



WALLACE - KUHL
& ASSOCIATES INC.

January 24, 2005

FEB 01 2005

Duarte Construction
PO Box 1260
Loomis, CA 95650

Special Inspection Final Report
CORTI BROTHERS ROOF REPAIR
Folsom Boulevard, Sacramento, California
Permit No. 04-16190
WKA No. 6106.81

In accordance with City of Sacramento special inspection requirements, our firm has performed *Special Inspection and Testing* in accordance with Sections 106 and 1701 of the Uniform Building Code for the subject project. Our observation and test results indicate that the following items were constructed, to the best of our knowledge, in accordance with the project's plans and specifications:

Concrete: Inspected placement of reinforcing steel and anchor bolts, and placement of structural repair mortar for column cap repair.
Sampled mortar and performed laboratory compression tests.

NOTE: The sample of mortar exceeded the design strength when tested at age 7 days. 28-day tests have not been performed as of this date.

Epoxy Dowels: Inspected installation of epoxied rebar dowels per manufacturers instructions and the project plans.

Tension Proof Load Tests: Performed tension proof load tests on epoxied 5/8" allthread hold-down anchors in south wall at roof level.

Structural Steel: Performed field welding inspection of fillet welds at beam bucket at west wall. Monitored contractor compliance with Welding Procedure Specifications (WPS). Checked welder certification records.

Last date at jobsite: January 14, 2005

Wallace - Kuhl & Associates, Inc.

David A. Redford, P.E.
Senior Engineer

cc: City of Sacramento



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COMPRESSION TEST REPORT

Report to:
ATTN:

DUARTE CONSTRUCTION
P.O. BOX 1260
LOOMIS, CA 95650

Date: 1/21/05

WKA Project No.: 6106.81

Project Name:

CORTI BROTHERS
5810 FOLSOM BLVD.
SACRAMENTO, CALIFORNIA

Mix Design No.: STRUCT REPAIR MORTAR

Supplier: MEADOW-CRETE

Location in Structure:

CONCRETE COLUMN REPAIR,
WEST WALL NEAR SOUTH END

Design Strength: 2,500 psi

Design Age: 28 days

Specimen: MORTAR

Air Content:

Mix Temp. 60 °F

Air Temp. 45 °F

| SPECIMEN LAB NO. | 1983 | 1984 | 1985 |
|-------------------|---------|---------|---------|
| Date Cast | 1/14/05 | 1/14/05 | 1/14/05 |
| Date Received | 1/15/05 | 1/15/05 | 1/15/05 |
| Date Tested | 1/21/05 | 2/11/05 | 2/11/05 |
| Age at Test, days | 7 | 28 | 28 |
| Diameter, in. | 2.0 | | |
| Height, in. | 4.0 | | |
| Area, sq. inches | 3.14 | | |
| Ult. Load, lbs. | 24140 | | |
| Comp. Str., psi | 7680 | | |
| Fracture Type | A | | |

Meets 28 day strength req.

Fails to Meet 28 day str. req.

No strength req. given

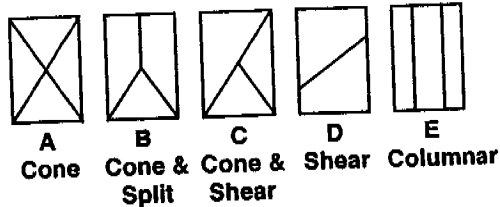
Cast by WKA

Cast by others

Remarks: @ 7 DAYS

Copies to:

TYPES OF FRACTURE



Reviewed by: DAR

Dave Redford

TEST METHODS: Compressive Strength - ASTM C39,
Air Content - ASTM C173 or C231

Memo

To: Justin Carmichael – Duarte Construction
From: Peter Sandlin
Date: 11/2/2004
Re: Corti Brothers – Roof Repair

Per our discussion yesterday, we understand the concrete pilaster repair shown in detail 1-S4 will be chipped downward 10" compared with the shown 12" dimension. Provided the epoxy hoop dowels are adequately secured to the existing wall panel, our office does not take exception to the proposed reduced 10" measurement.

Additionally, you inquired if the new ledgers' end bolt distance could be increased beyond the 12" maximum depicted in detail 5-S3. Considering the four new bolts installed at each 4x14 purlin per 5-S3 and the roof framing configuration, our office believes the ledgers' end bolts can be moved backward up to 20" from the splice point. Place three bolts minimum per new ledger section.

Please call our office (916) 418-9100 with any questions or comments you may have.

Thank you,

Peter Sandlin, P.E.





WALLACE - KUHL & ASSOCIATES INC.
 GEOTECHNICAL ENGINEERING • CONSTRUCTION TESTING

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| | | | | | | | | | |
|--|--|--|--------|--|--------|--|---------|-------|--|
| DATE 10-26-04 | | JOB NO. 6106.81 | | WEATHER | | TEMP. ° at | | AM | |
| PROJECT Corti Bras | | Technician I <input type="checkbox"/> | | Staff E/G <input type="checkbox"/> | | | | | |
| LOCATION 59th + Folsom | | Technician II <input type="checkbox"/> | | Project E/G <input type="checkbox"/> | | | | | |
| TYPE OF WORK pull test | | Technician III <input type="checkbox"/> | | Senior E/G <input type="checkbox"/> | | | | | |
| Inside 50 mi. radius <input checked="" type="checkbox"/> | | Outside 50 mi. radius <input type="checkbox"/> | | Nuclear Densities <input type="checkbox"/> | | Principal E/G <input type="checkbox"/> | | | |
| PERSONNEL | | REG. HRS | OT HRS | TOTAL HRS | TRAVEL | ON JOB | VEHICLE | MILES | |
| Aaron Schmitt | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |

OBSERVATIONS: Pull tested 2 5/8" all thread anchor bolts for #1772 hold downs. Anchors were located in south wall at roof level. Anchors were tested to 6000 pounds of tension per request of city building inspector. Both tests passed.

Also verified dimensions of newly installed glue lambs. All was per plans 6 3/4" x 21" x 44"

FIELD REPORT

Signed Aaron Schmitt