 ft	0.00 k-ft	0.00 k	0.0000 in
@ Left Cant. Location =	0.00 ft	0.00 k-ft	0.00 k	0.0000 in

Sketch & Diagram



NORMAN SCHEEL
STRUCTURAL ENGINEER
5022 SUNRISE BLVD.
FAIR OAKS, CA 95628
(916) 536-9585

Title : CAPITAL PARK Job # 98119
Dsgnr: DP Date: 10:40AM, 13 JUN 01
Description : LEXINGTON HOMES
Scope :

7/12

Rev: 510901
User: KW-0602320, Ver 5.1.3, 22-Jun-1999, Win32
(c) 1983-99 ENERCALC

General Timber Beam

Page 1

j:\jobfiles\98119\enercalc\98119.ecw:Calculat

Description PLAN 3 SIDE LOAD BEAM #20 GARAGE DOOD HEADER

General Information Calculations are designed to 1997 NDS and 1997 UBC Requirements

Section Name	5 1/4 x 16	Center Span	16.00 ftLu	0.00 ft
Beam Width	5.250 in	Left Cantilever	ftLu	0.00 ft
Beam Depth	16.000 in	Right Cantilever	ftLu	0.00 ft
Member Type		Truss Joist - MacMillan, Parallam 2.0E			
Load Dur. Factor	1.000	Fb Base Allow	2,900.0 psi		
Beam End Fixity	Pin-Pin	Fv Allow	290.0 psi		
		Fc Allow	650.0 psi		
		E	2,000.0 ksi		

Full Length Uniform Loads

Center	DL	382.00 #/ft	LL	516.00 #/ft
Left Cantilever	DL	#/ft	LL	#/ft
Right Cantilever	DL	#/ft	LL	#/ft

Point Loads

Dead Load	4,110.0 lbs	lbs	lbs	lbs	lbs	lbs	lbs
Live Load	lbs	lbs	lbs	lbs	lbs	lbs	lbs
...distance	4.000 ft	0.000 ft	0.000 ft	0.000 ft	0.000 ft	0.000 ft	0.000 ft

Summary

Beam Design OK

Span= 16.00ft, Beam Width = 5.250in x Depth = 16.in, Ends are Pin-Pin

Max Stress Ratio	0.694	:	1			
Maximum Moment	37.5 k-ft			Maximum Shear * 1.5	15.4 k	
Allowable	54.1 k-ft			Allowable	24.4 k	
Max. Positive Moment	37.54 k-ft	at	6.848 ft	Shear:	@ Left	10.27 k
Max. Negative Moment	0.00 k-ft	at	16.000 ft		@ Right	8.21 k
Max @ Left Support	0.00 k-ft			Camber:	@ Left	0.000 in
Max @ Right Support	0.00 k-ft				@ Center	0.411 in
Max. M allow	54.13				@ Right	0.000 in
fb	2,011.27 psi	Fv	183.33 psi	Reactions...	Left DL	6.14 k
Fb	2,900.00 psi	Fv	290.00 psi		Right DL	4.08 k
					Max	10.27 k
					Max	8.21 k

Deflections

Center Span...	Dead Load	Total Load	Left Cantilever...	Dead Load	Total Load
Deflection	-0.274 in	-0.486 in	Deflection	0.000 in	0.000 in
...Location	7.616 ft	7.808 ft	...Length/Defl	0.0	0.0
...Length/Defl	700.2	394.95	Right Cantilever...		
			Deflection	0.000 in	0.000 in
			...Length/Defl	0.0	0.0

Stress Calcs

Bending Analysis

Ck	21.298	Le	0.000 ft	Sxx	224.000 in3	Area	84.000 in2
	1.000	Rb	0.000	CI	0.000		
			Max Moment		Sxx Req'd		Allowable fb
@ Center			37.54 k-ft		155.35 in3		2,900.00 psi
@ Left Support			0.00 k-ft		0.00 in3		2,900.00 psi
@ Right Support			0.00 k-ft		0.00 in3		2,900.00 psi

Shear Analysis

	@ Left Support	@ Right Support
Design Shear	15.40 k	12.32 k
Area Required	53.103 in2	42.473 in2
Fv: Allowable	290.00 psi	290.00 psi

Bearing @ Supports

Max. Left Reaction	10.27 k	Bearing Length Req'd	3.008 in
Max. Right Reaction	8.21 k	Bearing Length Req'd	2.406 in

NORMAN SCHEEL
STRUCTURAL ENGINEER
5022 SUNRISE BLVD.
FAIR OAKS, CA 95628
(916) 536-9585

Title : CAPITAL PARK
 Dsgnr: DP
 Description : LEXINGTON HOMES

Job # 98119

Date: 10:40AM, 13 JUN 01

Scope :

Rev: 510301
 User: KW-D602320, Ver 5.1.3, 22-Jun-1999, Win32
 (c) 1983-99 ENERCALC

General Timber Beam

Page 2

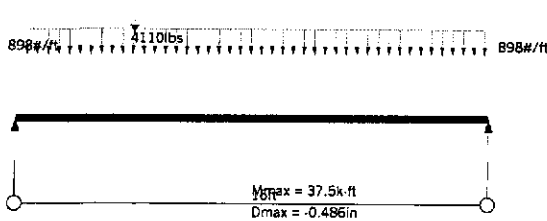
j:\jobfiles\98119\enercalc\98119.ecw:Calculat

Description PLAN 3 SIDE LOAD BEAM #20 GARAGE DOOD HEADER

Query Values

M, V, & D @ Specified Locations	Moment	Shear	Deflection
@ Center Span Location =	0.00 ft	0.00 k-ft	10.27 k
@ Right Cant. Location =	0.00 ft	0.00 k-ft	0.00 k
@ Left Cant. Location =	0.00 ft	0.00 k-ft	0.00 k
			0.0000 in
			0.0000 in
			0.0000 in

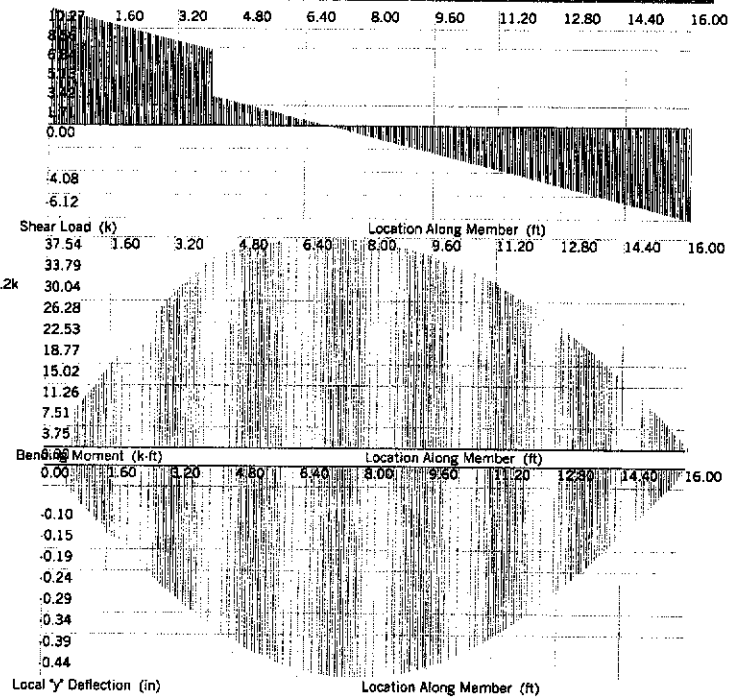
Sketch & Diagram



Rmax = 10.3k
 Vmax @ left = 10.3k

Mmax = 37.5k-ft
 Dmax = -0.486in

Rmax = 8.2k
 Vmax @ rt = 8.2k



Norman Scheel
Structural Engineer
5022 Sunrise Boulevard
Fair Oaks, CA 95628
(916) 536-9585

Project CAPITAL PARK
Client CAPITAL PARK HOMES
Job # 98119 **Project Mngr.** ROB
Date 6/13/01
Page # 48 **OF** 58

Lateral Analysis Calculations

P12 REAR OF HOUSE AT GARAGE

Wind Load Calculations

Total Width of Diaphragm	=	27 ft
Additional Diaphragm Width	=	0 ft
Plate Height	=	9 ft
Average Height above Plate	=	5 ft
Average Building Height	=	15 ft
Wind Load	=	15.35 psf
Additional Load	=	2149 #
Total Wind Load	=	4118 #

Seismic Load Calculation

Total Width of Diaphragm	=	27 ft
Total Length of Diaphragm	=	22 ft
Additional Diaphragm Width	=	0 ft
Additional Diaphragm Length	=	0 ft
Building dead load	=	26 #
V	=	0.1403 #
Additional Load	=	1605 #
Seismic load	=	2688 #
P	=	1.0381
Total Seismic Load	=	2790 #

Diaphragm Calculations

Length of Diaphragm	=	22 ft
Sheathing	=	15/32 in
Additional Load @ Diaphragm	=	0 #
Diaphragm Case (1 or 3)	=	3
Diaphragm Shear	=	89 plf

No Blocking Required

Shear Wall Calculations

# of Shear Walls	=	1
Total Length of Walls	=	22 ft
Base Shear	=	187 plf

USE SHEAR WALL TYPE H

Overturning Calculations

Length	Dead Load	Uplift Left	Location From Left	Uplift Right	Location From Left	Total Uplift Left	Total Uplift Right
22.0000	524					-2158	-2158

Shear Wall Specifications

#	Length	Type	Holdown Left	Holdown Right
SW1	22.0000	H	15	15

10/12

Norman Scheel
Structural Engineer
5022 Sunrise Boulevard
Fair Oaks, CA 95628
(916) 536-9585

Project CAPITAL PARK
Client CAPITAL PARK HOMES
Job # 98119 Project Mngr. ROB
Date 6/13/01
Page # 49 OF 58

Chord Force Calculation

P12 REAR OF HOUSE AT GARAGE

Total Wind Load = 3208 # Total Seismic Load = 2166 #

Load	Length	Depth	CT	Splice Specification
3208	22	27	327	Use 24-16d Nails each side of each splice.

Chord Force @ X =	0.0	0
Chord Force @ X =	0.0	0
Chord Force @ X =	0.0	0
Chord Force @ X =	0.0	0
Chord Force @ X =	0.0	0

Drag Strut Calculation

Diaphragm Shear = 89 plf Base Shear = 187 plf

SW #	S.W. Length	Location
SW1	22.0000	0.0000

Strap #	Location	Drag Force	Strap Specification
		0.00	
		0.00	
		0.00	
		0.00	
		0.00	

11/12

Norman Scheel
 Structural Engineer
 5022 Sunrise Boulevard
 Fair Oaks, CA 95628
 (916) 536-9585

Project CAPITAL PARK
 Client CAPITAL PARK HOMES
 Job # 98119 Project Mngr. ROB
 Date 6/13/01
 Page # 50 OF 58

Lateral Analysis Calculations

P13 LEFT SIDE OF HOUSE FIRST FLOOR

Wind Load Calculations

Total Width of Diaphragm = 22 ft
 Additional Diaphragm Width = 0 ft
 Plate Height = 9 ft
 Average Height above Plate = 5 ft
 Average Building Height = 15 ft
 Wind Load = 15.35 psf
 Additional Load = 3293 #

 Total Wind Load = 4897 #

Seismic Load Calculation

Total Width of Diaphragm = 22 ft
 Total Length of Diaphragm = 54 ft
 Additional Diaphragm Width = 0 ft
 Additional Diaphragm Length = 0 ft
 Building dead load = 26 #
 V = 0.1403 #
 Additional Load = 3009 #
 Seismic load = 5175 #
 p = 1.0000
 Total Seismic Load = 5175 #

Diaphragm Calculations

Length of Diaphragm = 54 ft
 Sheathing = 15/32 in
 Additional Load @ Diaphragm = 0 #
 Diaphragm Case (1 or 3) = 3

 Diaphragm Shear = 40 plf

No Blocking Required

Shear Wall Calculations

of Shear Walls = 1
 Total Length of Walls = 27 ft

 Base Shear = 192 plf

USE SHEAR WALL TYPE H

Overtuning Calculations

Length	Dead Load	Uplift Left	Location From Left	Uplift Right	Location From Left	Total Uplift Left	Total Uplift Right
27.0000	424					-2091	-2091

Shear Wall Specifications

#	Length	Type	Holdown Left	Holdown Right
SW1	27.0000	H	NOT REQUIRED	NOT REQUIRED

Norman Scheel
 Structural Engineer
 5022 Sunrise Boulevard
 Fair Oaks, CA 95628
 (916) 536-9585

Project CAPITAL PARK
 Client CAPITAL PARK HOMES
 Job # 98119 Project Mngr. ROB
 Date 6/13/01
 Page # 51 OF 58

Chord Force Calculation

P13 LEFT SIDE OF HOUSE FIRST FLOOR

Total Wind Load = 7875 # Total Seismic Load = 4332 #

Load	Length	Depth	CT	Splice Specification
7875	54	22	2416	Use 24-16d Nails each side of each splice.

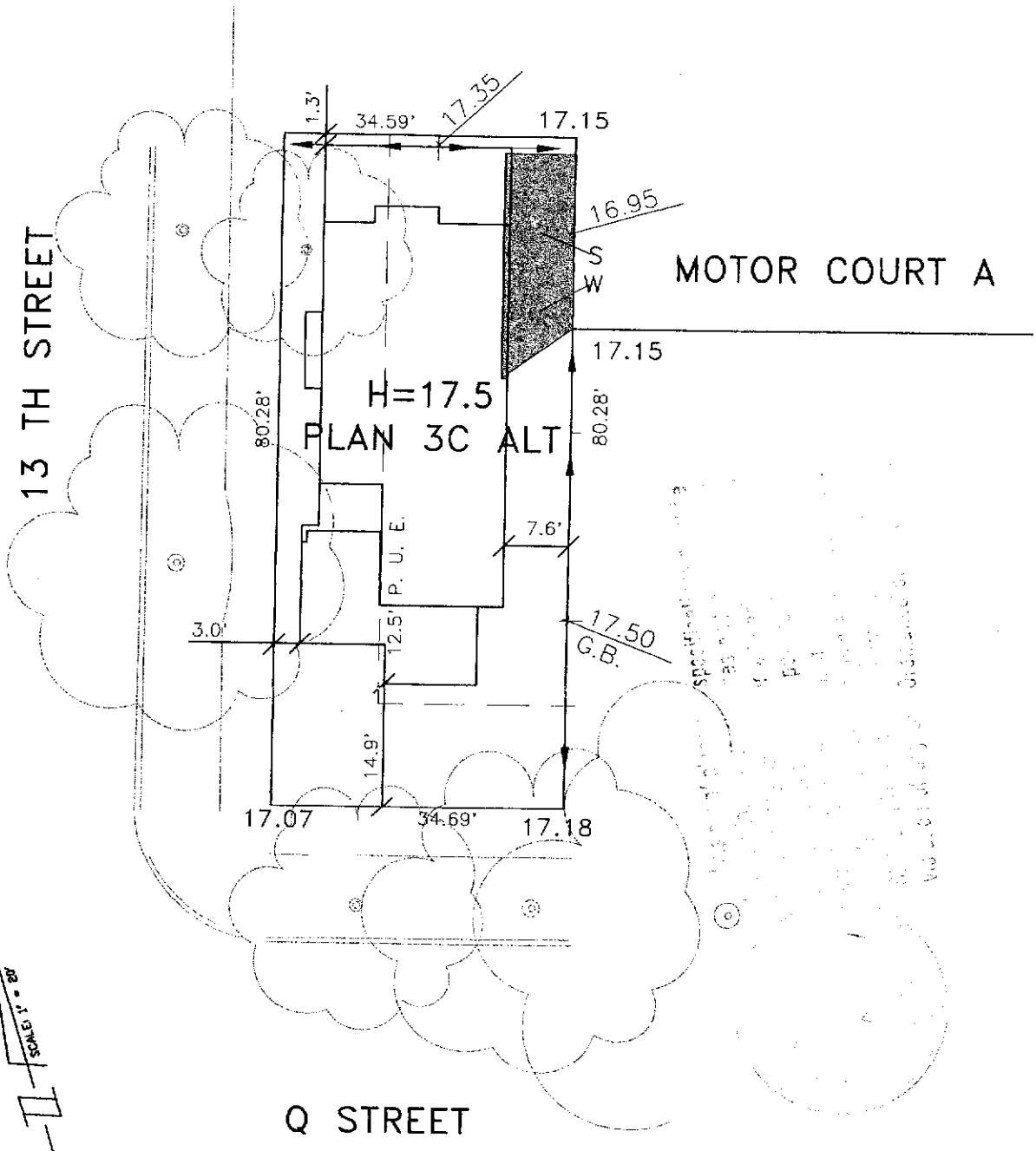
Chord Force @ X =	0.0	0
Chord Force @ X =	0.0	0
Chord Force @ X =	0.0	0
Chord Force @ X =	0.0	0
Chord Force @ X =	0.0	0

Drag Strut Calculation

Diaphragm Shear = 40 plf Base Shear = 192 plf

SW #	S.W. Length	Location
SW1	27.0000	0.0000

Strap #	Location	Drag Force	Strap Specification
		0.00	
		0.00	
		0.00	
		0.00	
		0.00	



NOTE:

This plan is only intended to reference the building plan designated for this lot. All setbacks other than minimum code requirements are approximate. The "as-built" field condition may vary from this plan.

LOT AREA: 2,781 SF
 LOT COVERAGE: 42%

DATE: 10-06-00
 A.P.N.: 006-2840-045
 ADDRESS: 1301 Q STREET

The Spink Corporation
 2590 VENTURE OAKS WAY
 SACRAMENTO, CA. 95833
 PH (916)925-5550 FAX (916)921-9274

CAPITOL PARK
 LOT 35
 PLAN 3C

CAPITOL PARK
 CITY OF SACRAMENTO, CA
 CLIENT: CNM CONSTRUCTION
 JOB NO.: 1222-002