

CITY OF SACRAMENTO
1231 I Street, Sacramento, CA 95814

Permit No: 0106290
Insp Area: 2

Site Address: 813 WEST COVE WY SAC
Parcel No: 031-1410-018

Sub-Type: NSFR
Housing (Y/N): N

CONTRACTOR
GROUP ONE BUILDERS

OWNER
SAKAI YI
81 WEST COVE WAY
SACRAMENTO CA 95833

ARCHITECT

Nature of Work: NEW 2 - STORY SFR

CONSTRUCTION LENDING AGENCY: I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, C.C.P.C.)

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 commencing with section 7000 of Division 8 of the Business and Professions Code and my license is in full force and effect.

License Class B License Number 66896 Date 7/30/01 Contractor Signature Robert Lockett

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.2, Business and Professions Code, any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than one hundred dollars (\$100.00).

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professions Code). The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code). The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law.

I am exempt under Sec. _____ B & P. for this reason: _____

Date _____ Owner Signature _____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 7/30/01 Applicant Agent Signature Robert Lockett

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier LEGION INSURANCE Policy Number WC11617157 Exp Date 01/01/2002

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 7/30/01 Applicant Signature Robert Lockett

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

Certification of Compliance
School District Development

Part I - To be completed by the APPLICANT

Owner's Name/Address MARK & TAMI SAKAUE / 1125CA Pyrites Way #7
Gold Riv. 94670

Project Address 813 West Cove Ave

Parcel Number 031-1410-018 Lot No. 18

Subdivision Name West Shore @ Riverlake No. of Units 1

Applicant's Signature Robert J. Jockel Title Agent

Phone No. 916.852.0300 Date 7/30/01

Notice to Applicant: Pursuant to Government Code Section 66020(d), this will serve to notify you that the 90-day approval period in which you may protest the fees or other payment identified above will begin to run on the date in which the building or installation permit for this project is issued or on which they are paid to the district(s) or to another public entity authorized to collect them on behalf of the district(s), whichever is earlier.

Part II - To be completed by the BUILDING DEPARTMENT

Plan Identification Number 0106290

Building Type (check one) Residential Apartment/Condominium Commercial/Industrial

Square Feet of Chargeable Building Area 3,216 #

Signature/Title Ben Maynard Bldg Tech Date 7-30-01

Part III - To be completed by the SCHOOL DISTRICT

School District _____ Certificate No. _____

Exempt Comments _____

Residential/Apartment/etc. _____ Square ft. x \$ _____ = \$ 5531.52

Commercial/Industrial _____ Square ft. x \$ _____ = \$ _____

Total fees collected..... = \$ 4662.52

This certification covers only the amount of square footage indicated above. Any additions or corrections to the square footage for this project will require an amendment to the Certificate of Compliance.

As the authorized school official, I hereby certify that the requirements of Government Code Section 65995 and any other authorized requirements have been complied with by the above signed applicant.

Signature _____ Date 7/30/01

Date of Request: _____
By: _____

**CITY OF SACRAMENTO DEVELOPMENT SERVICES DIVISION
PLANNING AND ZONING INFORMATION REQUEST**

Project
Address: 813 West Cove Way

Assessor's Parcel Number: 031-1410-018

Previous Use: VACANT

Description of Request/Proposed Use: New Single Family Dwelling

9272

Is This a Change of Use? YES

Prior Applications for Project Site(P#, Z#, DRPB#): NONE Zoning Designation: R-1 PUD

Comments: LOFT PLAN

LOT COVERAGE & SETBACKS & HEIGHT
OKAY

Are There Any Planning Issues?: (circle one) YES NO

- * Staff Site Plan Check Required? (Circle one) YES NO
- * Field Inspection Required? (Circle one) YES NO
- * Design Review/Preservation Required?: (Circle one) YES NO

Planning Review by/Date: Michael York 5-17-01

A list of items that must be reviewed by Planning is provided on the reverse side of this form.

MICROFILM AFTER FINAL

EL DORADO TRUSS COMPANY, INC.

300 Industrial Drive
Placerville, CA 95682
(530) 622-1264; Fax 622-0242

NAME GROUP ONE SAKOYE RESIDENCE

JOB SAKOYE PAGE 2 OF 21

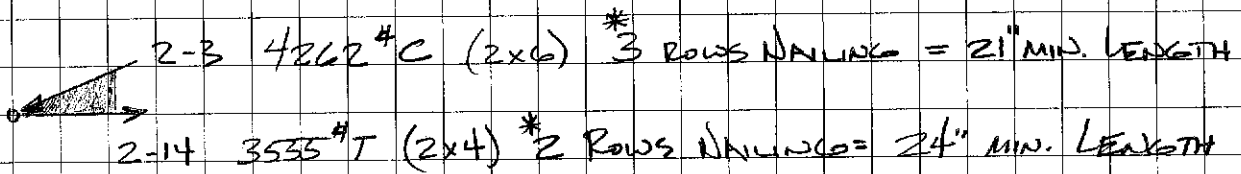
BY D. LARRY PETITE REV _____

SCALE _____ DATE 9.25.01

1.2 TRUSS A (REFER AS A2) GUSSET DESIGN

SEE ATTACHED "Plywood GUSSET DESIGN" 2 PAGES*

HEEL JOINTS 2 AND 9: (SYMMETRICAL TRUSS A2)



2-3 4262 #C (2x6) * 3 ROWS NAILING = 21" MIN. LENGTH
2-14 3535 #T (2x4) * 2 ROWS NAILING = 24" MIN. LENGTH

USE: 24" LONG GUSSET ALONG BOTTOM CHORD

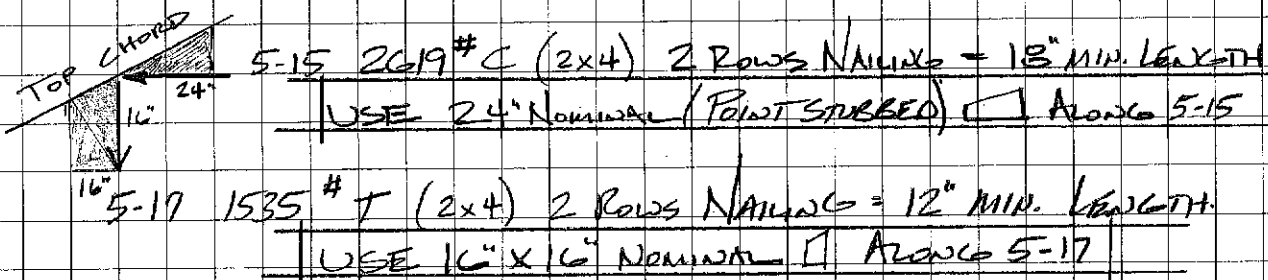
TOP CHORD JOINTS 3 AND 8:

(E) PLATE TRUSS A = 3x6 ; TRUSS A2 = 3x6 OK

TOP CHORD SPACE JOINT 4:

(E) PLATE TRUSS A = 4x6 ; TRUSS A2 = 4x6 OK

TOP CHORD JOINTS 5 AND 7:



5-15 2619 #C (2x4) 2 ROWS NAILING = 18" MIN. LENGTH
USE 24" NOMINAL (POINT STRIPPED) □ ALONG 5-15
5-17 1535 #T (2x4) 2 ROWS NAILING = 12" MIN. LENGTH
USE 16" X 16" NOMINAL □ ALONG 5-17

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JOB SAKOYE PAGE 3 OF 21

BY D. LARRY PETITE REV _____

SCALE _____ DATE 9.25.01

12 TRUSS A (REFER AS A2) GUSSET DESIGN - CONT.

TOP CHORD PEAK JOINT 6:

(E) PLATE TRUSS A = 5X6; TRUSS A2 = 5X6 OK

COURT TIE - KING POST JOINT 15:

↑ 6-15 287# T (2x4) 2 ROW NAILING = 6" MIN LENGTH
12" 12"

USE 12" X 12" GUSSETS AT JOINT 15

K-WEB JOINTS 16 AND 17: (SYMMETRICAL)

3-17 269# C (2x4) 2 ROW NAILING = 6" MIN. LENGTH
17
14-17 273# T (2x4) 2 ROW NAILING = 6" MIN. LENGTH

USE 12" X 12" GUSSETS AT JOINTS 16 AND 17

BOTTOM CHORD JOINTS 10 AND 14: (SYMMETRICAL)

SIMILAR TO 14-17 ABOVE ∴ USE

BOTTOM CHORD JOINTS 11 AND 13: (SYMMETRICAL)

↑ 16 13-17 1240# T (2x4) 2 ROW NAILING = 9" MIN. NAILING
16"

USE 16" X 16" GUSSETS AT JOINTS 11 AND 13

BOTTOM CHORD SPLICE JOINT 12: (E) 3X4 PLATE PERIOD 5X15

3,311# T

USE 2X8 SCAB ONE SIDE; SIMPSON LSTI 73 OPP. SIDE

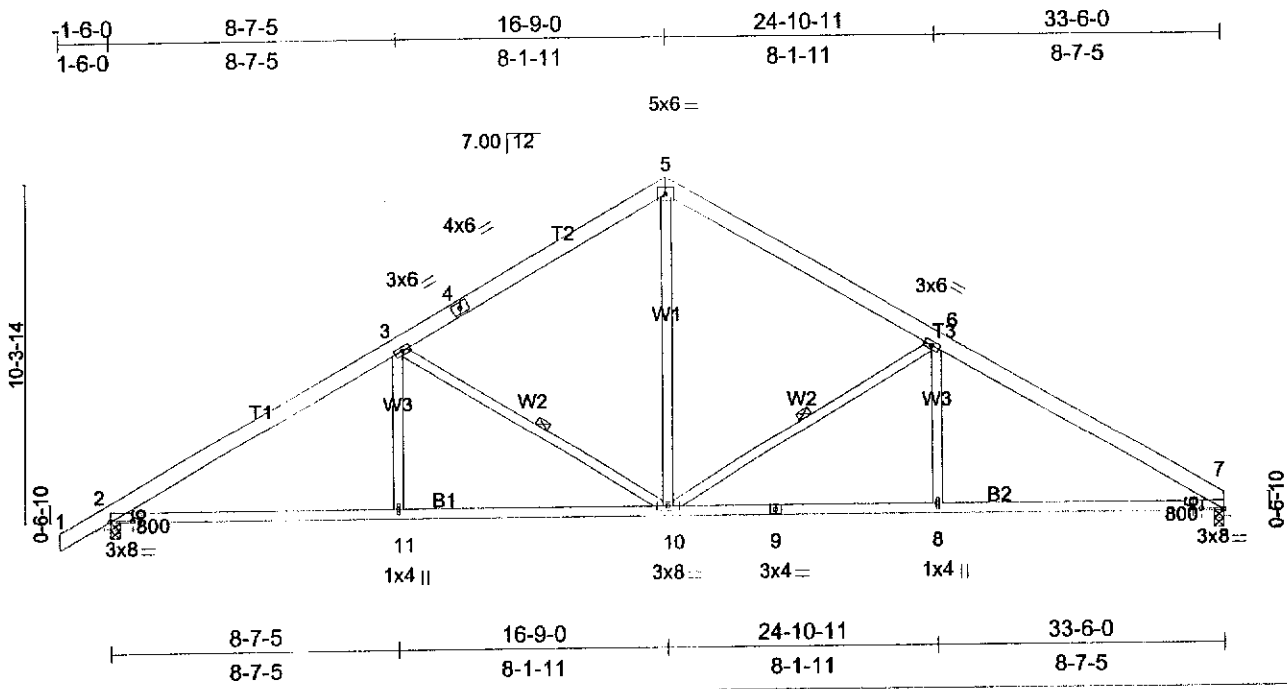


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [7:0-8-0,0-0-6]					
LOADING (psf)	SPACING	CSI	DEFL		PLATES GRIP
TCLL 20.0	2-0-0	TC 0.39	(ln) (loc) l/defl	M20	220/195
TCDL 15.0	Plates Increase 1.15	BC 0.77	Vert(LL) -0.13 2-11 >999		
BCLL 0.0	Lumber Increase 1.15	WB 0.51	Vert(TL) -0.31 8-10 >999		
BCDL 10.0	Rep Stress Incr NO		Horz(TL) 0.08 7 n/a	Weight: 184 lb	
	Code UBC97/ANSI95		1st LC LL Min l/defl = 360		

LUMBER
TOP CHORD 2 X 6 DF SS-G
BOT CHORD 2 X 4 DF No.1&Btr-G
WEBS 2 X 4 DF Stud/Std-G

BRACING
TOP CHORD Sheathed or 5-3-10 on center purlin spacing.
BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.
WEBS 1 Row at midpt 3-10, 6-10

REACTIONS (lb/size) 2=1810/0-3-8, 7=1494/0-3-8
Max Horz 2=21(load case 5)
Max Uplift 2=-127(load case 5), 7=-104(load case 5)
Max Grav 2=1634(load case 2), 7=1494(load case 1)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=23, 2-3=-2139, 3-4=-1449, 4-5=-1449, 5-6=-1449, 6-7=-2139
BOT CHORD 2-11=1826, 10-11=1826, 9-10=1826, 8-9=1826, 7-8=1826
WEBS 3-11=166, 5-10=890, 6-8=166, 3-10=-679, 6-10=-679

- NOTES**
- 1) This truss has been designed for the loads generated by 85 mph winds at 25 ft above ground level located 100 mi from the hurricane oceanline. ASCE 7-95 components and cladding external pressure coefficients for the interior (1) zone and 7.0 psf top chord and 7.0 psf bottom chord dead load are being used. The design assumes occupancy category II, terrain exposure C and internal pressure coefficient condition 1. The building dimensions are 45 ft by 24 ft. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 2) Unbalanced snow loads have been considered for this design.
 - 3) All plates are M20 plates unless otherwise indicated.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - 5) A plate rating reduction of 20% has been applied for the green lumber members.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 2 and 104 lb uplift at joint 7.
 - 7) This truss has been designed with ANSI/TPI 1-1995 criteria.
 - 8) Overhang snow load factor (2.00)

LOAD CASE(S) Standard



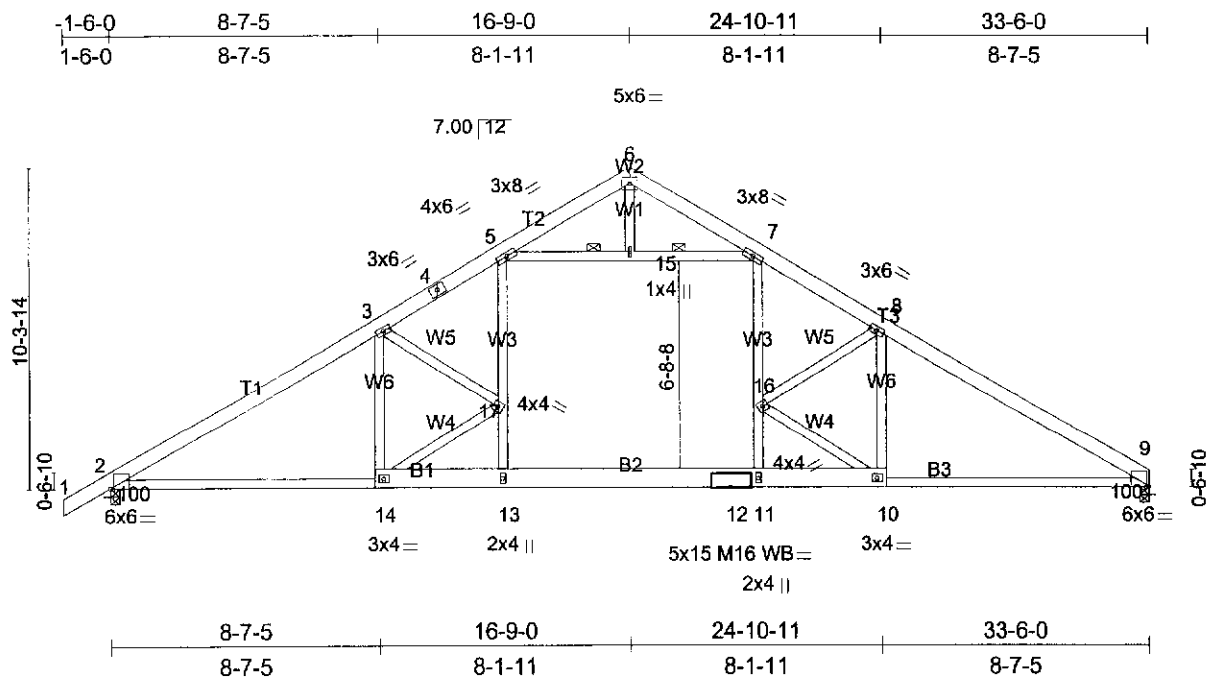


Plate Offsets (X, Y): [2'-0"-1'-0", edge], [9'-0"-1'-0", edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES GRIP
TCLL 20.0	2-0-0	TC 0.50	(in) (loc) l/defl	M20 220/195
TCDL 15.0	Plates Increase 1.15	BC 0.89	Vert(LL) -0.26 11-13 >999	M16 176/121
BCLL 0.0	Lumber Increase 1.15	WB 0.76	Vert(TL) -0.37 11-13 >999	Weight: 250 lb
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.15 9 n/a	
	Code UBC97/ANSI95		1st LC LL Min l/defl = 360	

LUMBER
TOP CHORD 2 X 6 DF SS-G
BOT CHORD 2 X 4 DF No.1 *Except*
WEBS 2 X 4 DF Stud/Std-G *Except*
LBR SCAB 10-14 2 X 8 DF X SS one side

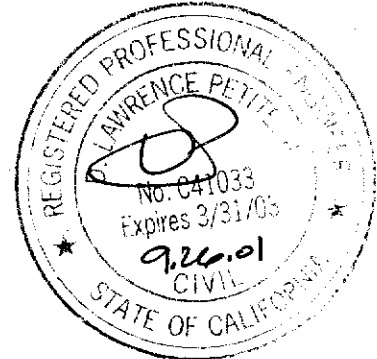
BRACING
TOP CHORD Sheathed or 3-6-0 on center purlin spacing.
BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.
WEBS 2 Rows at 1/3 pts 5-7

REACTIONS (lb/size) 9=2487/0-3-8, 2=2607/0-3-8
Max Horz 2=21 (load case 5)
Max Uplift 9=-20 (load case 5), 2=-45 (load case 5)
Max Grav 2=2633 (load case 2)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=47, 2-3=-4249, 3-4=-3963, 4-5=-3837, 5-6=-825, 6-7=-824, 7-8=-3968, 8-9=-4262
BOT CHORD 2-14=3539, 13-14=3310, 12-13=3311, 11-12=3311, 10-11=3310, 9-10=3555
WEBS 3-14=-218, 6-15=287, 8-10=-218, 8-16=-288, 13-17=1240, 5-17=1535, 11-16=1231, 7-16=1546, 10-16=292, 3-17=-269, 14-17=273, 5-15=-2619, 7-15=-2619

- NOTES**
- 1) This truss has been designed for the loads generated by 85 mph winds at 25 ft above ground level located 100 mi from the hurricane oceanline. ASCE 7-95 components and cladding external pressure coefficients for the interior (1) zone and 7.0 psf top chord and 7.0 psf bottom chord dead load are being used. The design assumes occupancy category II, terrain exposure C and internal pressure coefficient condition I. The building dimensions are 45 ft by 24 ft. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 2) Unbalanced snow loads have been considered for this design.
 - 3) All plates are M20 plates unless otherwise indicated.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - 5) Ceiling dead load (5.0 psf) on member(s), 5-6, 6-7, 5-15, 7-15
 - 6) Bottom chord live load (100.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room, 11-13
 - 7) A plate rating reduction of 20% has been applied for the green lumber members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 9 and 45 lb uplift at joint 2.
 - 9) This truss has been designed with ANSI/TPI 1-1995 criteria.
 - 10) Overhang snow load factor (2.00)

LOAD CASE(S) Standard

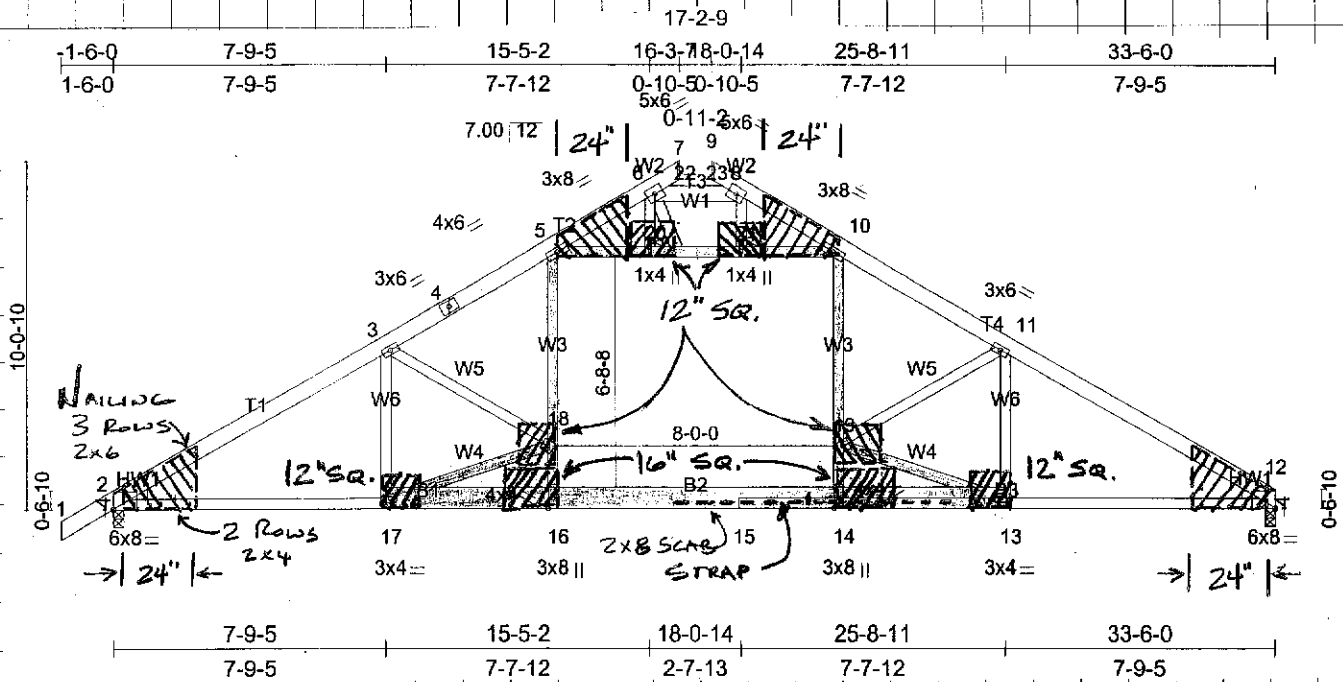


EL DORADO TRUSS COMPANY, INC.

300 Industrial Drive
 Placerville, CA 95682
 (530) 622-1264; Fax 622-0242

NAME GROUP ONE SAKOUYE RES.
 JOB SAKOUYE PAGE 6 OF 21
 BY D. LARRY PETTE REV _____
 SCALE _____ DATE 9.26.01

2.0 TRUSS A16 (RERUN AS A-16B - ATTACHED)

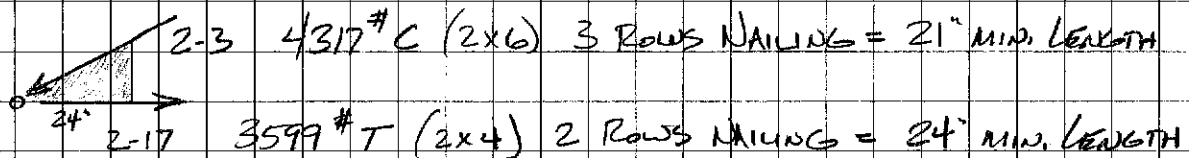


2.1 ADDITIONAL WEBS AND SCABS

- 2x4 DF N°1 E-BTR-G WEBS: (CONNECTED WITH PLYWOOD GUSSETS) 17-18 AND 13-19; 5-16 AND 10-14; AND 5-10
- 2x8 DFSS SCAB - ONE SIDE ONLY - 18'± 13-17
 FREE NAIL TO (E) 2x4 BOTTOM CHORD OVER PLYWOOD (FILL BETWEEN PLYWOOD GUSSETS): 16d SINKERS 4" O.C. STAGGERED

2.2 TRUSS A16 (RERUN AS A-16B) GUSSET DESIGNS

HEEL JOINTS 2 AND 12 (SYMMETRICAL)



USE: 24" LONG GUSSET ALONG BOTTOM CHORD EACH HEEL

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NAME GROUP ONE SAKOUYE RES.
 JOB SAKOUYE PAGE 7 OF 21
 BY D LARRY PETTE REV _____
 SCALE _____ DATE 9.26.01

2.2 TRUSS AIG (PERIOD AS A-16B) GUSSET DESIGNS CONT.

TOP CHORD JOINTS 3 AND 11:

(E) PLATE TRUSS AIG = 3x6; PERIOD TRUSS A-16B = 3x6 OK

TOP CHORD SPACE JOINT 4:

(E) PLATE TRUSS AIG = 4x6; PERIOD TRUSS A-16B = 4x6 OK

TOP CHORD JOINTS 5 AND 10:

USE 24" NOMINAL (POINT STUBBED) ALONG S-20
 2 ROWS NAILING = 18" MIN. LENGTH
 USE 16" x 16" NOMINAL ALONG S-18
 2 ROWS NAILING = 12" MIN. LENGTH

TOP CHORD PEAK FLAT JOINTS 6 AND 8

(E) PLATES TRUSS AIG = 5x6; PERIOD TRUSS A-16B = 5x6 OK

CEILING JOINTS 20 AND 21 (SYMMETRICAL)

186 # T (2x4) 2 ROWS NAILING = 6" MIN. LENGTH
 USE 12" x 12" AT JOINTS 20 AND 21

K-WEB JOINTS 18 AND 19 (SYMMETRICAL)

294 # C (2x4) 2 ROWS NAILING = 6" MIN. LENGTH
 275 # T (2x4) 2 ROWS NAILING = 6" MIN. LENGTH
 USE 12" x 12" AT JOINT 18

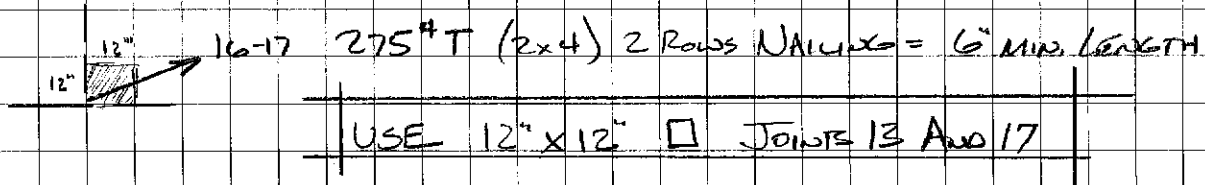
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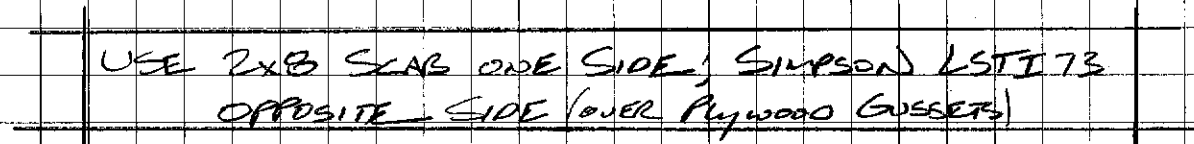
NAME GROUP ONE SAKOUYE RES.
JOB SAKOUYE PAGE 8 OF 21
BY D. LARRY PETITE REV _____
SCALE _____ DATE 9.26.01

212 TRUSS A16 (RERUN AS A-16B) GUSSET DESIGNS (CONT.)

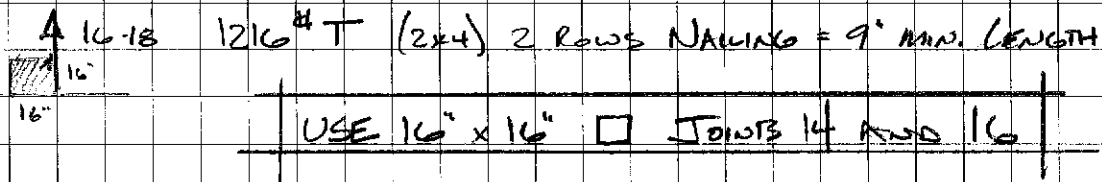
BOTTOM CHORD JOINTS 13 AND 17 (SYMMETRICAL)



BOTTOM CHORD SPLICE



BOTTOM CHORD JOINTS 14 AND 16



9 of 21

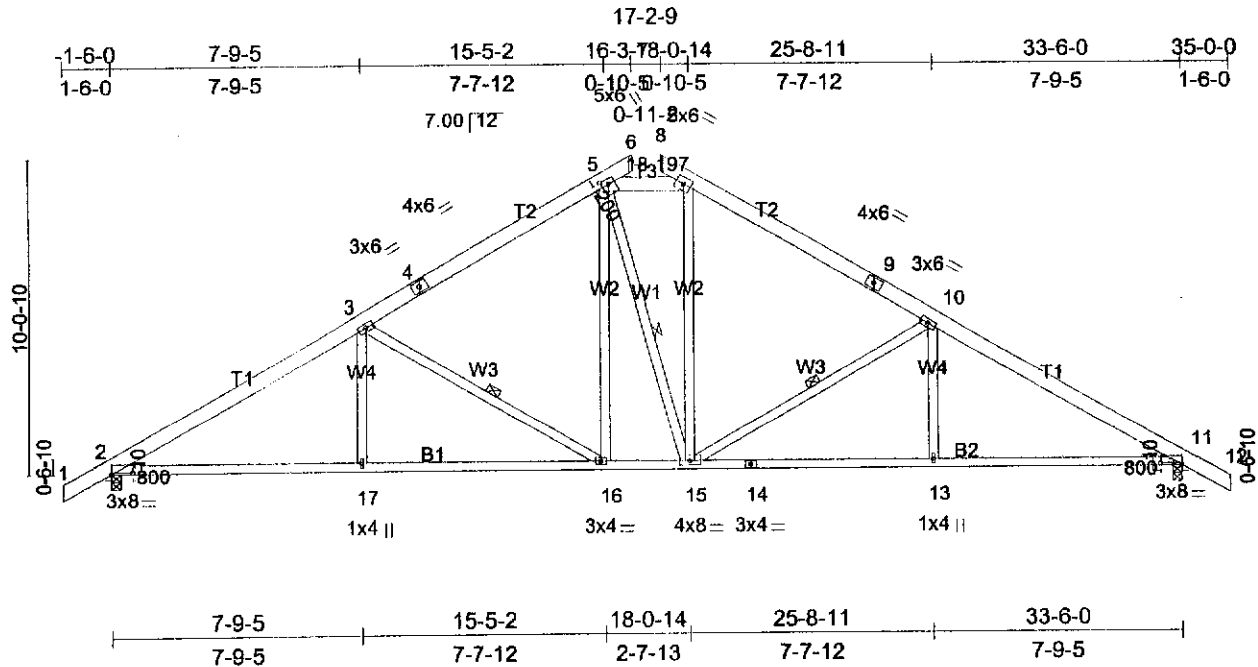


Plate Offsets (X,Y): [2'-0"-8'-0",0'-0"-10"], [5'-0"-2'-8",0'-2'-0"], [11'-0"-8'-0",0'-0"-10"]

LOADING (psf) TCLL 20.0 TCCL 15.0 BCLL 0.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr NO Code UBC97/ANSI95	CSI TC 0.30 BC 0.58 WB 0.43 (Matrix)	DEFL (in) (loc) l/defl Vert(LL) -0.08 16-17 >999 Vert(TL) -0.20 16-17 >999 Horz(TL) 0.08 11 n/a 1st LC LL Min l/defl = 360	PLATES GRIP M20 220/195 Weight: 211 lb
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LUMBER
 TOP CHORD 2 X 6 DF SS-G
 BOT CHORD 2 X 4 DF No.1&Btr-G
 WEBS 2 X 4 DF Std/Std-G

BRACING
 TOP CHORD Sheathed or 5-0-2 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.
 WEBS 1 Row at midpt 3-16, 5-15, 10-15

REACTIONS (lb/size) 2=1631/0-3-8, 11=1631/0-3-8
 Max Uplift 2=-141(load case 5), 11=-141(load case 5)
 Max Grav 2=1677(load case 2), 11=1677(load case 3)

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=47, 2-3=-2428, 3-4=-1790, 4-5=-1637, 5-6=-35, 5-18=-1440, 18-19=-1440, 7-19=-1440, 7-8=-35, 7-9=-1643, 9-10=-1783, 10-11=-2426, 11-12=47
 BOT CHORD 2-17=1991, 16-17=1991, 15-16=1434, 14-15=1989, 13-14=1989, 11-13=1989
 WEBS 3-17=171, 3-16=-645, 5-16=429, 5-15=21, 7-15=413, 10-15=-636, 10-13=170

- NOTES**
- 1) This truss has been designed for the loads generated by 85 mph winds at 25 ft above ground level located 100 mi from the hurricane oceanline. ASCE 7-95 components and cladding external pressure coefficients for the interior (1) zone and 7.0 psf top chord and 7.0 psf bottom chord dead load are being used. The design assumes occupancy category II, terrain exposure C and internal pressure coefficient condition I. The building dimensions are 45 ft by 24 ft. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 2) Unbalanced snow loads have been considered for this design.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) All plates are M20 plates unless otherwise indicated.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - 6) A plate rating reduction of 20% has been applied for the green lumber members.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 2 and 141 lb uplift at joint 11.
 - 8) This truss has been designed with ANSI/TPI 1-1995 criteria.
 - 9) Overhang snow load factor (2.00)

LOAD CASE(S) Standard



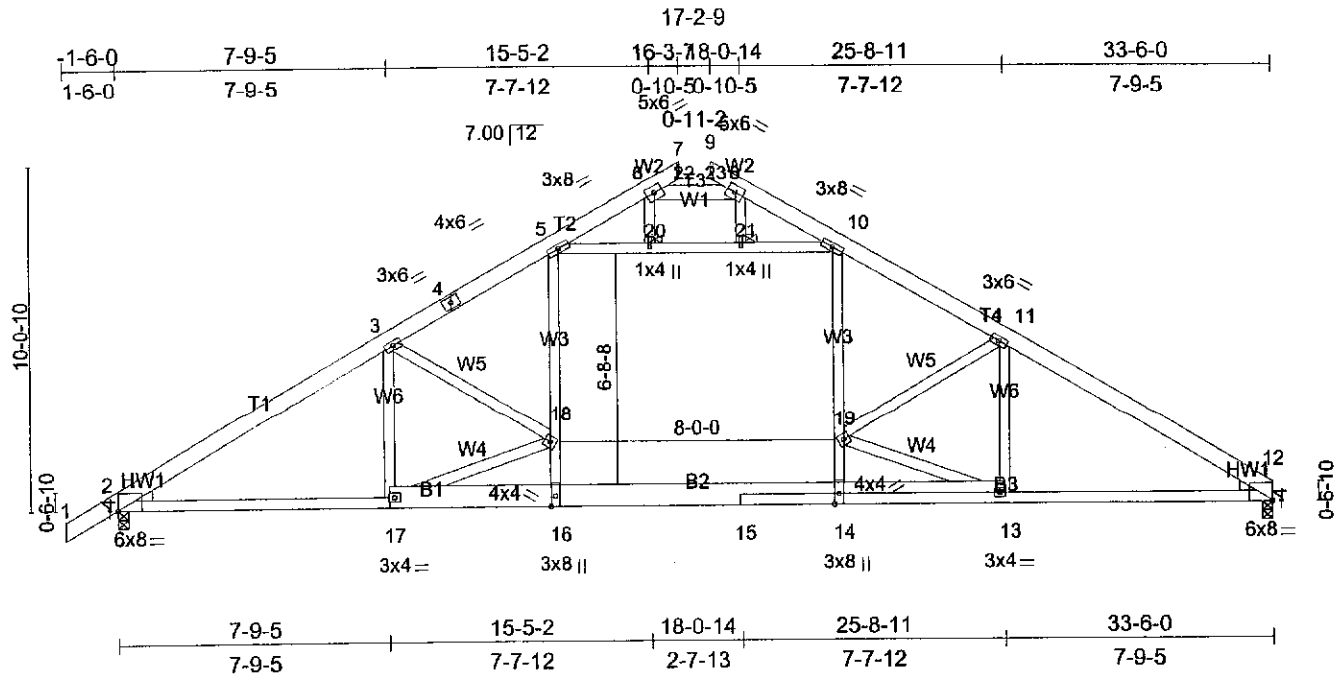


Plate Offsets (X,Y): [2:0-0-0, 0-0-4], [12:0-0-0, 0-0-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL (in) (loc)	PLATES GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.41	Vert(LL) -0.26 14-16 >999	M20 220/195
TCDL 15.0	Lumber Increase	1.15	BC 0.70	Vert(TL) -0.37 14-16 >999	
BCLL 0.0	Rep Stress Incr	NO	WB 0.72	Horz(TL) 0.15 12 n/a	
BCDL 10.0	Code	UBC97/ANSI95	(Matrix)	1st LC LL Min I/defl = 360	Weight: 261 lb

LUMBER
TOP CHORD 2 X 6 DF SS-G
BOT CHORD 2 X 4 DF No.1&Btr-G *Except*
B2 2 X 8 DF SS-G
WEBS 2 X 4 DF Stud/Std-G
LBR SCAB 13-17 2 X 8 DF X SS one side
WEDGE Left: 2 X 4 DF Stud/Std, Right: 2 X 4 DF Stud/Std

BRACING
TOP CHORD Sheathed or 3-7-2 on center purlin spacing.
BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.
WEBS 2 Rows at 1/3 pts 5-10

REACTIONS (lb/size) 12=2508/0-3-8, 2=2629/0-3-8
Max Horz 2=21 (load case 5)
Max Uplift 12=-34 (load case 5), 2=-59 (load case 5)
Max Grav 2=2673 (load case 2)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=47, 2-3=4317, 3-4=3998, 4-5=3875, 5-6=-1127, 6-7=-35, 6-22=-986, 22-23=-986, 8-23=-986, 8-9=-35, 8-10=-1125, 10-11=-4001, 11-12=-4332
BOT CHORD 2-17=3599, 16-17=3340, 15-16=3346, 14-15=3346, 13-14=3340, 12-13=3617
WEBS 3-17=-134, 3-18=-294, 11-19=-316, 11-13=-131, 16-18=1216, 5-18=1462, 14-19=1207, 10-19=1471, 5-20=-2350, 20-21=-2360, 10-21=-2350, 17-18=275, 13-19=294, 6-20=186, 8-21=185

- NOTES**
- This truss has been designed for the loads generated by 85 mph winds at 25 ft above ground level located 100 mi from the hurricane oceanline. ASCE 7-95 components and cladding external pressure coefficients for the interior (1) zone and 7.0 psf top chord and 7.0 psf bottom chord dead load are being used. The design assumes occupancy category II, terrain exposure C and internal pressure coefficient condition I. The building dimensions are 45 ft by 24 ft. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - Unbalanced snow loads have been considered for this design.
 - Provide adequate drainage to prevent water ponding.
 - All plates are M20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - Ceiling dead load (5.0 psf) on member(s) 5-6, 6-8, 8-10, 5-20, 20-21, 10-21
 - Bottom chord live load (100.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 14-16
 - A plate rating reduction of 20% has been applied for the green lumber members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 12 and 59 lb uplift at joint 2.
 - This truss has been designed with ANSI/TPI 1-1995 criteria.
 - Overhang snow load factor (2.00)

LOAD CASE(S) Standard



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 Placerville, CA 95682
 (530) 622-1264; Fax 622-0242

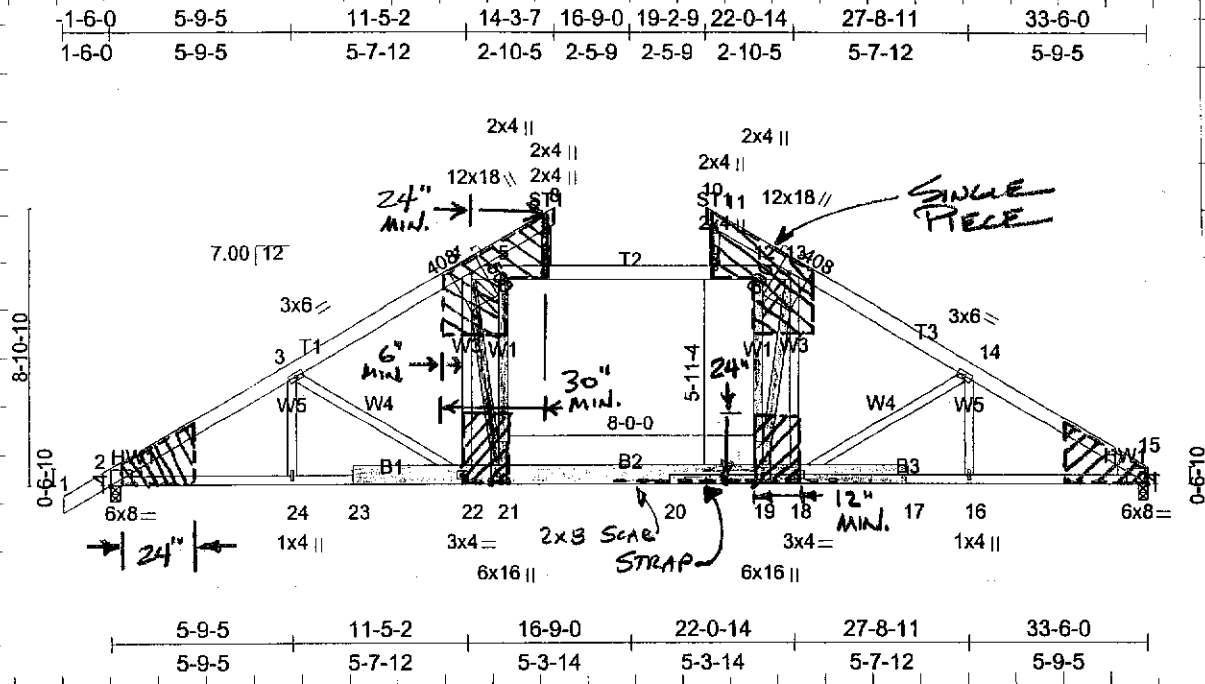
NAME GROUP ONE SAKOUYE RESIDENCE

JOB SAKOUYE PAGE 11 OF 21

BY D. LARRY PETITE REV _____

SCALE _____ DATE 9.26.01

3.0 TRUSS A-14 (RERUN AS A-14B - ATTACHED)

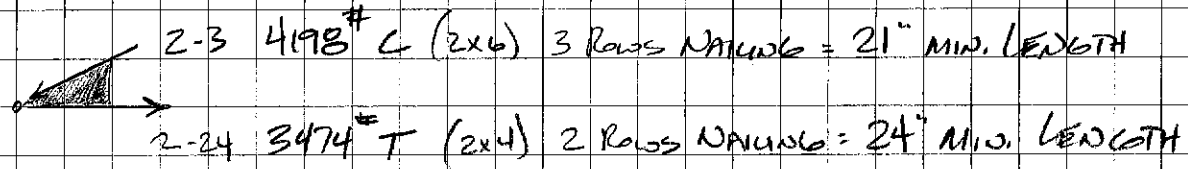


3.1 ADDITIONAL WEBS AND SCABS

- 2x4 DF N° 1 & BTR-G WEBS: (CONNECTED WITH PLYWOOD GUSSETS)
 4-21 AND 13-19; 5-21 AND 12-19 (W1)
- 2x8 DF SS SCAB - ONE SIDE ONLY - 18" ± LONG 17-23
 FACE NAIL TO (E) 2x4 BOTTOM CHORD OVER PLYWOOD
 (FULL BENDED) PLYWOOD GUSSETS: 16d SINKERS 4" O.C. STAGGERED

3.2 TRUSS A-14 (RERUN AS A-14B) GUSSET DESIGNS

HEEL JOINTS 2 AND 15 (SYMMETRICAL)



USE 24" LONG GUSSET ALONG BOTTOM CHORD EACH HEEL

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JOB SAKOYE PAGE 12 OF 21

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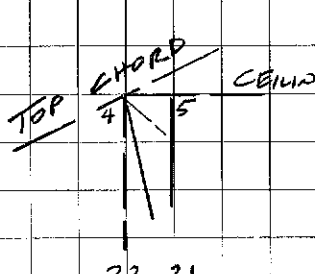
SCALE _____ DATE 9.26.01

3.2 TRUSS A14 (REVISED AS A-14B) GUSSET DESIGNS CONT.

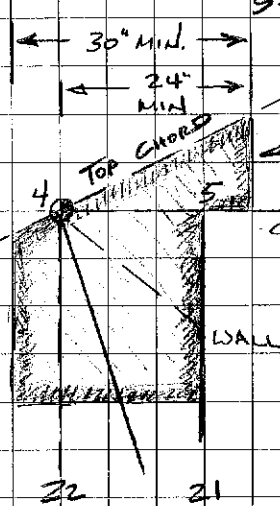
TOP CHORD JOINTS 3 AND 14 (SYMMETRICAL)

(E) PLATE TRUSS A14 = 3x6 ; REVISED TRUSS A-14B = 3x6 OK

TOP CHORD JOINTS 4 AND 5; 12 AND 13; AT CEILING

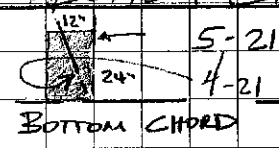


- 4-5 3681# C (2x6) 3 ROWS NAILING = 18" MIN.
- 4-21 1732# T (2x4) 2 ROWS NAILING = 12" MIN.
- 4-22 310# C (2x4) 2 ROWS NAILING = 6" MIN.
- 5-21 215# C (2x4) 2 ROWS NAILING = 6" MIN.



USE COMPOSITE GUSSET AS SINGLE PIECE

BOTTOM CHORD JOINTS 19 AND 21 (SYMMETRICAL)



- 5-21 (SEE ABOVE - 6" MIN.)
- 4-21 (SEE ABOVE - 12" MIN.)

USE 12" WIDE X 24" HIGH GUSSETS JOINTS 19 AND 21

BOTTOM CHORD SPACE:

USE 2x8 SEAB ONE SIDE ; SIMPSON LST I 73
OPPOSITE SIDE (OVER PLYWOOD GUSSETS)

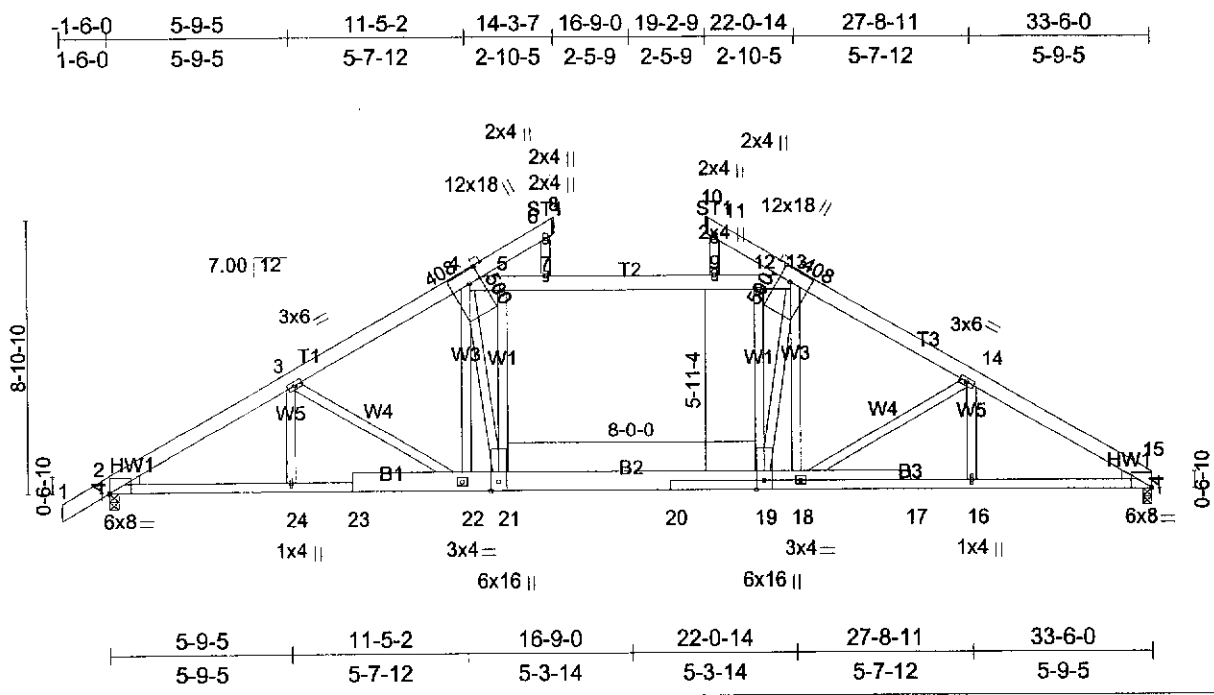


Plate Offsets (X, Y): [2:0-0-0,0-0-4], [4:0-4-8,0-5-0], [13:0-4-8,0-5-0], [15:0-0-0,0-0-4]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 20.0	Plates Increase 1.15	TC 0.33	Vert(LL) -0.26 19-21 >999	M20 220/195
TCDL 15.0	Lumber Increase 1.15	BC 0.72	Vert(TL) -0.37 19-21 >999	
BCLL 0.0	Rep Stress Incr NO	WB 0.99	Horz(TL) 0.15 15 n/a	Weight: 275 lb
BCDL 10.0	Code UBC97/ANSI95	(Matrix)	1st LC LL Min l/defl = 360	

LUMBER
 TOP CHORD 2 X 6 DF SS-G
 BOT CHORD 2 X 4 DF No.1&Btr-G *Except*
 B2 2 X 8 DF SS-G
 WEBS 2 X 4 DF Stud/Std-G
 OTHERS 2 X 4 DF Stud/Std-G
 LBR SCAB 17-23 2 X 8 DF X SS one side
 WEDGE Left: 2 X 4 DF Stud/Std, Right: 2 X 4 DF Stud/Std

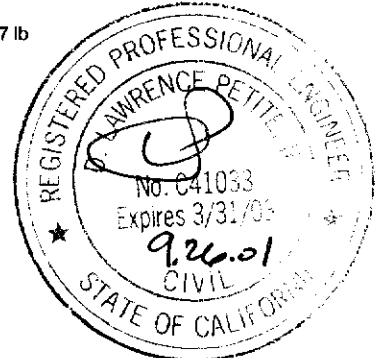
REACTIONS (lb/size) 15=2423/0-3-8, 2=2543/0-3-8
 Max Horz 2=21(load case 5)
 Max Uplift 15=-183(load case 5), 2=-187(load case 5)
 Max Grav 2=2607(load case 2)

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=47, 2-3=-4198, 3-4=-3936, 4-6=-103, 6-8=-10, 4-5=-3681, 5-7=-3681, 7-9=-3681, 9-12=-3681, 12-13=-3681, 10-11=-10, 11-13=-102, 13-14=-3940, 14-15=-4198
 BOT CHORD 2-24=3474, 23-24=3474, 21-22=3414, 20-21=3681, 19-20=3681, 18-19=3415, 17-18=3500, 16-17=3500, 15-16=3500
 WEBS 3-24=36, 3-22=-79, 4-22=-310, 13-18=-295, 14-18=-107, 14-16=44, 6-7=-9, 9-11=-11, 5-21=-215, 12-19=-213, 4-21=1732, 13-19=1723

BRACING
 TOP CHORD Sheathed or 3-9-14 on center purlin spacing.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing.

- NOTES**
- 1) This truss has been designed for the loads generated by 85 mph winds at 25 ft above ground level located 100 mi from the hurricane oceanline. ASCE 7-95 components and cladding external pressure coefficients for the interior (1) zone and 7.0 psf top chord and 7.0 psf bottom chord dead load are being used. The design assumes occupancy category II, terrain exposure C and internal pressure coefficient condition I. The building dimensions are 45 ft by 24 ft. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 2) Unbalanced snow loads have been considered for this design.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) All plates are M20 plates unless otherwise indicated.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - 6) Bottom chord live load (100.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 19-21
 - 7) A plate rating reduction of 20% has been applied for the green lumber members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 163 lb uplift at joint 15 and 187 lb uplift at joint 2.
 - 9) This truss has been designed with ANSI/TPI 1-1995 criteria.
 - 10) Overhang snow load factor (2.0)

LOAD CASE(S) Standard

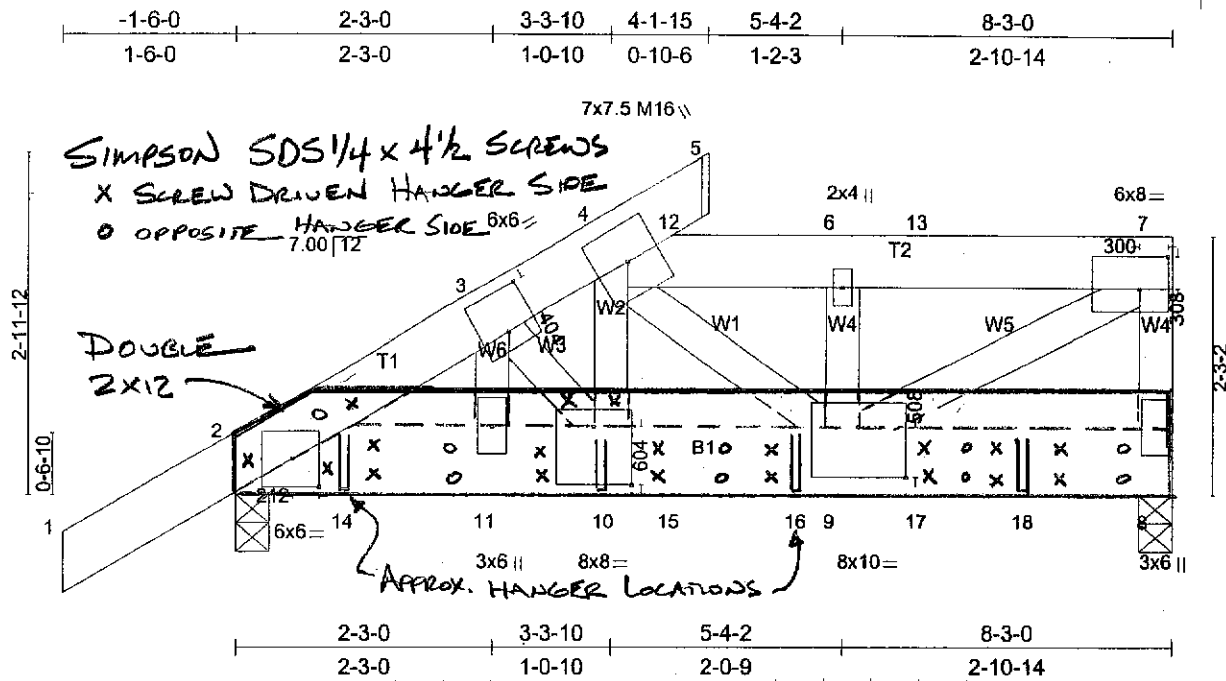


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 JOB SAKOUYE PAGE 15 OF 21
 BY D. LARRY PETTE REV _____
 SCALE _____ DATE 9.26.01

4.0 TRUSS B (RERUN AS B (9.24.01))



RERUN TRUSS B SHOWN WITH (2) 2x12 BOTTOM CHORD SCABS.

DOUBLE CHECK WITH WOODWORKS CALCULATION USING 4 POINT LOADS OF 1027# LIVE FOR EACH INCOMING TRUSS. $1027 / 272 = 4$ SIMPSON SCREWS/HANGER

USE (E) B TRUSS WITH (2) 2x12 DF #1 FULL LENGTH BOTTOM CHORD SCABS ATTACHED WITH SIMPSON SDS 1/4 x 4 1/2 WOOD SCREWS AS SHOWN ABOVE.

NOTE: IF HANGER NAILS EXTEND THROUGH (E) SINGLE PLY TRUSS REMOVE AND REPAIR WITH 16d COMMONS DO NOT INSTALL SIMPSON SCREWS THROUGH (E) METAL GUSSET TRUSS PLATES

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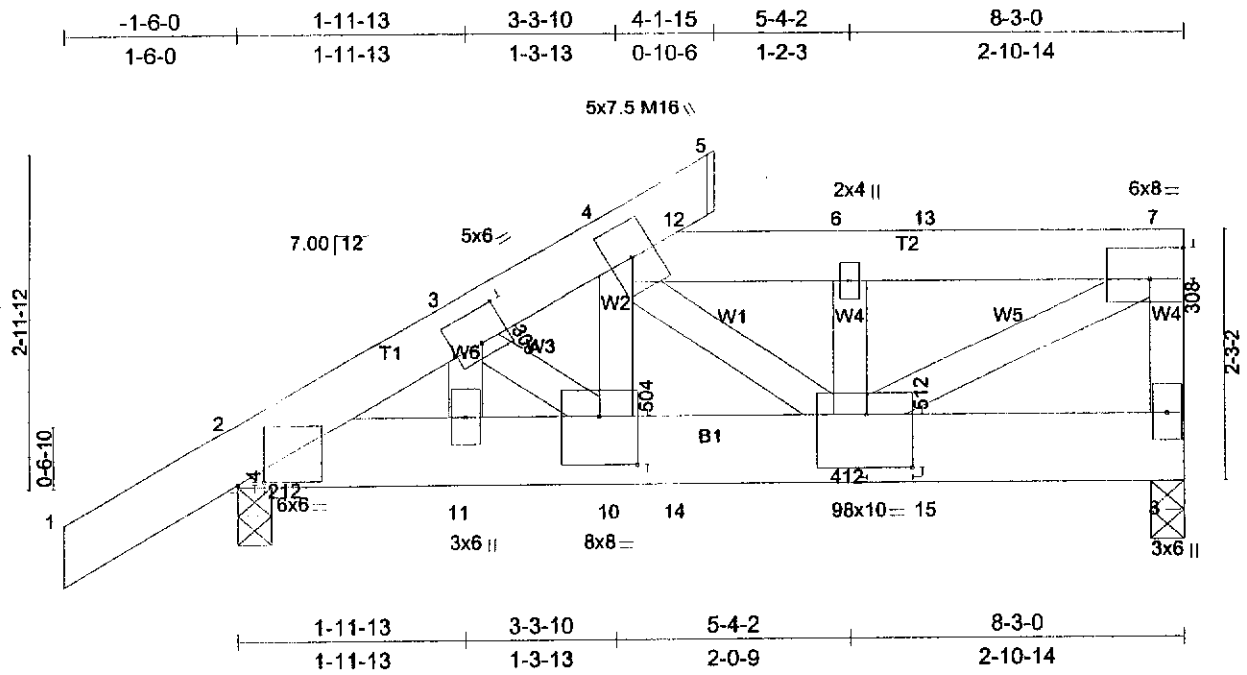


Plate Offsets (X, Y): [2:0-2-12,0-0-4], [3:0-3-0,0-3-8], [7:edge,0-3-8], [9:0-4-12,0-5-12], [10:0-4-0,0-5-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL (in)	(loc)	I/defl	PLATES	GRIP
TCLL 20.0	Plates Increase 1.15		TC 0.51	Vert(LL) -0.03	9-10	>999	M20	220/195
TCDL 15.0	Lumber Increase 1.15		BC 0.78	Vert(TL) -0.06	9-10	>999	M16	176/121
BCLL 0.0	Rep Stress Incr NO		WB 0.55	Horz(TL) 0.01	8	n/a		
BCDL 10.0	Code UBC97/ANSI95		(Matrix)	1st LC LL Min I/defl = 360				Weight: 62 lb

LUMBER
 TOP CHORD 2 X 6 DF SS-G
 BOT CHORD 2 X 8 DF No.2-G
 WEBS 2 X 4 DF Stud/Std-G *Except*
 W1 2 X 4 DF No.1&Btr-G, W5 2 X 4 DF No.1&Btr-G

BRACING
 TOP CHORD Sheathed or 3-6-14 on center purlin spacing, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing. Except: 6-0-0 on center bracing: 8-9.

REACTIONS (lb/size) 8=3318/0-3-9 (input: 0-3-8), 2=3422/0-3-12 (input: 0-3-8)
 Max Horz 2=96(load case 5)
 Max Uplift 8=-209(load case 5), 2=-201(load case 5)
 Max Grav 2=3506(load case 2)

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=55, 2-3=-4286, 3-4=-4023, 4-5=-35, 4-12=-3398, 6-12=-3399, 6-13=-3399, 7-13=-3399, 7-8=-2332
 BOT CHORD 2-11=3504, 10-11=3504, 10-14=3683, 9-14=3683, 9-15=0, 8-15=0
 WEBS 3-11=700, 3-10=372, 4-10=1209, 4-9=-384, 6-9=-59, 7-9=4011

- NOTES**
- This truss has been designed for the loads generated by 85 mph winds at 25 ft above ground level located 100 mi from the hurricane oceanline. ASCE 7-95 components and cladding external pressure coefficients for the interior (1) zone and 7.0 psf top chord and 7.0 psf bottom chord dead load are being used. The design assumes occupancy category II, terrain exposure C and internal pressure coefficient condition I. The building dimensions are 45 ft by 24 ft. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - Unbalanced snow loads have been considered for this design.
 - Except as shown below, special connection(s) required to support concentrated load(s). Design of connection(s) is delegated to the building designer.
 - Provide adequate drainage to prevent water ponding.
 - All plates are M20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - A plate rating reduction of 20% has been applied for the green lumber members.
 - WARNING:** Required bearing size at joint(s) 8, 2 greater than input bearing size.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 209 lb uplift at joint 8 and 201 lb uplift at joint 2.
 - This truss has been designed with ANSI/TPI 1-1995 criteria.
 - Overhang snow load factor (2.00)

LOAD CASE(S) Standard
 1) Snow: Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: 1-2=-70.0, 2-3=-70.0, 3-4=-70.0, 4-5=-70.0, 4-12=-30.0, 6-12=-97.7, 6-13=-97.7, 7-13=-97.7, 2-11=-730.1, 10-11=-730.1, 10-14=-730.1, 9-14=-730.1, 9-15=-730.1, 8-15=-730.1
 Concentrated Loads (lb)
 Vert: 4=-107

H0528 (4)



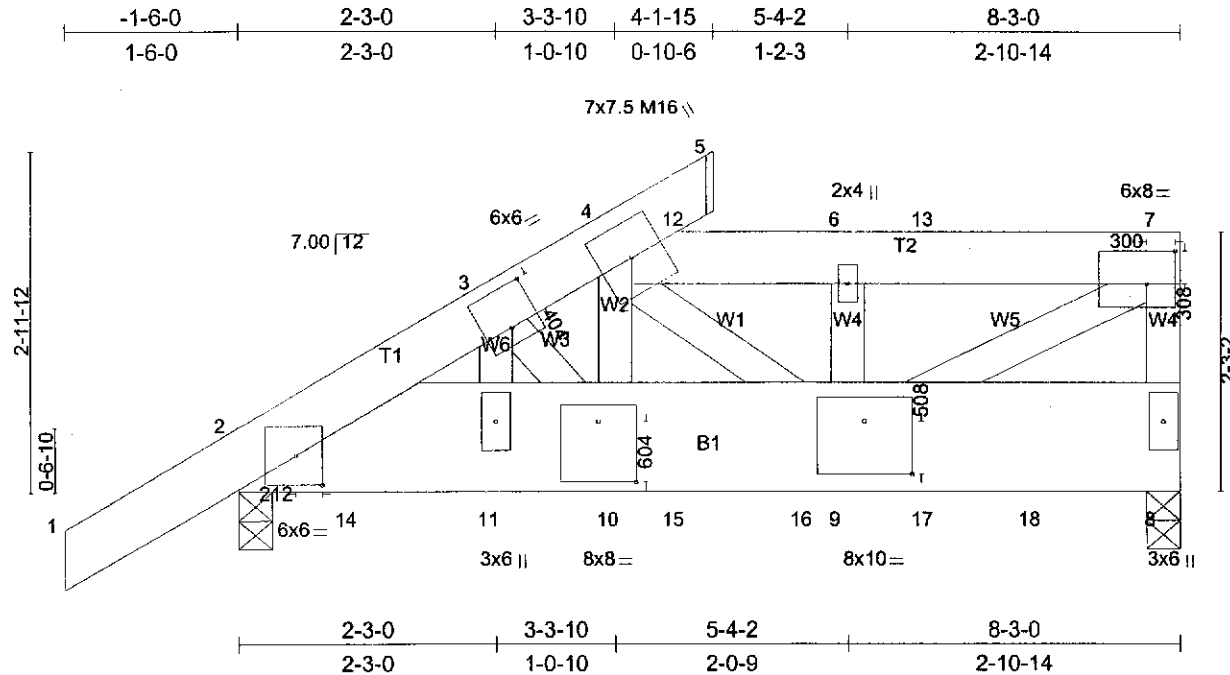


Plate Offsets (X,Y): [2:0-2-12,0-3-0], [3:0-3-0,0-4-4], [7:0-3-0,0-3-8], [9:0-5-0,0-5-8], [10:0-4-0,0-6-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL (in) (loc) l/defl	PLATES GRIP
TCLL 20.0	Plates Increase	1.15	TC 0.58	Vert(LL) 0.00 10 >999	M20 220/195
TCDL 15.0	Lumber Increase	1.15	BC 0.62	Vert(TL) -0.06 9-10 >999	M16 176/121
BCLL 0.0	Rep Stress Incr	NO	WB 0.67	Horz(TL) 0.01 8 n/a	Weight: 134 lb
BCDL 10.0	Code	UBC97/ANSI95	(Matrix)	1st LC LL Min l/defl = 360	

LUMBER
TOP CHORD 2 X 6 DF SS-G
BOT CHORD 2 X 12 DF SS-G *Except*
B1 2 X 8 DF SS-G
WEBS 2 X 4 DF Stud/Std-G *Except*
W5 2 X 4 DF No.1&Btr-G
LBR SCAB 2-8 2 X 12 DF X SS both sides

BRACING
TOP CHORD Sheathed or 3-3-15 on center purlin spacing, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 on center bracing, Except: 6-0-0 on center bracing: 8-9.

REACTIONS (lb/size) 8=5437/0-5-13 (input: 0-3-8), 2=5684/0-6-1 (input: 0-3-8)
Max Horz 2=96(load case 5)
Max Uplift 8=-92(load case 5), 2=-84(load case 5)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=55, 2-3=-4600, 3-4=-4698, 4-5=-35, 4-12=-3826, 6-12=-3826, 6-13=-3826, 7-13=-3826, 7-8=-2605
BOT CHORD 2-14=3810, 11-14=3810, 10-11=3810, 10-15=4349, 15-16=4349, 9-16=4349, 9-17=0, 17-18=0, 8-18=0
WEBS 3-11=176, 3-10=1058, 4-10=1506, 4-9=-707, 6-9=-47, 7-9=4516

- NOTES**
- 1) This truss has been designed for the loads generated by 85 mph winds at 25 ft above ground level located 100 mi from the hurricane oceanline. ASCE 7-95 components and cladding external pressure coefficients for the interior (1) zone and 7.0 psf top chord and 7.0 psf bottom chord dead load are being used. The design assumes occupancy category II, terrain exposure C and internal pressure coefficient condition I. The building dimensions are 45 ft by 24 ft. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 2) Unbalanced snow loads have been considered for this design.
 - 3) Except as shown below, special connection(s) required to support concentrated load(s). Design of connection(s) is delegated to the building designer.
 - 4) Provide adequate drainage to prevent water ponding.
 - 5) All plates are M20 plates unless otherwise indicated.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 - 7) A plate rating reduction of 20% has been applied for the green lumber members.
 - 8) **WARNING:** Required bearing size at joint(s) 8, 2 greater than input bearing size.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 92 lb uplift at joint 8 and 84 lb uplift at joint 2.
 - 10) This truss has been designed with ANSI/TPI 1-1995 criteria.
 - 11) Load case(s) 1 has been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
 - 12) Overhang snow load factor (2.00)

LOAD CASE(S) Standard Except:

- 1) Snow: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-2=-70.0, 2-3=-70.0, 3-4=-70.0, 4-6=-70.0, 4-12=-30.0, 6-12=-97.7, 6-13=-97.7, 7-13=-97.7, 2-14=-27.9, 11-14=-27.9, 10-11=-27.9, 10-15=-27.9, 15-16=-27.9, 9-16=-27.9, 9-17=-27.9, 17-18=-27.9, 8-18=-27.9
Concentrated Loads (lb)
Vert: 4=-107 10=-2487 14=-2508 16=-2487 18=-2487





WoodWorks
SOFTWARE FOR WOOD DESIGN

COMPANY
PETITE & ASSOCIATES
2149 N Shingle Rd
Shingle Springs, CA 95682
(530) 677-6682; Fax 677-4672
Sep. 26, 2001 16:35:18

PROJECT
Group One
Sakouye Residence

SakouyeBeam1

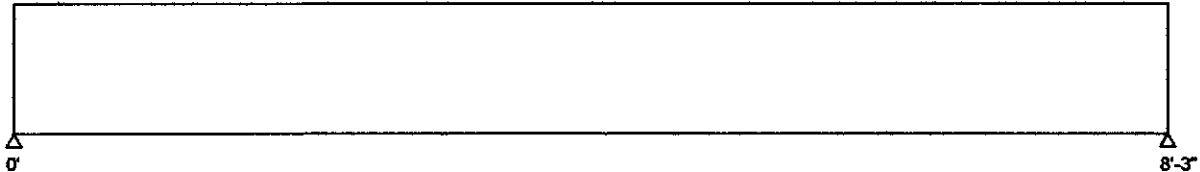
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Design Check Calculation Sheet

LOADS: (lbs, psf, or pif)

Load	Type	Distribution	Magnitude		Location [ft]		Pattern Load?
			Start	End	Start	End	
1	Live	Point	1027		1.00		Yes
2	Live	Point	1027		3.00		Yes
3	Live	Point	1027		5.00		Yes
4	Live	Point	1027		7.00		Yes

MAXIMUM REACTIONS (lbs) and BEARING LENGTHS :



Dead	66		66
Live	2116		1992
Total	2182		2058
Bearing:			
Length	1-3/16"		1-1/8"

Lumber n-ply, D.Fir-L, No.1, 2x12", 2-Plys

Self Weight of 274.65 pif automatically included in loads

lateral support: Top= Full Bottom= Full Load Combinations: ICBO-UBC

SECTION vs. DESIGN CODE NDS-1997: (stress=psi, and in)

Criterion	Analysis Value	Design Value	Analysis/Design
Shear	$f_v \theta_d = 95$	$F_v' = 95$	$f_v/F_v' = 1.00$
Bending(+)	$f_b = 851$	$F_b' = 1000$	$f_b/F_b' = 0.85$
Live Defl'n	$0.09 = <L/999$	$0.28 = L/360$	0.33
Total Defl'n	$0.09 = <L/999$	$0.55 = L/180$	0.17

ADDITIONAL DATA:

FACTORS: F CD CM Ct CL CF CV Cfu Cr LC#
 $F_b' = 1000$ 1.00 1.00 1.00 1.000 1.00 1.000 1.00 1.00 2
 $F_v' = 95$ 1.00 1.00 1.00 (CH = 1.000) 2
 $F_{cp}' = 625$ 1.00 1.00 -
 $E' = 1.7$ million 1.00 1.00 2

Custom duration factor for Wind load = 1.33

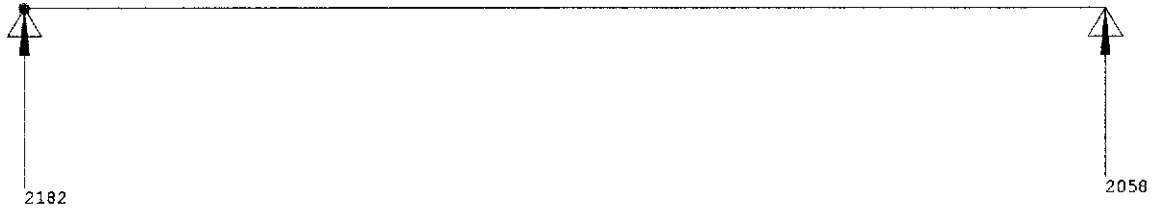
Bending(+): LC# 2 = L, M = 4486 lbs-ft
 Shear : LC# 2 = L, V = 2149, $V \theta_d = 2142$ lbs
 Deflection: LC# 2 = L EI= 302.56e06 lb-in²/ply
 Total Deflection = 1.50(Defln_{dead}) + Defln_{Live}.
 (D=dead L=live S=snow W=wind I=impact C=construction)
 (All LC's are listed in the Analysis output)
 (Load Pattern: s=S/2, X=L+S or L+C, _=no pattern load in this span)

DESIGN NOTES:

1. Please verify that the default deflection limits are appropriate for your application.
2. Sawn lumber bending members shall be laterally supported according to the provisions of NDS Clause 4.4.1.
3. BUILT-UP BEAMS: It is assumed that each ply is a single continuous member (that is, no butt joints are present) fastened together securely at intervals not exceeding 4 times the depth and that each ply is equally top-loaded. Where beams are side-loaded, special fastening details may be required.

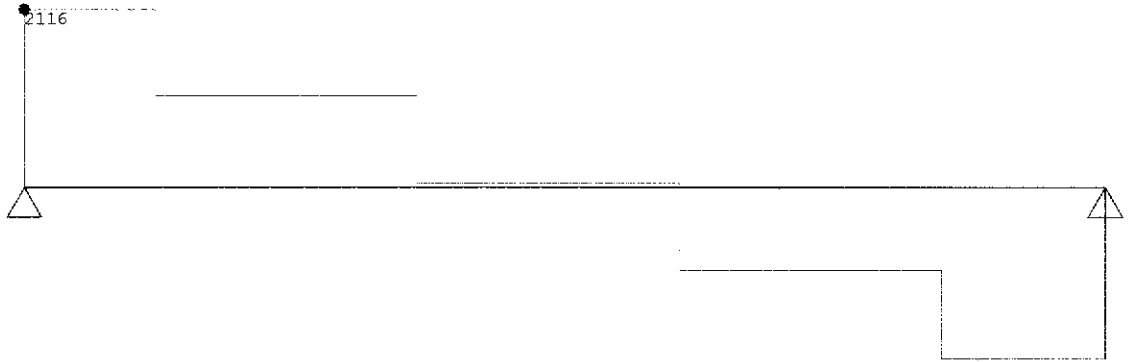
REACTION [lbs]

Maximum...
Uplift: 0
Bearing: 2182



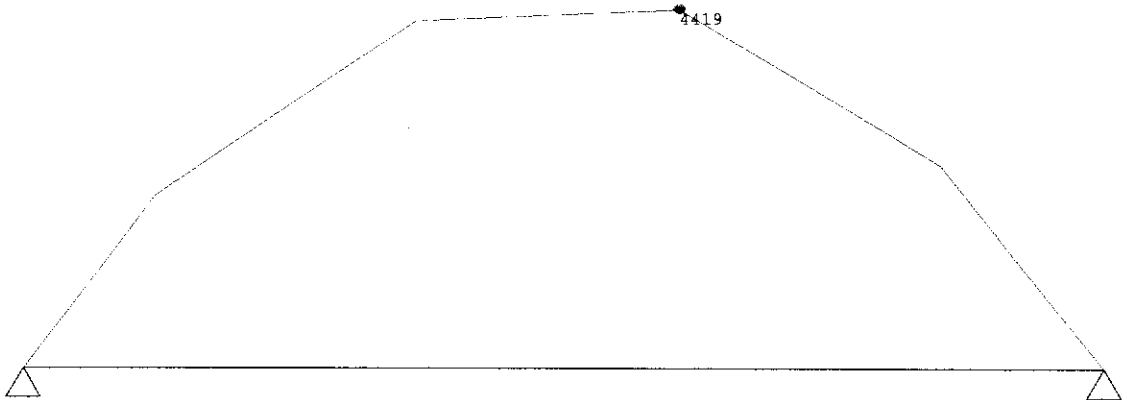
SHEAR [lbs]

+V max: 2116
-V max: -1992
LC# = 2



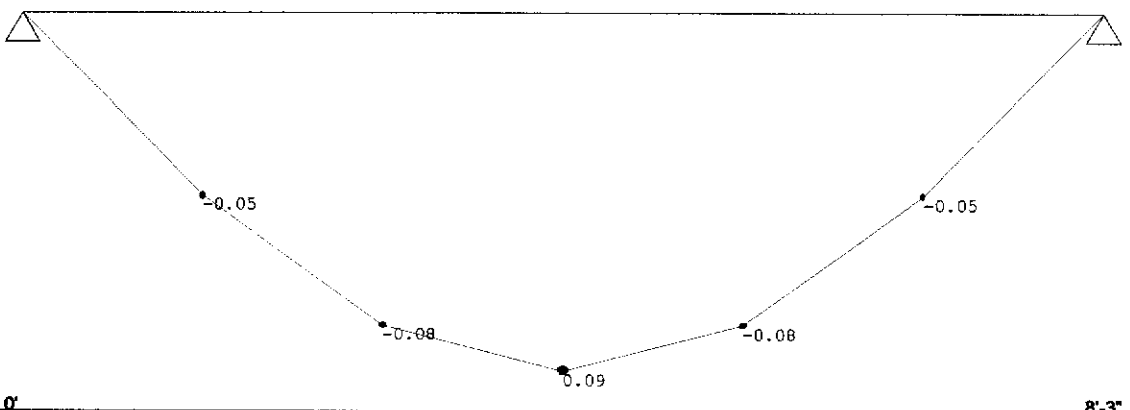
BENDING [lbs-ft]

+M max: 4419
LC# = 2



DEFLECTION [in]

Max Live: 0.09
Max Total: 0.09
LC# = 2



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300 Industrial Drive
Placerville, CA 95667
(530) 622-1264; Fax 622-0242

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SAKOUYE
9.26.01

Plywood Gusset Design

Plywood: 1/2" APA 32/16 span rated plywood, exterior adhesive. *OR 1/2" OSB.*
Nails: 10d gun nails (0.120" x 3.0" min.), 65# per nail; or larger (16d sinker max).

Capacity of 10d Clenched Nail (1977 NDS Section 12.3.3):

Check adequate nail length for clenched connection:

min. clenched length: 3 x's diameter (0.120") = 0.36"

available clenched length: 3.0" - 1.5" - 2(0.5") = 0.50" OK

Check required penetration for full nail value:

$p = 12D = 12 \times 0.120" = 1.44" < 1.5"$ OK for nominal 2x member.

Capacity, $z = 65 \text{ \#/nail} \times 1.00Cd \times 1.75 \text{ clenched} = \underline{113 \text{ \#/clenched nail}}$

Installation: Face nail through front gusset, 2x member and back gusset and then clench against back gusset (nails generally driven through only from one side after tacking back gusset). Use 3/4" min. end and edge nailing distances for both 2x member and gusset. Use 1.5" min. nail spacing between nail rows. This leaves at least a 1/4" wide nailing zone for each nail row.

Notes:

- 1) Brace and properly support truss to maintain plumb, line and camber as dictated by installation location prior to installing plywood gussets.
- 2) Provide a 1/8-inch space between edge of plywood gusset and edges of top and bottom truss chords where sheathing or finish material is attached to the truss to allow for plywood dimension changes due to changing moisture content.
- 3) Provide a 1/8-inch minimum space between adjacent plywood gussets.
- 4) If wood splitting occurs, pre-drill nail holes or contact engineer for further instructions.
- 5) Insure that metal connector plates occurring under gussets are not dislodged during plywood gusset installation and are properly secured tight against truss.
- 6) If you have any questions about how to install the gussets or the size or location of gussets is not clear, please contact the engineer at EDT prior to continuing work.

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2x4 with 1/2" 32/16 Span Rated Plywood Gusset both sides
 Use Two (2) nail rows of 10d clenched gun nails; 1.5" o.c. each row
 Equivalent Connection Capacity of 150.67 pounds per inch

<u>Gusset Length along 2x4</u>	<u>Total Number of Nails</u>	<u>Capacity</u>
6 inches	8 nails	904 pounds
9 inches	12 nails	1356 pounds
12 inches	16 nails	1808 pounds
15 inches	20 nails	2260 pounds
18 inches	24 nails	2712 pounds
21 inches	28 nails	3164 pounds
24 inches	32 nails	3616 pounds

2x6 with 1/2" 32/16 Span Rated Plywood Gusset both sides
 Use Three (3) nail rows of 10d clenched gun nails; 1.5" o.c. each row
 Equivalent Connection Capacity of 226 pounds per inch

<u>Gusset Length along 2x6</u>	<u>Total Number of Nails</u>	<u>Capacity</u>
6 inches	12 nails	1356 pounds
9 inches	18 nails	2034 pounds
12 inches	24 nails	2712 pounds
15 inches	30 nails	3390 pounds
18 inches	36 nails	4068 pounds
21 inches	42 nails	4746 pounds
24 inches	48 nails	5424 pounds